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**SHAKING TABLE TESTS OF A REINFORCED
CONCRETE PRECAST BUILDING SYSTEM**

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Trabalho realizado para o projecto:
PRECAST STRUCTURES EC8 – Seismic Behaviour
of Precast Concrete Structures with Respect to Eurocode 8

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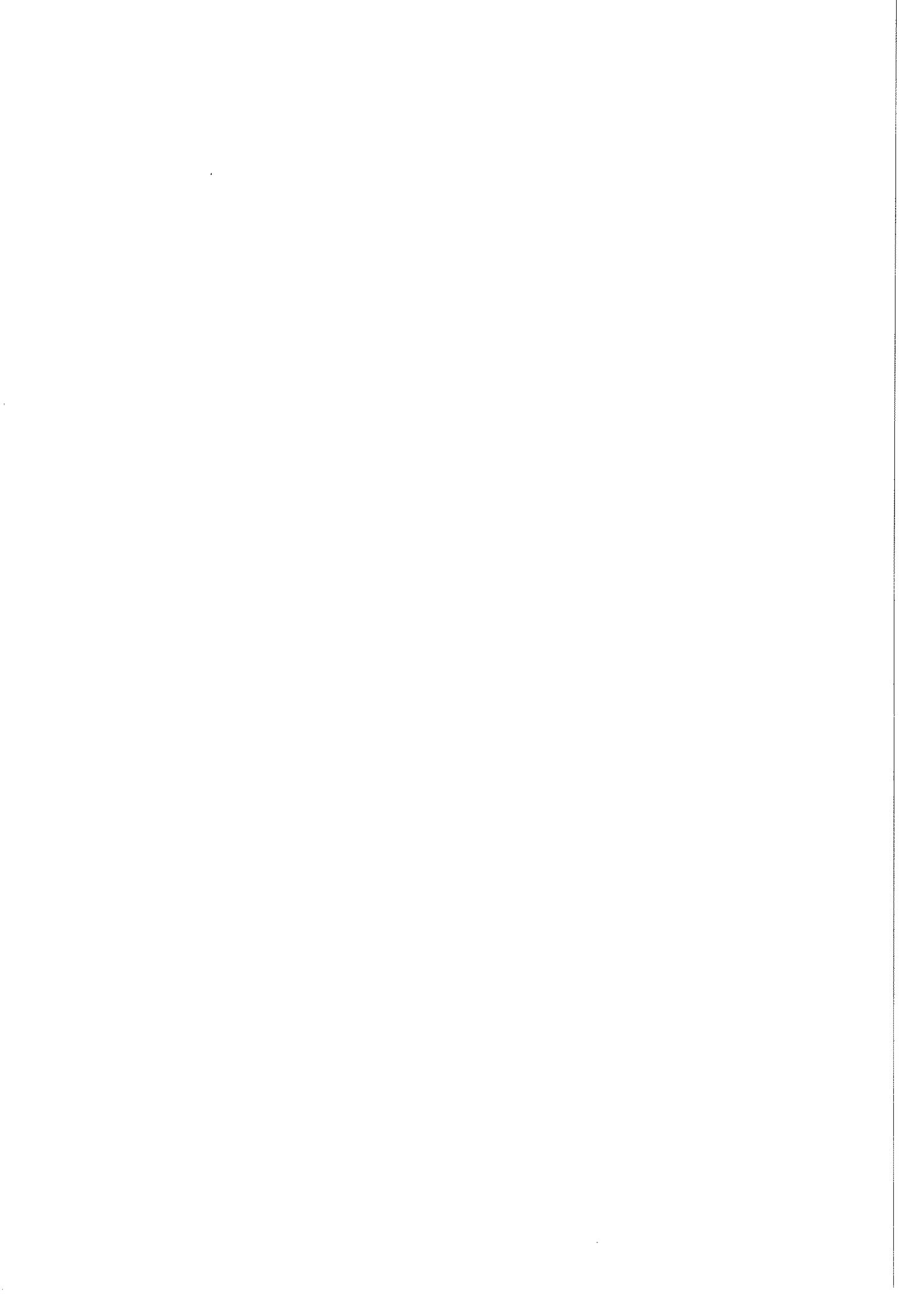


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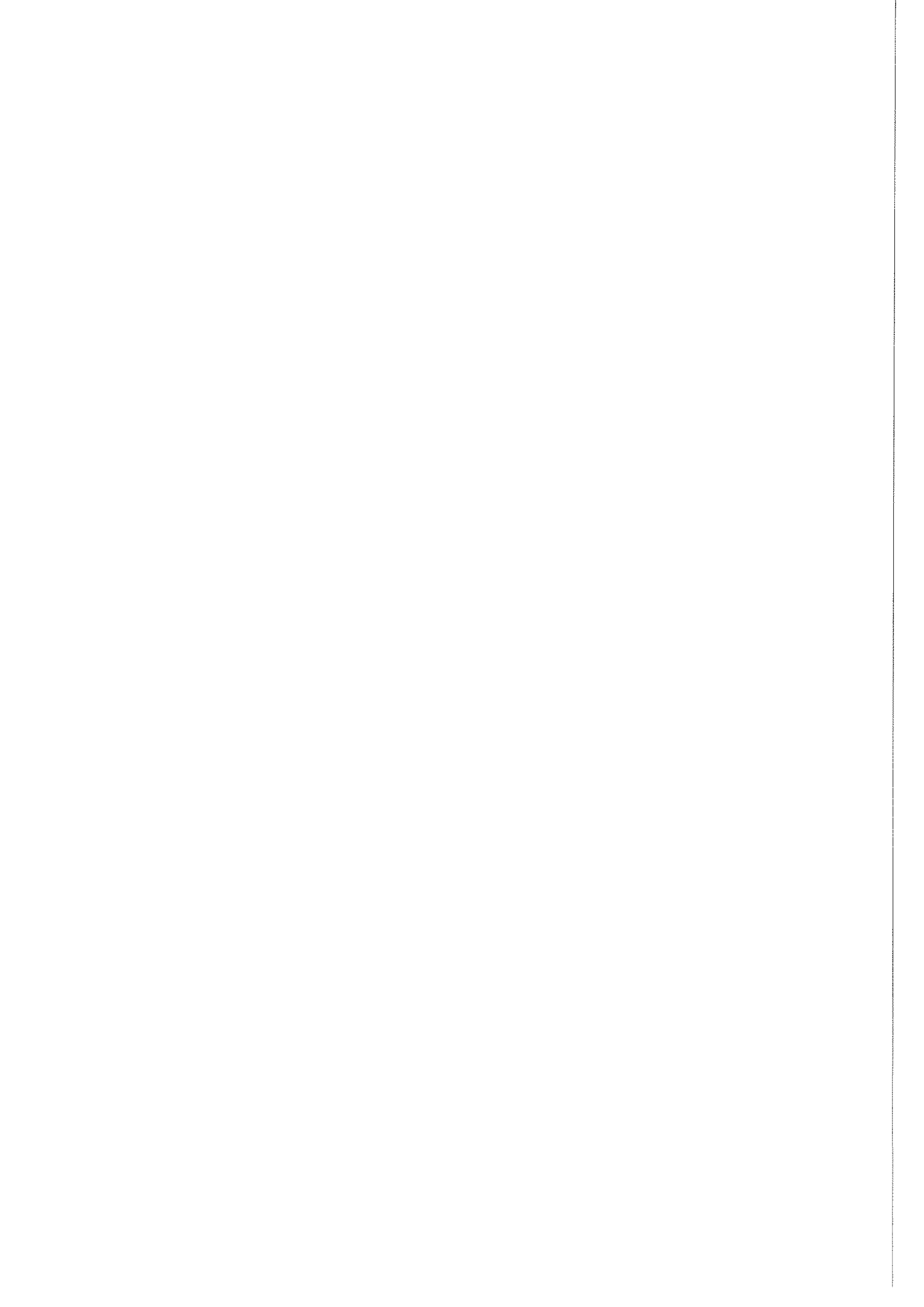
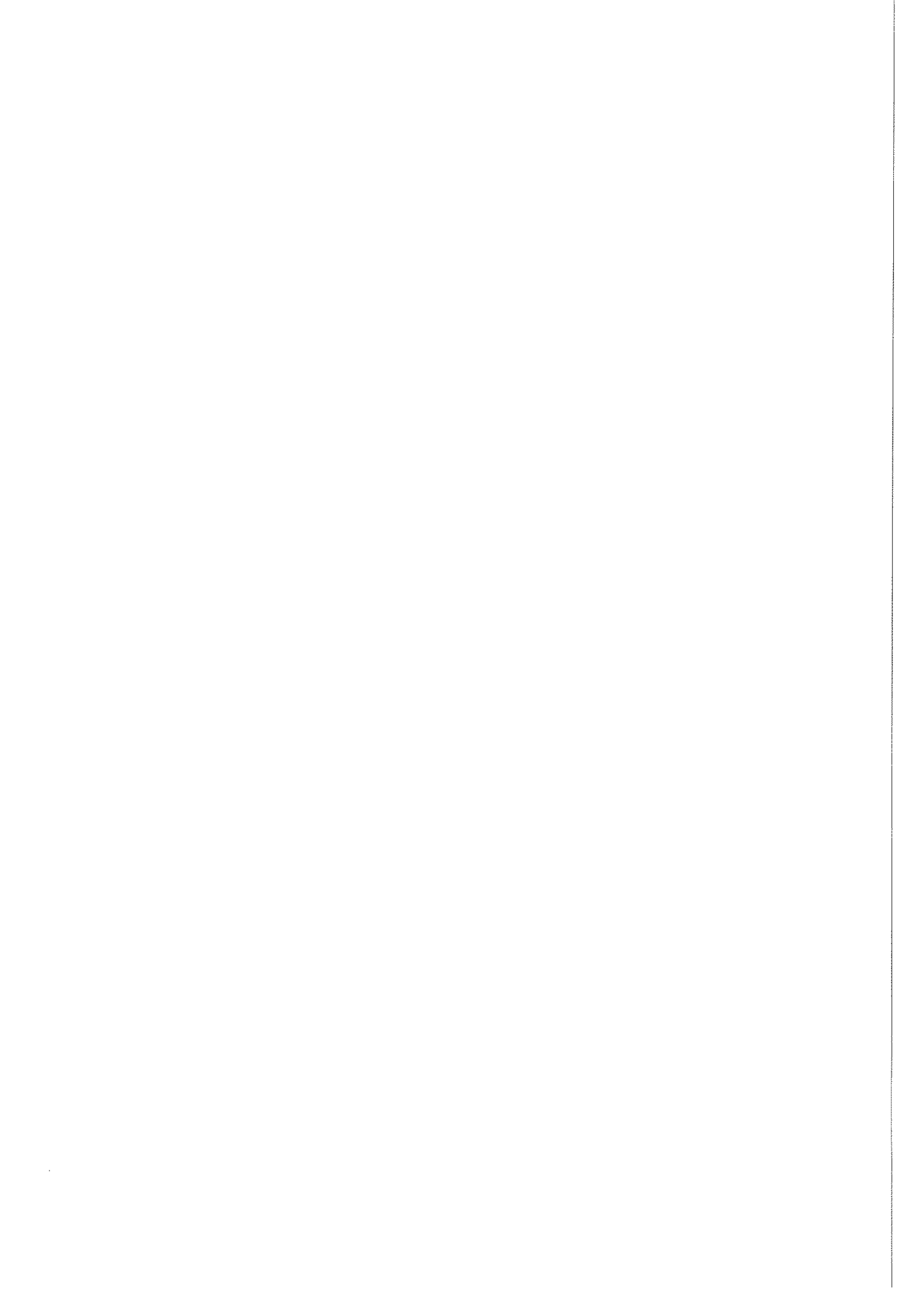


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1 INTRODUCTION

This report describes the first phase activity of the Portuguese group in the *Precast Structures EC8* research project. The Portuguese group is composed by *Laboratório Nacional de Engenharia Civil* (LNEC) and the *Civibral, S.A.* precast company.

In this phase, seismic tests were made in the LNEC's triaxial shaking table to a 2 stories RC precast structure, designed by *Civibral* using their usual practice. The same company supplied the precast elements and erected them directly on top of the shaking table. The rest of the tasks were performed by the LNEC staff.

This report presents the results and the analysis of the seismic behaviour of the tested specimen. The text is organized in 9 sections. After this introduction, the reference prototype and the idealized model are presented, followed by the instrumentation and test setup. Afterwards, is discussed the global behaviour of the structure and of the beam-column connections, followed by the test conclusions, acknowledgments and bibliography. To make the text more readable, the majority of the test data is presented in 14 annexes and only summaries of results are presented in the main text.

2 PROTOTYPE

2.1 General characteristics

The chosen prototype is a typical precast system from *Civibral* and it is used mainly for industrial buildings. The structure has 2 stories with 4.5m of height each and 10.5x12.0 m² in plan. In the longest direction there are 2 bays and only one in the other. All the columns have 0.45x0.45 m² of cross section. There are two types of beams: one is L shaped for receiving the loads from the slabs, with outside dimensions of 0.70x0.45 m²; the other is a rectangular section of 0.70x0.30 m² and does not receive load directly from the slabs. The slabs are 2.0 m wide ribbed panels, which structurally works only in one direction. After the erection of the structure, a 5 cm thickness topping layer is concreted on top of the panels. The connections between elements are discussed in section 3 (Model). More detailed information about the prototype is presented in the Annex B drawings.

2.2 Prototype design

The prototype was designed by *Civibrat* using their usual practice and considering the Portuguese codes [REAE 1986; REBAP 1983; RSA 1983]. The design report of the prototype is presented in the Annex A.

2.2.1 Loads

The dead loads were calculated using the usual specific weight of RC elements (25 kN/m³). It was also considered a live load of 5 kN/m² on the floors ($\psi_0 = 0.7$; $\psi_1 = 0.6$; $\psi_2 = 0.4$). The earthquake load was obtained from the horizontal components of the semi-artificial *Tolmezzo* accelerograms (Figure 4-1), which were corrected to be compatible with the Eurocode 8, type B soil and 5% of damping [EC8 2003]. The amplitudes of the accelerograms were scaled for PGA=0.25g.

It was used the earthquake spatial combination of the Eurocode 8 [EC8 2003]:

$$\begin{cases} E_x + 0.3 E_y \\ 0.3 E_x + E_y \end{cases} \quad (2.1)$$

The combination of loads was calculated using the Portuguese code RSA [RSA 1983]:

$$\begin{cases} S_d = \sum_i \gamma_{g,i} G_{k,i} + \gamma_q \left(Q_{k,l} + \sum_{j=2}^n \psi_{0,j} Q_{k,j} \right) \\ S_d = \sum_i G_{k,i} + \gamma_q E_k + \sum_{j=2}^n \psi_{2,j} Q_{k,j} \end{cases}, \quad (2.2)$$

where:

- $G_{k,i}$ – is the effect of the dead loads;
- $Q_{k,l}$ – is the effect of the main live load used in each combination;
- $Q_{k,j}$ – is the effect of the other live loads considered;
- E_k – is the effect of the earthquake load;
- $\gamma_{g,i}$ – is the partial factor for the dead loads (1.35 for unfavourable effects and 1.0 for favourable);
- γ_q – is the partial factor for the live loads (1.5);
- ψ – is the combination factor.

For comparing the results presented in this report, with the results obtained using other structural codes, that do not apply a partial factor to the earthquake load (e.g. [EC8 2003]), the structure was design for PGA=0.375g, because the effects of the earthquake loads were amplified by a 1.5 scale factor (2.2).

The effects of the earthquake load were obtained from the elastic values reduced by a global behaviour factor of $q=2.0$.

2.2.2 Materials

The adopted materials are indicated in the Annex B drawings.

2.2.3 Method of analysis

A linear time-history analysis was computed in a commercial 3D finite element program, considering pinned connections between the columns and the beams and full restrained connections between the columns and the foundations.

For considering second order and time effects it was adopted the accidental eccentricity, the creep eccentricity and the second order eccentricity defined in RSA [RSA 1983].

2.2.4 Safety verifications

The safety verifications were made considering the ultimate limit states and serviceability limit states defined in the Portuguese codes.

3 MODEL

3.1 Similitude law

The model conception took in consideration the characteristics of the LNEC's triaxial shaking table (presented in Annex C), namely:

1. Dimensions of the shaking table platform ($4.60 \times 5.60 \text{m}^2$);
2. Payload capacity of the shaking table (40 tons).

It was chosen a geometric scale of $1/3$ as indicated in equation (3.1). Adopting this geometric scale made possible to use in the scaled specimen the same materials of the prototype (3.2) and still not exceed the 40 tons payload capacity:

$$L_P/L_M = \lambda = 3, \quad (3.1)$$

$$E_P/E_M = 1. \quad (3.2)$$

In dynamic tests it is usual to adopt the Cauchy similitude and the Froude similitude simultaneously [Carvalho 1998], that can be expressed through the following relations:

$$\text{Cauchy value} = \frac{\rho v^2}{E}, \quad (3.3)$$

$$\text{Froude value} = \frac{v^2}{Lg}, \quad (3.4)$$

leading to the additional relations:

$$\left(\frac{\rho v^2}{E} \right)_P = \left(\frac{\rho v^2}{E} \right)_M; \quad (3.5)$$

$$\left(\frac{v^2}{Lg} \right)_P = \left(\frac{v^2}{Lg} \right)_M. \quad (3.6)$$

With the relations indicated in the equations (3.1), (3.2), (3.5) and (3.6), is possible to define all the relevant scale factors (see Table 3-1).

Parameter	Symbol	Scale Factor $()_P / ()_M$
Length	L	$L_P / L_M = \lambda = 3$
Modulus of elasticity	E	$E_P / E_M = e = 1$
Specific mass	ρ	$\rho_P / \rho_M = \rho = \lambda^{-1} = 0.33333$
Area	A	$\lambda^2 = 9$
Volume	V	$\lambda^3 = 27$
Mass	m	$\lambda^2 = 9$
Displacement	d	$\lambda = 3$
Velocity	v	$\lambda^{1/2} = 1.73205$
Acceleration	a	1
Weight	w	$\lambda^2 = 9$
Force	F	$\lambda^2 = 9$
Moment	M	$\lambda^3 = 27$
Stress	σ	1
Strain	ε	1
Time	t	$\lambda^{1/2} = 1.73205$
Frequency	f	$\lambda^{-1/2} = 0.57735$

Table 3-1: Similitude law scale factors.

The major consequences of using this similitude law are:

- time “compression” of the test,

$$t_P / t_M = \sqrt{\lambda} = 1.73205; \quad (3.7)$$

- same accelerations on the model and on the reference prototype:

$$a_P / a_M = 1; \quad (3.8)$$

- the specific mass of the model should be λ times greater than the mass of the prototype:

$$\rho_P / \rho_M = \lambda^{-1} = 0.333(3). \quad (3.9)$$

This last relation has an important consequence on the tests. Since we are using the same materials (initial criteria), it is required to use additional masses on the scaled specimen.

3.2 Additional masses calculations

3.2.1 Number of masses

In Figure 3-1 is presented a summary of the calculations made for determining the amount of additional masses to be added to the specimen.

Weight of the prototype after concreting: $W^P = 1784.25 \text{ kN};$
Equivalent value for the model: $W^M = \frac{1784.25}{\lambda^2} = 198.25 \text{ kN};$
Weight of the model after concreting (includes topping layer): $W^P = 157.55 \text{ kN};$
Weight due to the reduced value of the live load: $W^{SC} = \psi_2 \times Q \times A_f \times \text{nfloors} = 56.00 \text{ kN};$
Weight to be added: $W^{MAS} = 198.25 - 157.55 + 56 = 96.7 \text{ kN};$
$= 8 \text{ masses of } 6 \text{ kN in each floor.}$

Figure 3-1: Additional masses calculations.

3.2.2 Positioning criteria

The used criteria was to enforce that the additional masses should have the same translational inertia and polar moment of inertia, of a uniform mass thin shell with the same mass and with the dimensions of the slabs (see Figure 3-2)

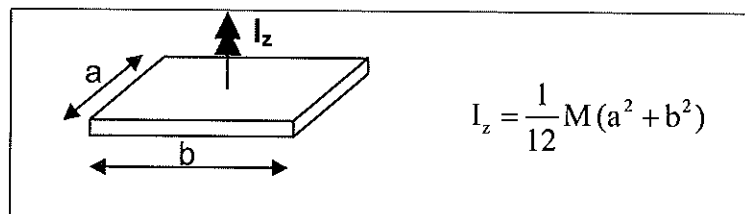


Figure 3-2: Additional masses positioning criteria.

The adopted distribution is presented on drawing n°9 in the Annex D and a detailed view of the fixing system in the Figure F-9.

3.3 General characteristics

The adopted model is directly a 1/3 scaled version of the prototype with differences only on the slabs, which are solid instead of ribbed because of construction difficulties. The thickness of the model's topping layer was chosen to match the flexure moment of inertia of the prototype. Rubber supports were used to simulate the non-continual contact points between the slabs and the L shaped beams.

There are two types of beam-column connections:

1. The first connects the columns and the L shaped beams (see Figure 3-3 and Figure 3-5a). During erection of the structure, the beams are supported by hollow box sections that are firmly connected inside the columns. On top of the beams there is a steel plate where a pre-stressed bolt connects it to the hollow box. The steel plate is also connected to the column using welded bolts. Before concreting the topping layer the voids are filled with a mortar through an inclined hole.

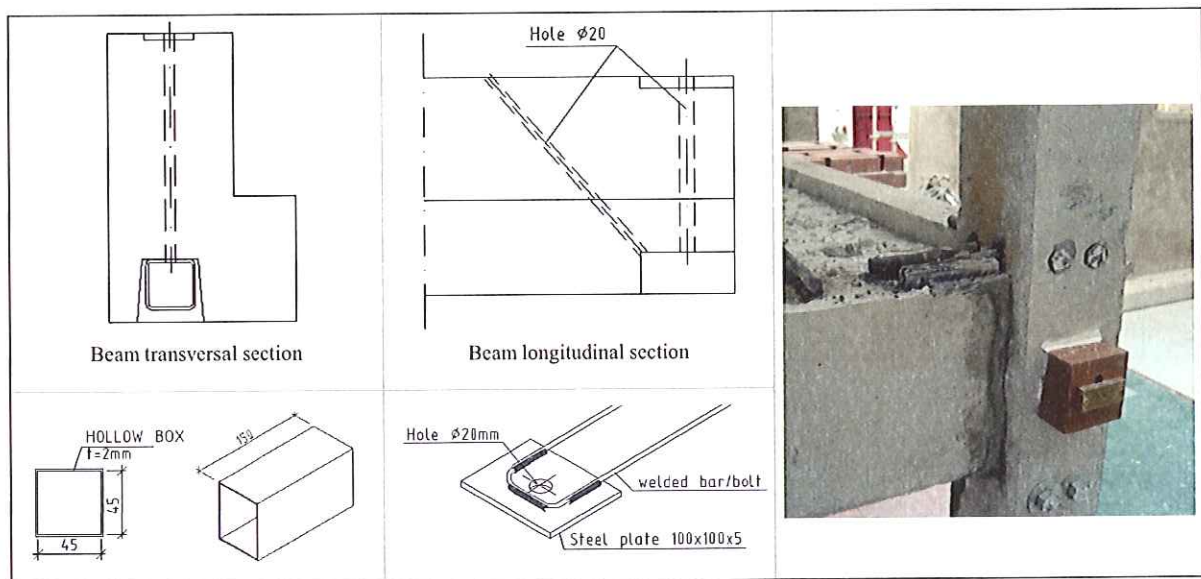


Figure 3-3: Beam-column connection type #1.

2. The second type of beam-column connection is similar to the first one (see Figure 3-4 and Figure 3-5b). The major difference consists in using an angle section instead of the hollow box referred above.



Figure 3-4: Beam-column connection type #2.

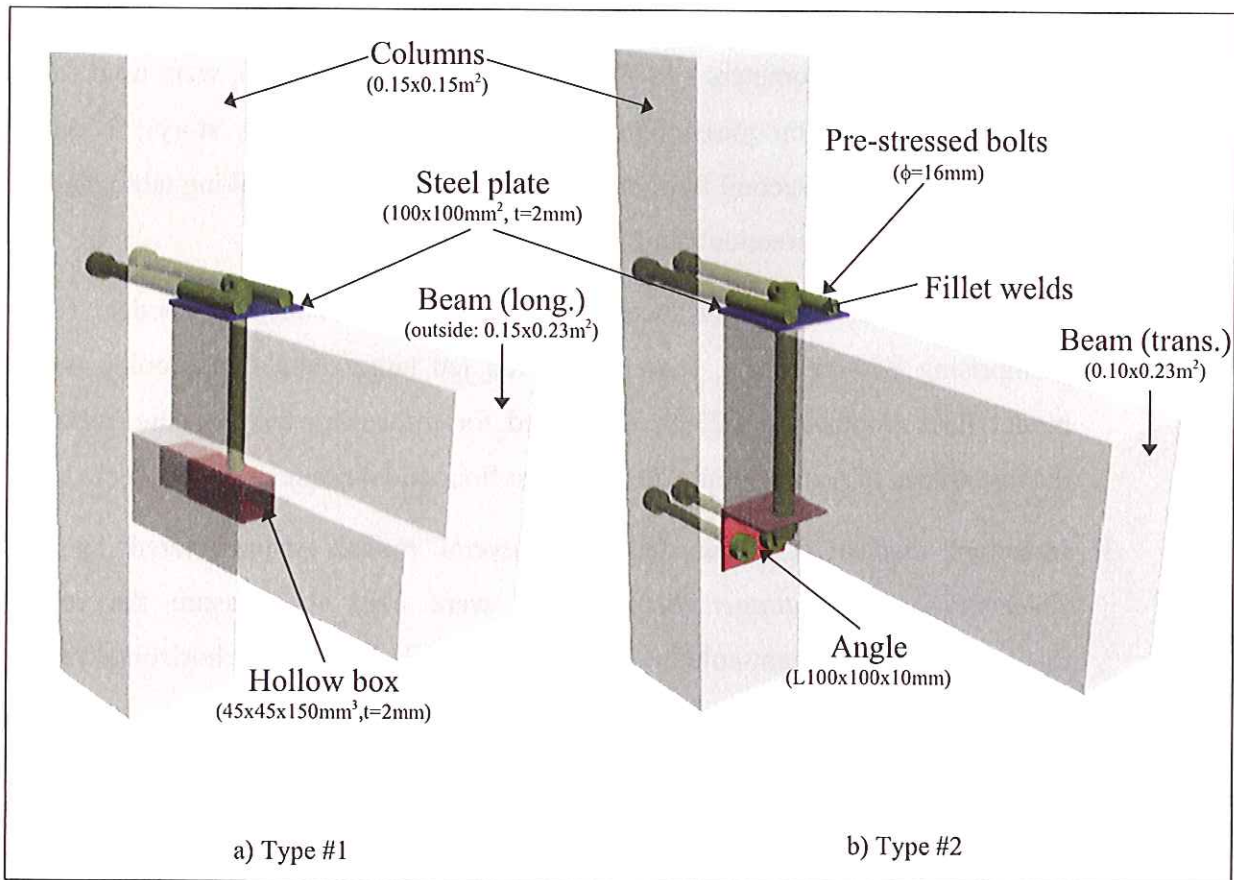


Figure 3-5: 3D visualization of the beam-column connections.

The columns and the footings were cast together in one monolithic element.

Detailed information of the model is available in the Annex D drawings and in the Figures F-1 to F-8.

3.4 Erection of the specimen

The specimen was erected directly on top of the shaking table by *Civibral's* specialized personal. The joint sealing operation and the concreting of the topping layer were made by the LNEC staff.

4 INSTRUMENTATION AND TEST SETUP

4.1 Instrumentation

The specimen was instrumented with the following type of sensors:

1. Piezoelectric accelerometers - *PCB Piezotronics*, model 337A26, were used for the horizontal acceleration measurements on 12 joints (6 in each story), 3 vertical accelerations on the second floor slabs and 4 accelerations in shaking table platform (2 in the horizontal direction and 2 in the vertical).
2. Optical displacement transducers – *Hamamatsu Photonics*, model C5949 (comprising F50 mm lens, sensor head and led target) and conditioning device, model PSH Controllers C2399, were used for measuring the absolute horizontal displacements of 6 slab points (2 on the first floor and 4 on the second floor).
3. Inductive displacement transducers – Several models manufactured by *RDP Electronics* and *Hottinger Baldwin M.*, were used to measure the relative displacements of 8 beam-column connections, 4 slab diagonals, 2 horizontal relative displacements and the horizontal displacements of the shaking table.

More information about the sensors technical specifications are available in the Annex E.

Photographic and video recording were also made during the tests. The photographs were taken using a digital camera and the video records were made using a tripod mounted camera.

4.2 Test setup

In the Annex E drawings is presented detailed information about the positioning of the sensors, channels lists and reports of the math channels calculations. A selection of photographs of the test setup is presented in the Annex F (Figures F10 to F16).

4.2.1 Earthquake input

The earthquake input used on the tests was the horizontal components of the *Tolmezzo* semi-artificial accelerogram, generated to be compatible with the EC8, soil B, 5% damping response spectra (see Figure 4-1). The *Acc X* component was the input in the transversal direction of the shaking table (E-W) and the *Acc Y* component in the longitudinal (N-S), as indicated in Figure 4-2.

The records were time scaled to cope with the similitude law (3.7), adapted to the shaking table, and transformed into displacement records.

4.2.2 Test programme

The test programme included two types of series:

1. The *Stage Series*, which are the main earthquake series. The first stage (Stage 00) was around $PGA=0.1g$ on both directions, followed by increasing intensity stages with the approximated pattern: $PGA^{Trans} \approx PGA^{Long} \times 1.5$. At the end of the test there were 5 stages (Stage 00 – Stage 04).
2. The *Cat Series*, that precede every Stage Series, are low amplitude, broadband base signals with the objective of estimating the specimen's dynamic characteristics and their evolution during the test.

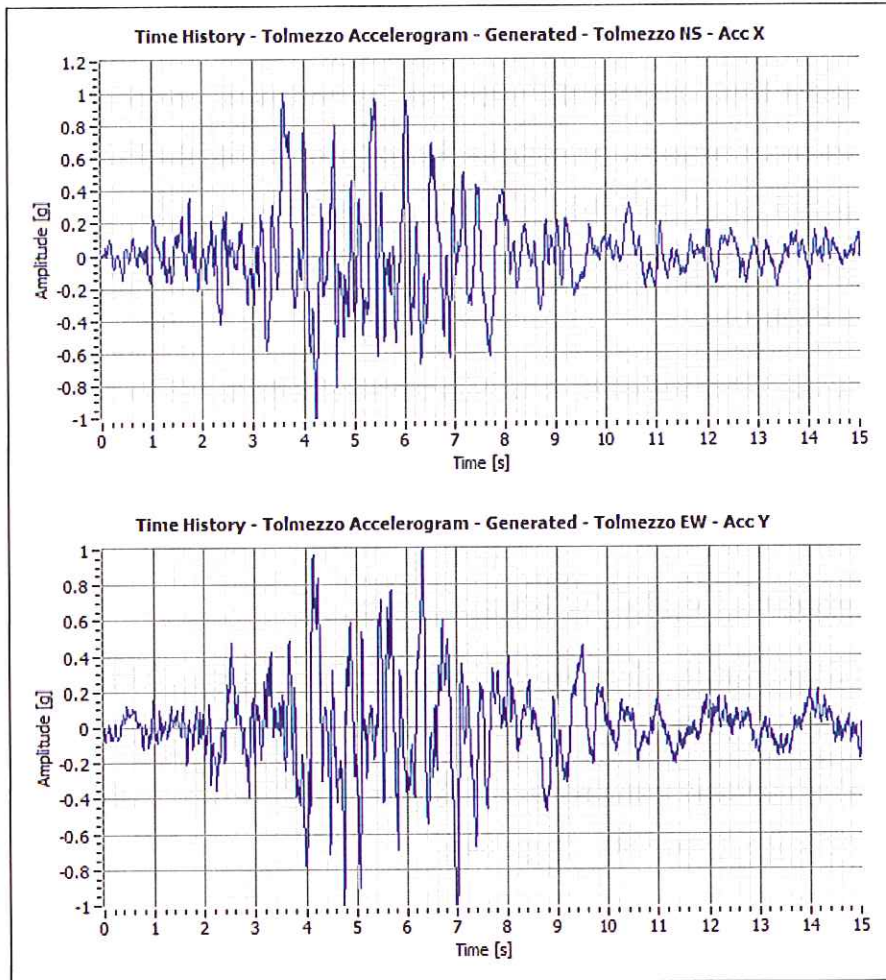


Figure 4-1: Semi-artificial *Tolmezzo* accelerogram.

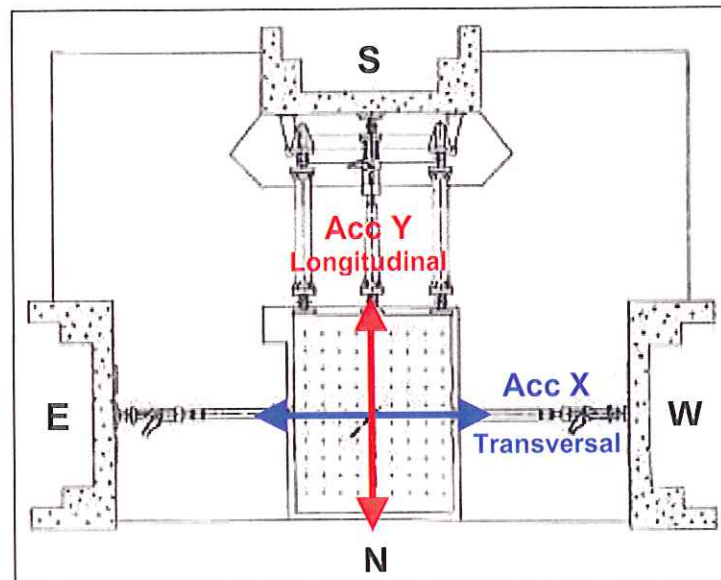


Figure 4-2: Earthquake components.

5 GLOBAL BEHAVIOUR ANALYSIS

5.1 Observed behaviour

During and after the test the following behaviour was observed:

- Beams and columns (away from the beam-column connections)

Away from the connections, the beams sustained a very small amount of visible damage (see Figure F-18, F-19).

The visible damage on the columns was also very small, even in high moment zones, probably due to cantilever design (see Figure F-20, F-33).

- Beam-column connections – Type #1

In this type of connection medium to high damage was observed (see Figure F-18 to F-19 and F-29 to F-32).

In many cases the concrete was crushed on the beams and on the columns (see Figure F-27, F-28).

- Beam-column connections – Type #2

Severe damage was observed and there were visible crushed concrete in all cases (see Figure F-23, F-24).

One large crack appeared on the bottom of the beams, through the vertical bolt hole (see Figure F-21, F-22).

- Cracking

A small amount of cracks, not directly related with the beam-column connection mechanism, were observed and the opening of the cracks was small (see Figure F-31, F-32, F-34)

- Topping layer

Near the columns and in the earlier stages of the tests, the topping layer was crushed and separated (see Figure F-35, F-36). In the rest of the areas only light damage was visible (small cracking).

5.2 Experimental modal frequencies

The modal frequencies were identified using frequency response functions estimations and the peak picking method. Only translational modes were identified because the torsion modes do not have enough amplitude. The lack of amplitude of the torsion modes is due to 2 main reasons: firstly, the structure is symmetric on both horizontal directions, leading to a very close center of mass (CM) and center of stiffness (CS), even after damage; secondly, the stimulus in the *Cat Series* is only of translational type, because the rotation d.o.f. are passively restrained in LNEC's triaxial shaking table.

The identified modal frequencies are presented numerically in Table 5-1 and graphically in Figure 5-1. The identified frequency response functions (FRF) peaks are presented in the Annex G.

Experimental Modal Frequencies (Model) [Hz]				
Series	1st Trans.	2nd Trans.	1st Long.	2nd Long.
Cat 00	2.9	9.0	3.2	14.3
Cat 01	2.3	7.7	2.9	12.6
Cat 02	2.0	6.6	2.7	11.5
Cat 03	1.4	5.5	2.4	9.6
Cat 04	1.2	4.9	1.9	8.4

Table 5-1: Experimental modal frequencies – Model.

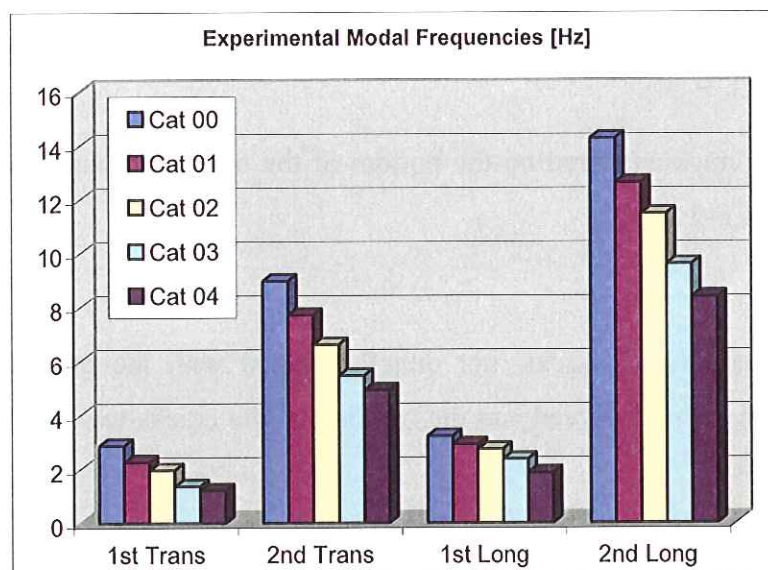


Figure 5-1: Experimental modal frequencies – Model.

A linear dynamic analytical model was used for validating the results of the experimental modal frequencies. These results are presented and compared with the experimental values in Table 5-2.

Modal Frequencies [Hz]	Linear Dynamic Model			Experimental				
	Model #1	Model #2	Model #3	Cat 00	Cat 01	Cat 02	Cat 03	Cat 04
First - Transversal	3.04	1.32	0.93	2.9	2.3	2.0	1.4	1.2
First - Longitudinal	4.01	1.32	0.93	3.2	2.9	2.7	2.4	1.9
First - Torsion	5.65	2.88	2.47	-	-	-	-	-
Second - Transversal	10.39	8.61	6.12	9.0	7.7	6.6	5.5	4.9
Second - Longitudinal	11.59	8.61	6.12	14.3	12.6	11.5	9.6	8.4
Second - Torsion	16.67	13.34	9.99	-	-	-	-	-
Beam-column connec.	Rigid	Pinned	Pinned	-	-	-	-	-
Cracking	No	No	50% stiff.	-	-	-	-	-

Table 5-2: Analytical and experimental modal frequencies.

For the first modes, the frequencies of the analytical model with rigid beam-column connections and with uncracked sections (model #1) are close to the initial experimental results (Cat 00). After all test stages, the results for the first modes are similar to the analytical model with pinned beam-column connections and 50% flexure stiffness reduction due to cracking (model #3). In the longitudinal directions the experimental frequencies are greater than expected, mainly due to less damage in the connections leading some rotational stiffness.

To obtain the equivalent modal frequencies in the prototype is necessary to apply the expression (3.7).

5.3 Earthquake scenario

A first analysis to the measured base accelerations gave the PGA values presented in Table 5-3 and in Figure 5-2.

PGA [g]	Trans.	Long.
Stage 00	0.09	0.10
Stage 01	0.27	0.18
Stage 02	0.45	0.29
Stage 03	0.89	0.72
Stage 04	1.26	1.18

Table 5-3: Measured peak ground accelerations.

An extensive list of ground motion parameters is presented in the Annex H.

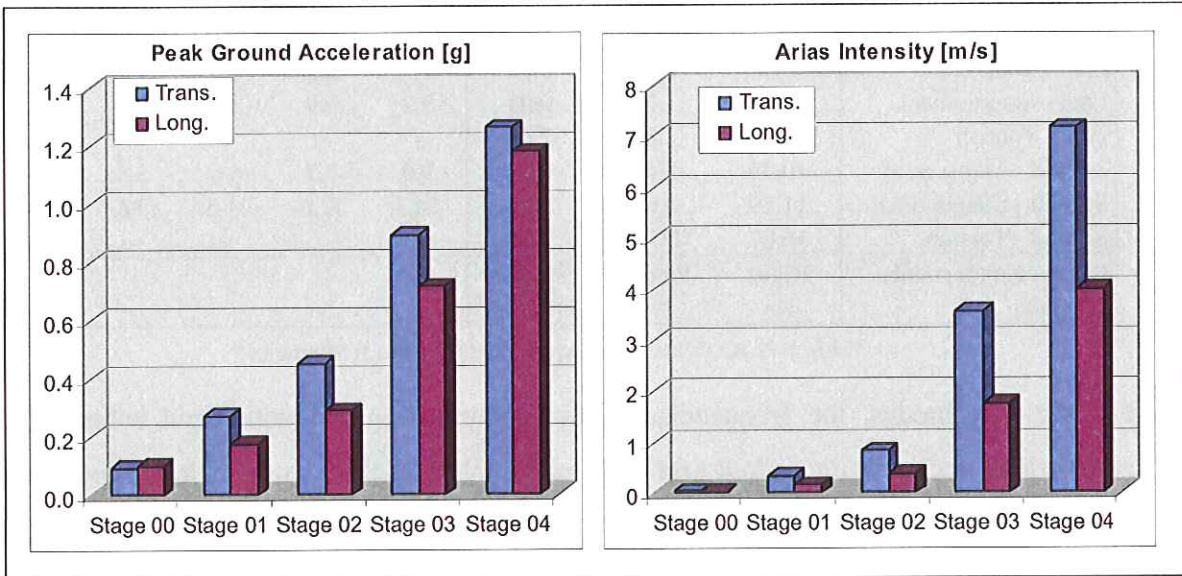


Figure 5-2: Measured PGA and Arias intensity.

Analysing the results more deeply is possible to conclude that the measured values of PGA were influenced by shaking table high frequency, narrow band acceleration peaks (away from the structure principal modal frequencies) and are not the best parameter to quantify the earthquake intensity (see Figure 5-3)

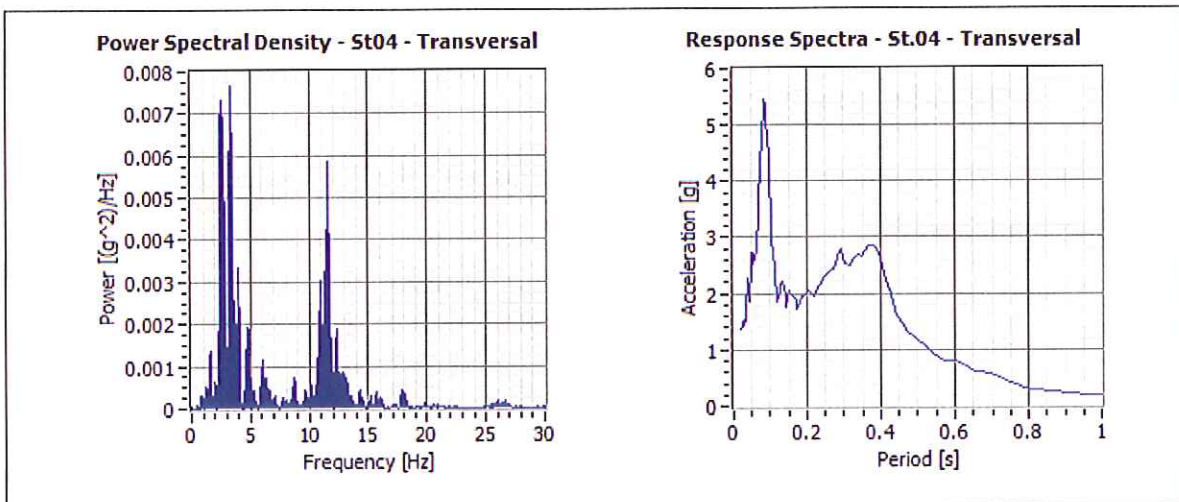


Figure 5-3: Power spectral density and elastic response spectra of typical base accelerations.

To reduce this effect on the PGA values it was used an energy related parameter, *Arias Intensity* (4.1) and the actual target accelerogram,

$$I_a = \frac{\pi}{2g} \int [a(t)]^2 dt . \quad (4.1)$$

The adopted procedure was to use a modified peak ground acceleration (PGA*), which the Arias intensity parameter of the *Tolmezzo* accelerogram corresponds to the same value on the measured series (see Figure 5-4).

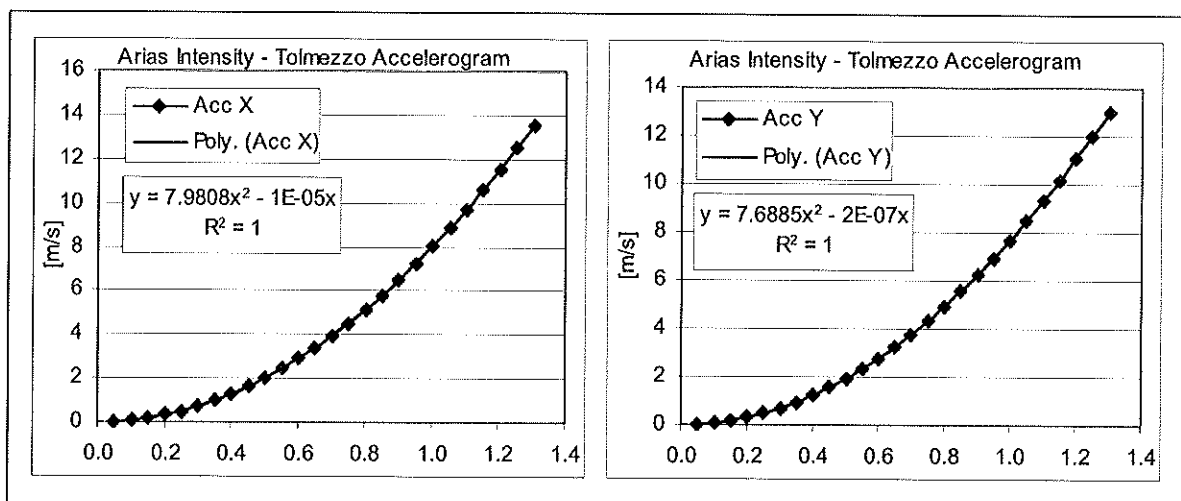


Figure 5-4: Discrete values of the Arias intensity parameter for different PGA values.

The results are presented in Table 5-4. These values revealed some reduction of the PGA values and good correspondence with the target in the response spectra diagrams (see Annex H). The values of the peak ground acceleration (PGA*) were adopted in this report.

PGA* [g]	Trans.	Long.
Stage 00	0.07	0.08
Stage 01	0.21	0.14
Stage 02	0.32	0.23
Stage 03	0.67	0.48
Stage 04	0.95	0.72

Table 5-4: Corrected peak ground accelerations.

Comparing the response spectra of the measured ground acceleration with the target values (see Annex H) revealed good correspondence in the transversal direction (although some differences in the plateau zone and in the higher periods) and only medium quality in longitudinal direction. The principal reason for this problem was due to different specimen-shaking table interaction that the one considered in the signal adaptation, which were performed long-time before and for another test programme.

5.4 Floor diaphragm

For monitoring the deformation of the floors and for testing the viability of considering them as rigid diaphragms, it was measured the deformations of 4 diagonals on the second story floor. Some technical reports indicate that the relative rotations in beam-column connections, typical of some precast structures, leads to beam elongation that can promote relative displacements between elements (see Figure 5-5).

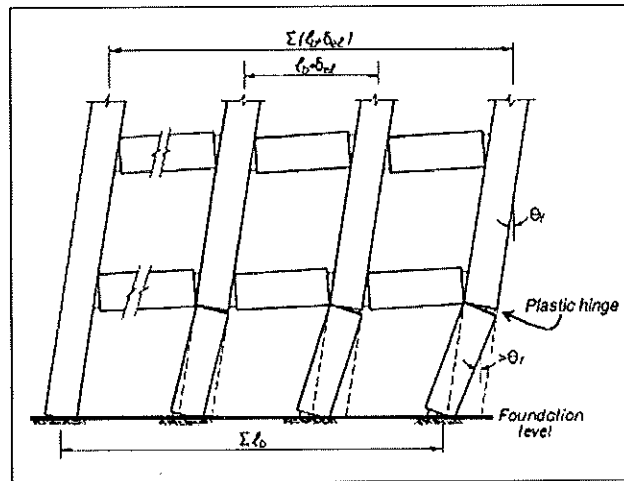


Figure 5-5: Effect of beam elongation (adapted from [FIB 2003]).

The results of the diagonals deformations are presented in Annex I and summarized in Table 5-5.

Deformation	Stage 00	Stage 01	Stage 02	Stage 03	Stage 04
Diagonal RD1	0.00%	0.00%	0.01%	0.04%	0.05%
Diagonal RD2	0.00%	0.00%	0.01%	0.06%	0.09%
Diagonal RD3	0.00%	0.00%	0.01%	0.02%	0.03%
Diagonal RD4	0.00%	0.00%	0.02%	0.07%	0.11%

Table 5-5: Maximum values of the diagonals deformations.

These values revealed very small deformations in the diagonals, less than 0.11% or 4.4 mm in 3.95 m diagonals, and part can be due to the transducers layout.

Taking also in consideration that some codes classify, for design proposes, a diaphragm as flexible when their deformation is more than twice the interstory drift of the associated floor [ICBO 1997], that in our case is about 3% (see section 5.6), so using this criteria the specimen's floors can be consider as rigid and it is feasible to use a 3 d.o.f. model for each.

5.5 Center of mass displacements

The center of mass accelerations, relative displacements and the polar diagrams presented in the Annex J were obtained using a 3 d.o.f. model (2 translations and 1 rotation about the vertical axis in each floor). The maximum values are summarized in Table 5-6 and in Figure 5-6.

Center of Mass	Units	Stage 00	Stage 01	Stage 02	Stage 03	Stage 04
1st Floor - Rel. Disp. - Trans.	[mm]	4.0	8.5	11.2	20.6	23.6
1st Floor - Rel. Disp. - Long.	[mm]	2.4	4.6	6.5	13.6	27.2
1st Floor - Rel. Rotation	[°]	0.02	0.02	0.04	0.06	0.08
2nd Floor - Rel. Disp. - Trans.	[mm]	6.6	16.5	24.1	47.1	59.0
2nd Floor - Rel. Disp. - Long.	[mm]	2.5	6.6	10.6	23.9	48.1
2nd Floor - Rel. Rotation	[°]	0.00	0.00	0.00	0.01	0.01

Table 5-6: Center of mass maximum relative displacements and rotations.

The displacements were higher in the transversal direction (see Figure 5-6) but in the last stage the longitudinal displacements increased more, probably due to the development of damage. As expected, the floors rotations are very small because of the biaxial symmetry of the structure.

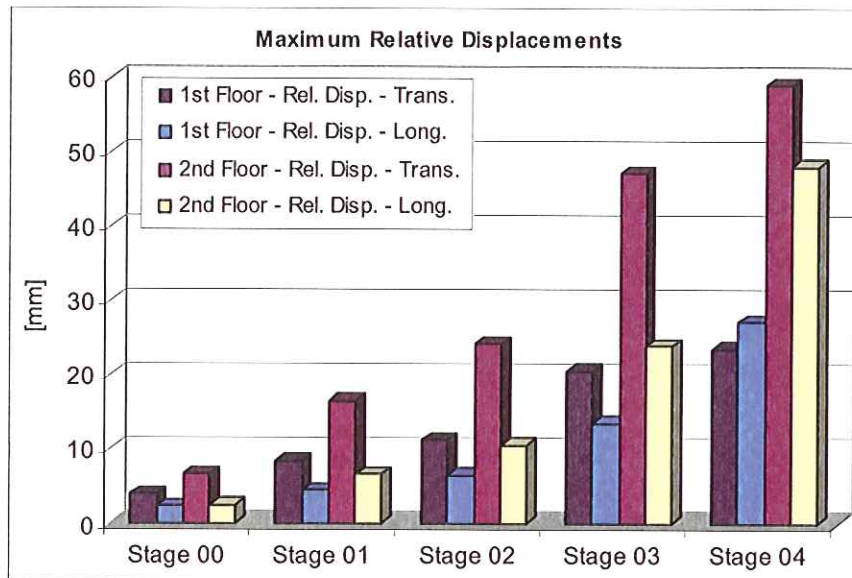


Figure 5-6: Center of mass maximum relative displacements.

5.6 Interstorey drifts

In Table 5-7 is presented the maximum values of the interstorey drifts obtained on the conner columns and on the center of mass of each storey. The time-histories are presented in the Annex K.

The maximum values of the center of mass interstorey drifts were about 0.3% in stage 00, 0.6% in stage 01, 0.9% in stage 02, 2.1% in stage 03 and 2.9% in stage 04. Some columns achieved higher values (e.g. 3.4% on the second floor NW column) as the CM values tends to be averaged due to incipient torsion.

Position		Stage 00		Stage 01		Stage 02		Stage 03		Stage 04	
		Trans.	Long.	Trans.	Long.	Trans.	Long.	Trans.	Long.	Trans.	Long.
Columns	1 st F. NW	0.3%	0.2%	0.6%	0.3%	0.8%	0.5%	1.5%	1.0%	1.7%	1.9%
	1 st F. SE	0.2%	0.1%	0.6%	0.2%	0.7%	0.4%	1.3%	0.9%	1.3%	1.9%
	1 st F. NE	0.2%	0.1%	0.4%	0.1%	0.5%	0.2%	0.9%	0.6%	0.8%	0.6%
	1 st F. SW	0.2%	0.1%	0.5%	0.1%	0.7%	0.2%	1.2%	0.5%	1.4%	0.8%
	2 nd F. NW	0.3%	0.1%	0.8%	0.3%	1.2%	0.6%	2.7%	1.2%	3.4%	2.6%
	2 nd F. SE	0.2%	0.2%	0.5%	0.2%	0.9%	0.3%	2.1%	0.9%	2.9%	1.4%
	2 nd F. NE	0.2%	0.1%	0.6%	0.2%	1.0%	0.4%	2.1%	0.9%	2.7%	1.7%
	2 nd F. SW	0.2%	0.2%	0.7%	0.3%	1.1%	0.5%	2.2%	1.1%	2.8%	2.6%
CM	1 st F. CM	0.3%	0.2%	0.6%	0.3%	0.7%	0.4%	1.4%	0.9%	1.6%	1.8%
	2 nd F. CM	0.2%	0.1%	0.6%	0.2%	0.9%	0.4%	2.1%	0.9%	2.9%	1.4%

Table 5-7: Maximum interstorey drifts.

The maximum values were achieved on the second floor, mainly because in the final test stages the columns behaved more as cantilevers than as part of a frame system. In the early stages, the maximum interstorey drifts were obtained on the first floor (frame effect).

The graphical representation of the center of mass interstorey drifts versus earthquake input (PGA^*) presented in Figure 5-7, shows an almost linear relation with some slope increasing due to damage. The only visible exception is on stage 04, in the transversal direction, that seems to “regain stiffness”. In this direction the damage of the structure shifted its dynamic behaviour to a frequency band with lower energy contents, so the large increase in the PGA^* value is not representative of the real earthquake input (see section 5.7). Another explanation could be that the interstorey drift is near the maximum physical value, leading to a shape typical of a fragility curve.

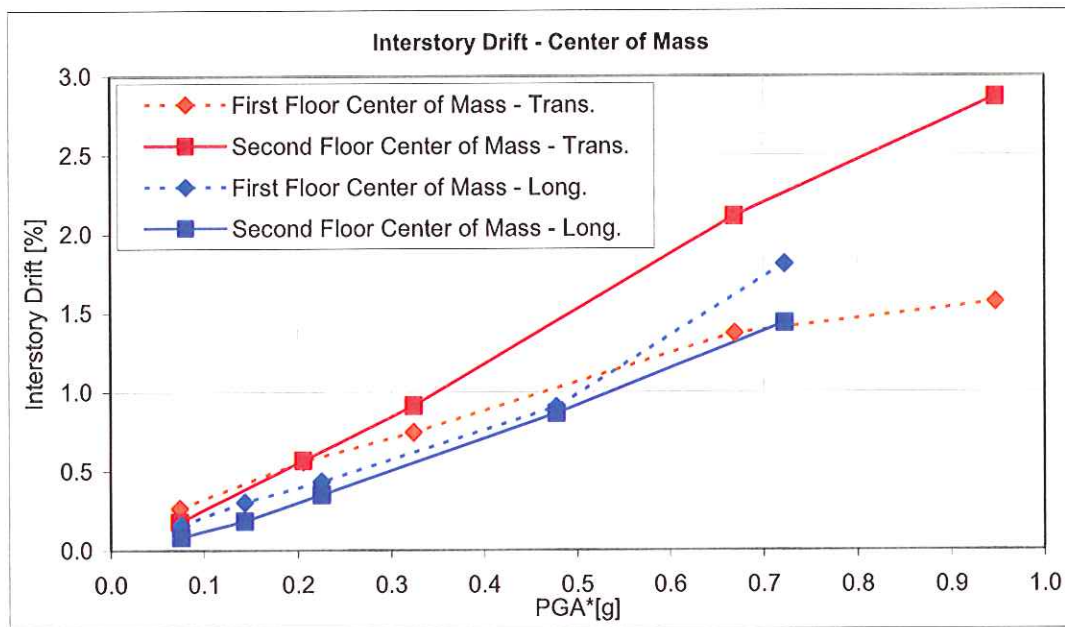


Figure 5-7: Center of mass interstorey drifts.

5.7 Global forces

For estimating the global forces developed on the specimen it was used a simplified model that assumes that the inertia forces are equal to the restoring forces (neglecting damping forces):

$$\begin{cases} \{F_i\} + \{F_d\} + \{F_k\} = 0 \\ \{F_d\} \approx 0 \end{cases} \Rightarrow \{F_i\} \approx -\{F_k\}. \quad (4.2)$$

This model gives only approximated values, with more accuracy when the velocities are small (viscous damping), but enables to compute forces and moments using only the measured cinematic values.

The inertia and torque forces (IF, IT), the story inertia and torque forces (SF, ST), the base shear (BS) and the base overturning moment (BOM) were computed as defined in Figure 5-8.

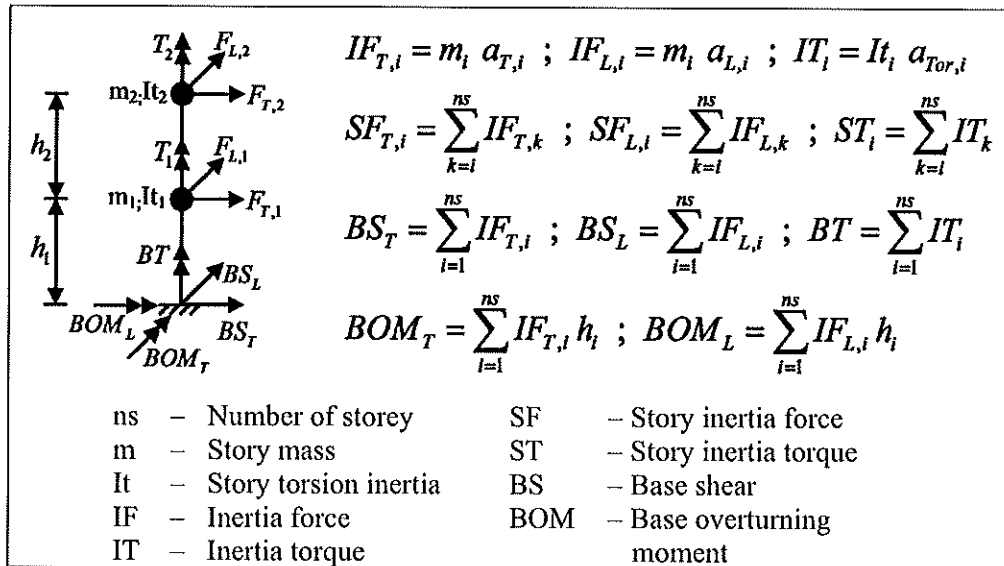


Figure 5-8: Simplified global model.

The global forces time-histories are presented in the Annex L. The maximum values are summarized in Table 5-8.

	Units	Stage 00	Stage 01	Stage 02	Stage 03	Stage 04
Base Shear - Trans.	kN	35.7	60.8	61.8	78.0	76.7
Base Shear - Long.	kN	22.1	39.7	51.3	84.0	128.5
Base Torque	kN.m	0.03	0.10	0.19	0.42	0.54
Base Overt. Mom. - Trans.	kN.m	82.4	139.9	163.6	192.9	139.1
Base Overt. Mom. - Long.	kN.m	50.0	94.2	122.5	216.3	296.0

Table 5-8: Maximum values of the global forces.

The maximum values of base shear are initially higher in the transversal direction, but in the last 2 stages the values in the longitudinal direction increased significantly (see Figure 5-9). This behaviour is due to less earthquake input in the transversal direction in Stage 03 and 04, although the PGA values are always greater. These results are expected because of the changes in the dynamic characteristics of the structure to an input frequency band with lower energy contents (see Figure 5-10).

Very small values of the base torque were obtained.

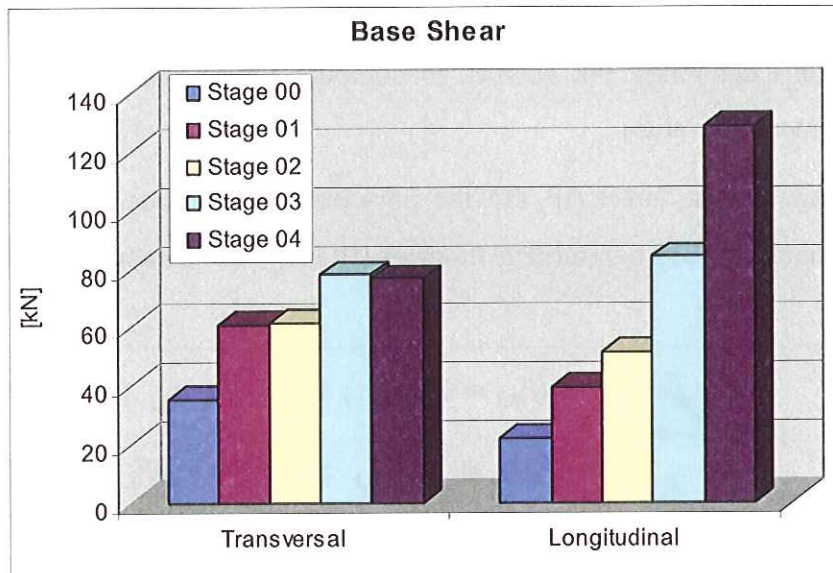


Figure 5-9: Base shear maximum values.

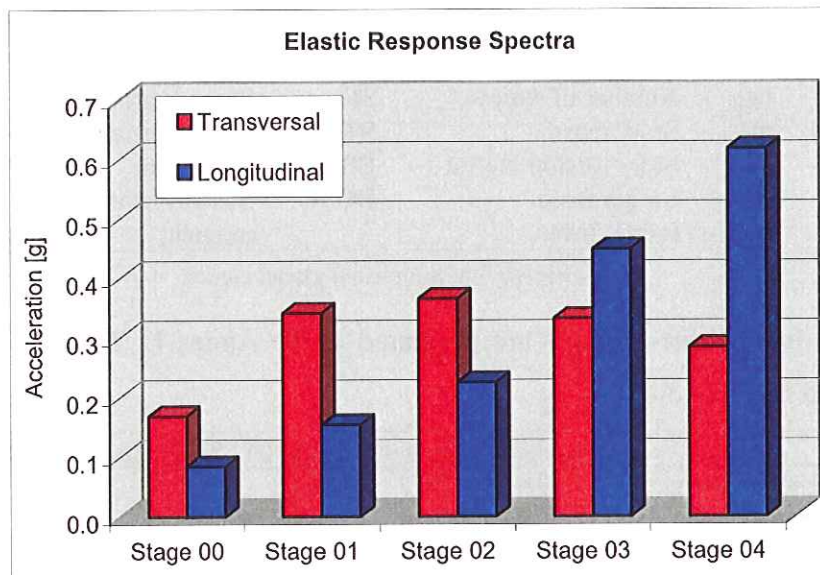


Figure 5-10: Elastic response spectra values for the fundamental modal frequency.

Taking in consideration that the specimen weighted 254 kN (see Figure 3-1) the values of the ratio of the maximum base shear vs. weight of the specimen are presented in Table 5-9. We conclude that the sum of the calculated inertia forces reached 31% of the specimen weight in the transversal direction and 51% in the longitudinal direction.

Stage 00	Stage 01	Stage 02	Stage 03	Stage 04
0.14	0.24	0.24	0.31	0.30
0.09	0.16	0.20	0.33	0.51

Table 5-9: Values of the maximum base shear vs. weight of the specimen ratio.

5.8 Hysteresis loops

The simplified model used for the global forces (Figure 5-8) allows computing the global and the storey hysteresis loops presented in the Annex M.

In the global hysteresis loops (base shear vs. top relative displacement) is possible to observe the following behaviour:

- In Stage 00, the results are strongly influenced by high noise/signal ratio and have no physical meaning;
- In Stage 01 is possible to observe non-linear behaviour in the transversal direction, with some energy dissipation, and in the other direction an almost elastic response;
- In Stage 02, the global behaviour is similar to the previous stage, with increasing amplitude in the cycles, decreasing of the stiffness and some pinching effect in the transversal direction. In the other direction the non-linear behaviour is still not evident;
- In Stage 03, the stiffness of the structure in the transversal direction is very small and the diagrams exhibit the presence of higher mode contributions. On the longitudinal direction non-linear phenomena appears more visible;
- In Stage 04, the transversal direction shows only residual stiffness and in the longitudinal direction the diagram shows a highly non-linear behaviour with a clear pinching effect, due to the opening and closing of beam-column connections;

In the storey hysteresis loops (interstorey shear vs interstorey drift), also presented in the Annex M, is possible to observe the same behaviour, confirming the results obtained in the global hysteresis loops.

6 BEAM-COLUMN CONNECTIONS

As indicated in section 4.1, the displacements of 8 beam-column connections were measured using displacement transducers in the top and bottom faces of the beams (see drawing nº 2 of the Annex E). These displacements were also converted to global opening and relative rotation as indicated in the Annex N. In the same annex is also presented the time-histories of the displacements, with all stages joined in one diagram to allow a more clear view of the displacements' progression. The maximum values are presented in Table 6-1 and in Figure 6-1.

Joint	Top opening (max-final)		Bottom opening (max-final)		Global relative disp. (max-final)		Global relative rotation (max-final)			
	[mm]		[mm]		[mm]		[rad]		[°]	
Joint 3 - Trans.	5.0	2.3	5.9	2.4	4.8	2.4	0.018	0.000	1.0	0.0
Joint 9 - Trans.	2.5	1.2	8.4	5.3	4.8	3.3	0.036	0.021	2.1	1.2
Joint 3 - Long.	1.4	0.0	4.7	0.5	3.0	0.2	0.021	0.003	1.2	0.1
Joint 9 - Long.	0.6	0.4	3.6	0.4	1.7	0.0	0.022	0.005	1.3	0.3
Joint 2 - NE	0.5	0.0	0.5	0.2	0.3	0.1	0.004	0.001	0.2	0.0
Joint 2 - SE	0.4	0.0	0.7	0.1	0.3	0.1	0.005	0.001	0.3	0.0
Joint 8 - NE	0.9	0.0	0.7	0.3	0.4	0.2	0.006	0.001	0.3	0.1
Joint 8 - SE	0.9	0.0	0.5	0.2	0.3	0.1	0.007	0.001	0.4	0.1

Table 6-1: Beam-column connections maximum displacements and rotations.

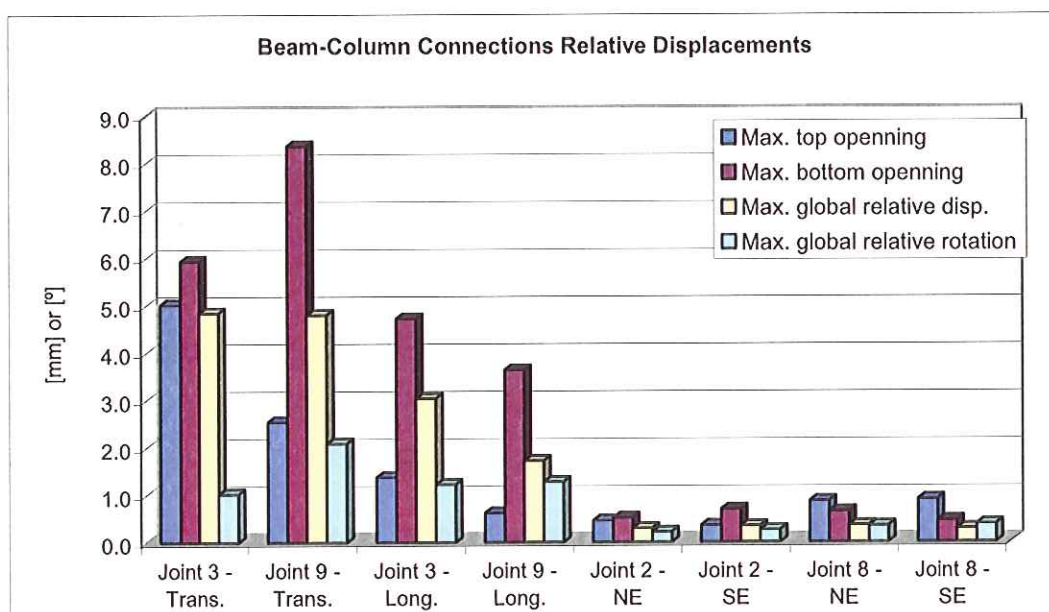


Figure 6-1: Beam-column connections maximum displacements and rotations.

For obtaining the equivalent displacements in the prototype the values should be multiply by 3 (scale factor), except for the rotations that are dimensionless.

Analysing the results is possible to observe that:

- Smaller displacements were measured in the connections of inner columns (joints 2 and 8, see Figure 6-1);
- The openings at the bottom of the beams (8.4 mm) were greater than at the top (5.0 mm);
- The maximum final opening is also in the bottom of the beams (5.3mm);
- The maximum global relative displacement was of 4.8 mm, with a permanent value of 3.3 mm at the end of the test;

- The maximum relative rotation was of 2.1°, with a permanent value of 1.2° at the end of the test;
- In the transversal direction the openings and relative displacements were larger (beam-column connection type #2).

7 CONCLUSIONS

The tested precast structure, designed by *Civibrat* for $PGA=0.375g$ using $q=2.0$, was subjected in the LNEC's triaxial shaking table to 5 earthquake motions with increasing intensity. The results of the tests made possible to establish the following conclusions:

1. Observed Behaviour:

- The majority of the visible damage was concentrated on the beam-column connections;
- Away from the connections the elements sustained very few damage;
- The presence of torsion was very small (Center of Mass \approx Center of Stiffness).

2. Global Behaviour:

- The first modal frequencies were about 2.9 Hz (Trans.), 3.2 Hz (Long.) before the tests, and 1.2 Hz (Trans.), 1.9 Hz (Long.) after all stages, so the specimen suffered a very intense stiffness degradation (around 80% reduction in the transversal direction);
- The slabs had enough axial stiffness to be considered as a rigid diaphragm in its plane;
- The interstory drifts reached very high values (around 3% in the last stage) so the global collapse could be eminent;
- The estimated base shear reached also very high values (more than 50% the weight of the specimen);
- Although, higher values of PGA^* were in the transversal direction, the first stages damaged heavily the structure in that direction, shifting the dynamic characteristics of the specimen to frequencies with lower energy contents;

- The global hysteresis loops showed in Stages 01 and 02 an almost elastic response in the longitudinal direction and a clear non-linear behaviour in the transversal direction. In the following stages the non-linear behaviour appeared in the longitudinal direction and the transversal direction showed signs of residual stiffness.

3. Beam-column connections:

- On both connections types the bottom faces of the beams showed higher flexibility. The steel plate and bolts of the top face had a better response (higher stiffness and ductility);
- Comparatively, the type #2 connection had a worse performance (greater damage and higher stiffness degradation), although in that direction the stresses were higher due to structural configuration (only one bay);
- On type #2 connections the vertical bolt lost its contribution when a large crack appeared through the bolt hole (see Figure 4-c) leading to almost free bottom relative displacements;
- To understand the behaviour of these connections a more detailed study is required (i.e. cyclic tests).

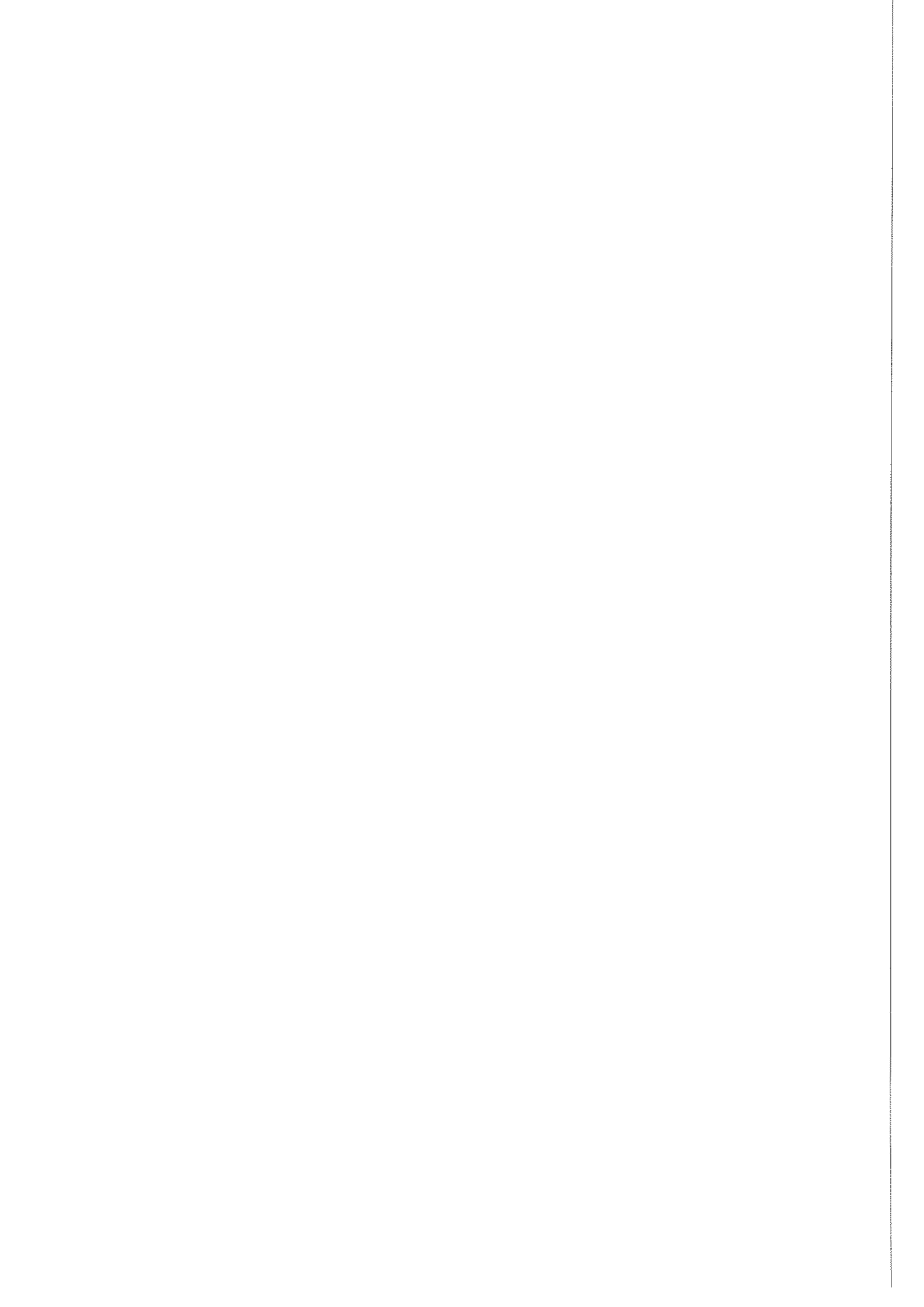
4. Performance of the structure:

- In the less intense test stages the beam-column connections revealed non-neglectable rotational stiffness and the overall behaviour was typical of a frame structure. At the final stages the overall behaviour has governed by cantilever columns (inverted pendulum response);
- The beam-column connections revealed enough ductility to sustain the earthquake input without collapsing;
- To design this structure for collapse prevention a conservative and real model is to consider the beam-column connections as pinned;
- The most probable collapse mechanisms identified were the local failure of the beam-column connections (in particular type #2 connections) and the global failure of the second floor columns (because of higher interstory drifts and the good response of the column's bases);

- Taking into consideration the performance of the structure presented in this paper, for the usual safety requirements (collapse prevention) it is expected that this structure should be able to receive a more intense seismic action than the one considered for design purposes ($PGA=0.375g$);
- To determine the safety margin against collapse prevention and damage limitation, further analysis should be carried out taking into account these results and the seismic hazard of the region where the structure will be constructed.

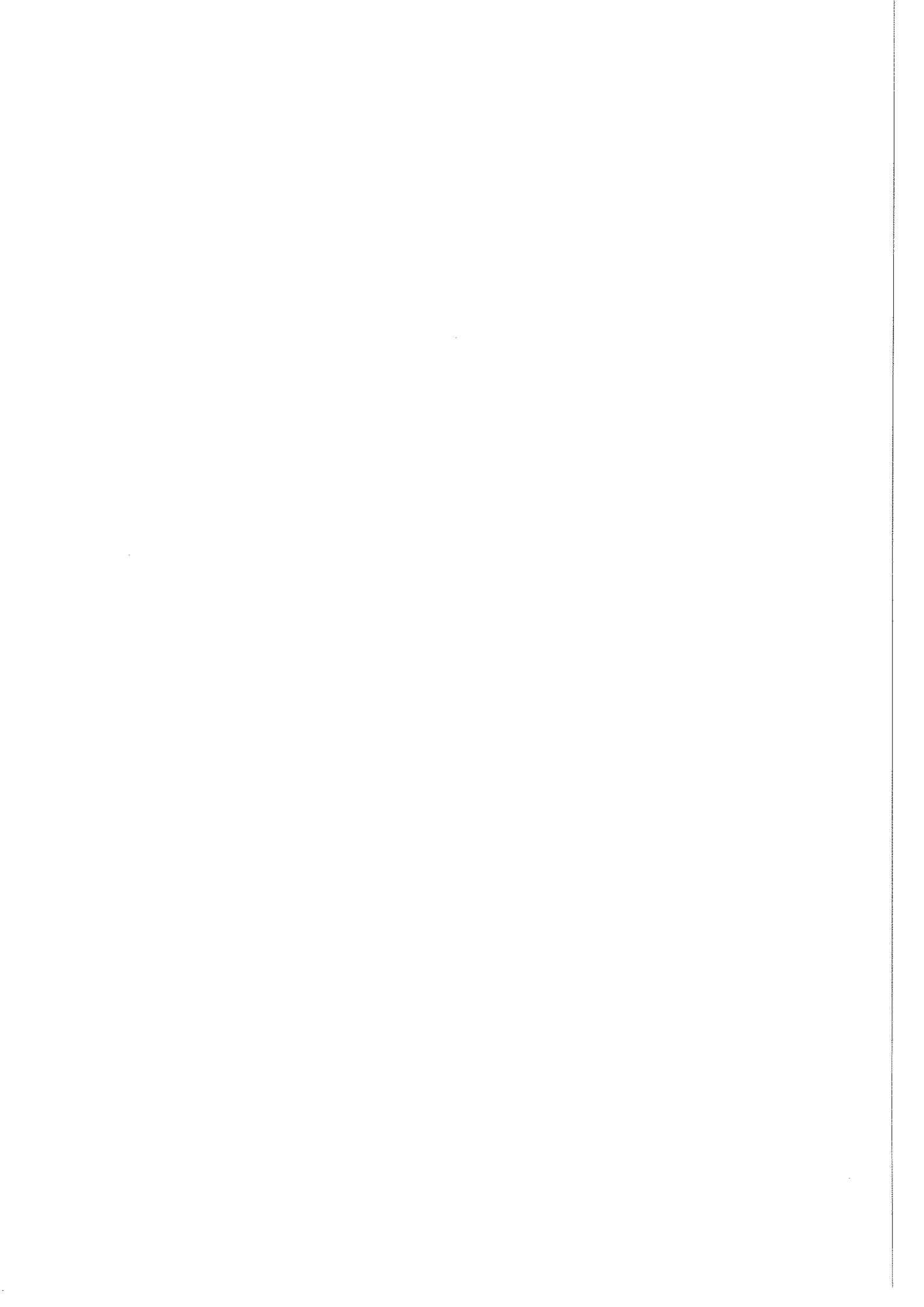
8 ACKNOWLEDGMENTS

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Lisbon, National Laboratory of Civil Engineering, May of 2006,

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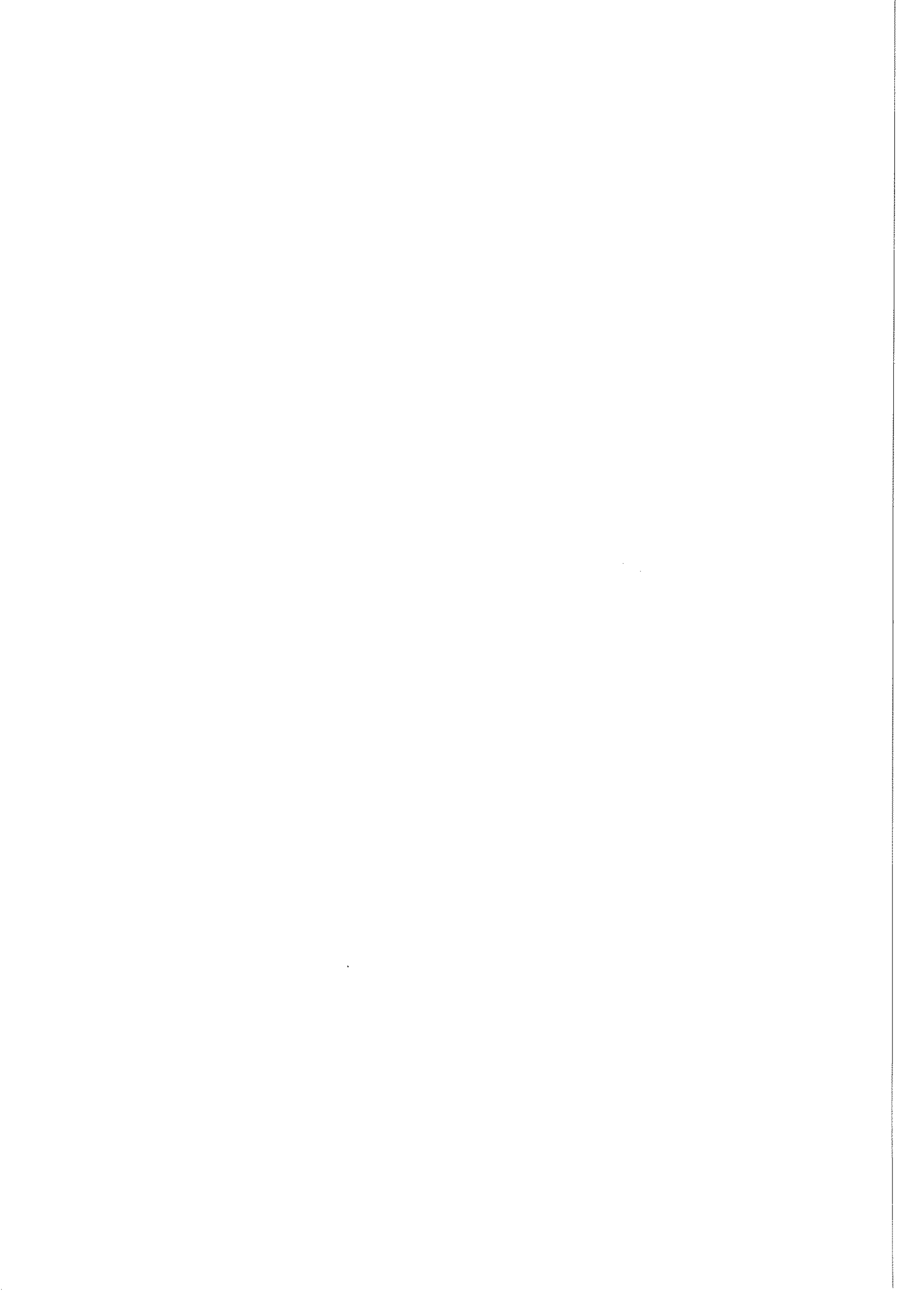
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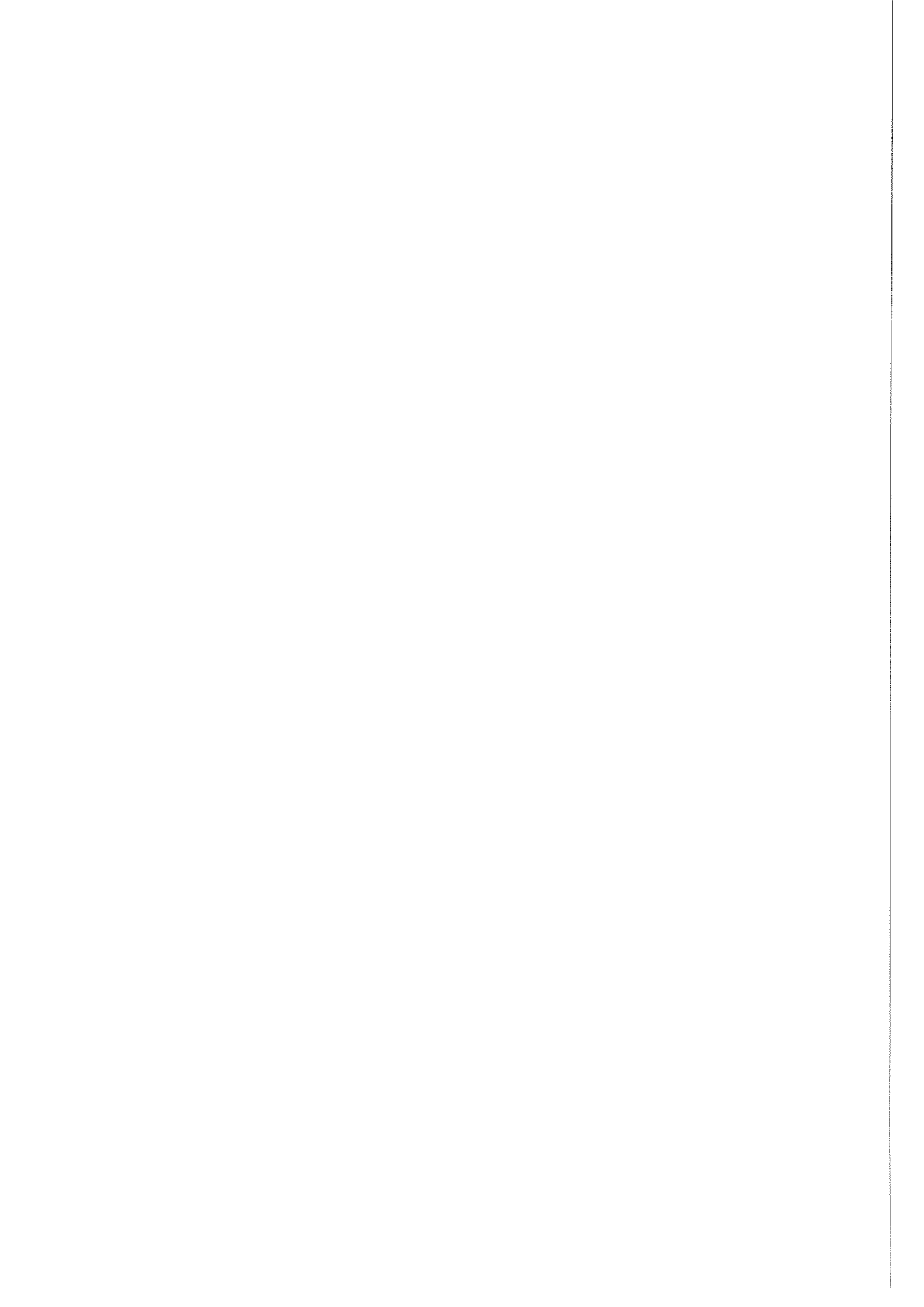
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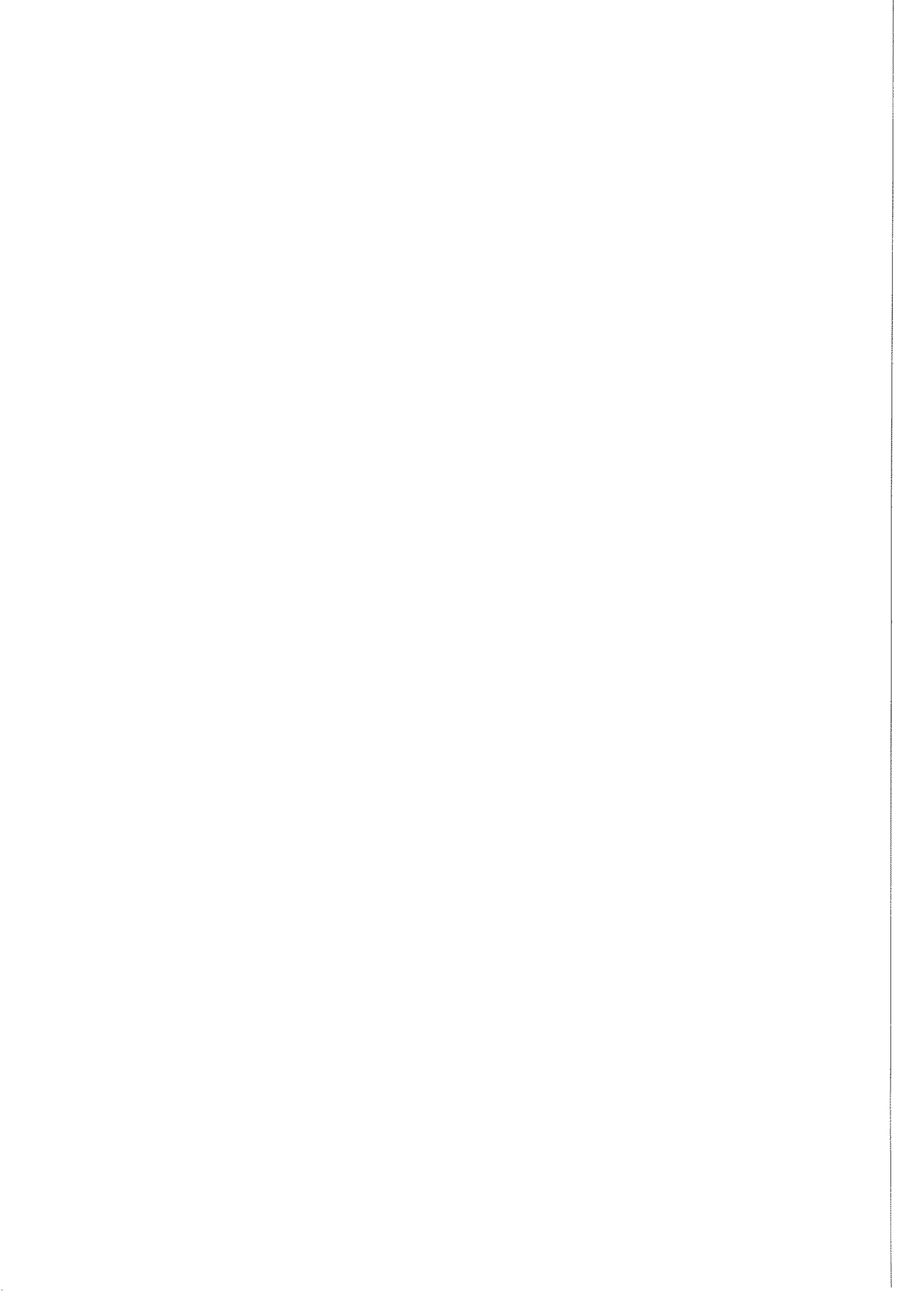
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ANNEX A DESIGN REPORT FROM CIVIBRAL





MEMÓRIA DESCRITIVA

1. INTRODUÇÃO

Refere-se a presente memória descritiva ao dimensionamento de uma estrutura pré-fabricada tipo CIVIBRAL.

A solução adoptada consiste essencialmente na utilização de elementos pré-fabricados de betão armado e pré-esforçado, excepto as fundações que são realizadas “*in situ*”, a camada de solidarização das lajes e selagens pontuais.

Os elementos estruturais a utilizar fazem parte de um conjunto de soluções que se encontram optimizadas pela empresa de pré-fabricação CIVIBRAL e que foram objecto de estudos detalhados.

2. CONCEPÇÃO ESTRUTURAL

2.1. Tipo de estrutura

Adoptou-se uma solução pré-fabricada em betão armado e pré-esforçado do tipo CIVIBRAL. No que se refere às fundações e face às características da construção estas são directas, constituídas por sapatas isoladas.

Procurou-se adoptar recobrimentos das armaduras que conduzissem a uma boa durabilidade da estrutura, tendo-se adoptado como recobrimento 2.5cm para vigas e pilares e 5cm para sapatas.

Assim, a estrutura será constituída por pilares e vigas executadas em fábrica betão pré-esforçado solidarizadas entre si “*in-situ*”.

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Estruturalmente a estrutura apresenta ligações rotuladas entre as vigas dos pisos e os pilares, admitindo-se assim um funcionamento das vigas como bielas, que compatibilizam os deslocamentos dos pilares, mas sem qualquer transmissão de momentos. As ligações entre pilares e vigas são igualmente rotuladas. Inferiormente os pilares encontram-se perfeitamente encastrados em sapatas de fundação.

2.2. Ligações

A ligação pilar/sapata é realizada introduzindo a extremidade inferior do pilar numa cavidade existente na fundação. O preenchimento do espaço remanescente da cavidade é realizado com uma argamassa. A eficácia desta ligação é garantida pela resistência da argamassa de selagem, da sua aderência às superfícies do pilar e do comprimento do pilar embebido na fundação. É de notar que a resistência da referida argamassa deverá pelo menos ser igual à resistência mínima dos dois elementos a ligar.

3. CRITÉRIOS GERAIS DE SEGURANÇA E DE DIMENSIONAMENTO

3.1. Acções

A quantificação das acções foi efectuada a partir dos seus valores característicos, conforme descritos no R.S.A.;

3.1.1. Acções permanentes

As acções permanentes são aquelas que, assumem valores constantes ou com pequena variação em torno do seu valor médio, consistem nos pesos próprios dos elementos estruturais e não estruturais, os pesos dos equipamentos fixos, os impulsos das terras e os pré-esforços.

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Relativamente à estrutura em estudo as acções permanentes resultam do peso próprio da estrutura, revestimentos, efectuando-se a distinção entre cargas mobilizadas à data da aplicação do pré-esforço e as que apenas posteriormente irão ser aplicadas.

Assim teremos:

Piso:

- peso próprio dos elementos estruturais – 25.0 kN/m^3 (vigas, pilares, lajes...)
- camada de compressão – 5cm

3.1.2. Acções variáveis

As acções variáveis são aquelas que assumem valores com variação significativa em torno do seu valor médio durante a vida da estrutura. Consideram-se como acções variáveis, para o presente caso, as sobrecargas e a acção do sismo. Em seguida são apresentados os valores característicos de cada acção variável.

3.1.2.1. Sobrecargas

De acordo com o especificado, foram consideradas as seguintes sobrecargas uniformemente distribuídas:

- 5.0 kN/m^2 nos pisos

Os valores reduzidos são obtidos através dos seguintes coeficientes:

- $\psi_0 = 0,7$; $\psi_1 = 0,6$; $\psi_2 = 0,4$



3.1.2.2. Acção do sismo

Os acelerogramas utilizados foram os de Tolmezzo, corrigidos por forma a corresponderem ao espectro elástico de resposta do EC8 para solos do tipo 1, e do tipo B, escalados para $PGA=1g$.

De acordo com as zonas sísmicas e a intensidade da acção sísmica em território nacional, a estrutura foi dimensionada para $0.25g$, que seria o equivalente a considerar de acordo com o RSA um coeficiente de sismicidade (α) igual a 0.85 .

Os acelerogramas foram aplicados à estrutura nas direcções correspondentes, considerando uma análise incremental por passos (Time History Analysis). A análise foi do tipo linear e o “time history analysis” foi realizado através do método de sobreposição modal, considerando apenas os seis primeiros modos de vibração.

A combinação da acção sísmica horizontal em ambas as direcções, foi efectuada de acordo com o especificado no EC8:

$$Ex + 0.3Ey$$

$$0.3Ex + Ey$$

3.1.3. Combinação de acções

A verificação da segurança da estrutura é feita quer em relação aos diversos estados limites últimos, quer em relação aos estados limites de utilização (deformação e fendilhação), para as combinações de acções adequadas e definidas de acordo com o R.S.A. e o R.E.B.A.P..



A dimensão dos elementos estruturais (lajes, vigas, pilares e fundações) assim quantidade e disposição das armaduras resistentes, são as necessárias para garantir a segurança aos esforços mais gravosos, quer em estado limite último, quer em estado limite de utilização.

A obtenção destes e quaisquer outros esforços nos elementos estruturais é feita com o recurso aos métodos de cálculo mais actuais e recorrendo à utilização de programas de cálculo automático. Estes métodos permitem a obtenção dos esforços para as combinações de acções consideradas, assim como o dimensionamento das armaduras (passivas e de pré-esforço) segundo as regras e os critérios definidos regulamentarmente.

A verificação aos estados limites foi feita em termos de esforços, com base na condição,

$$S_d \leq R_d$$

em que S_d e R_d designam respectivamente os valores de cálculo do esforço actuante e do esforço resistente.

Consideram-se as seguintes combinações fundamentais:

Em geral:

$$S_d = \sum \gamma_{gi} S_{Gik} + \gamma_q (S_{Q1k} + \sum \psi_{0j} S_{Qjk})$$

em que

S_{Gik} – esforço resultante de uma acção permanente, tomada com o seu valor característico ;

S_{Q1k} – esforço resultante da acção variável considerada como acção de base da combinação, tomada com o seu valor característico (S_{Ek} no caso da acção sísmica) ;

S_{Qjk} – esforço resultante da acção variável distinta da acção de base, tomada com o seu valor característico;

γ_{gi} – coeficiente de segurança relativo às acções permanentes;

γ_q – coeficiente de segurança relativo às acções variáveis;

$\psi_{0j} \psi_{2j}$ – coeficiente ψ correspondente à acção variável de ordem j ;

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Os coeficientes de segurança γ_{gi} e γ_q considerados, respectivamente para as acções permanentes e variáveis são os seguintes:

- peso próprio da estrutura $\gamma_{gi} = 1.35$ ou 1.0 (conforme mais desfavorável)
- restantes cargas permanentes $\gamma_{g2} = 1.35$ ou 1.0 (conforme mais desfavorável)
- acções variáveis $\gamma_q = 1.50$, para todas as acções variáveis

4. Materiais

Os materiais utilizados apresentam as características constantes do R.E.B.A.P. e R.E.A.E., nomeadamente em termos de valores característicos e de cálculo de tensões de rotura do betão à compressão referidos a provetes cilíndricos (respectivamente f_{ck} e f_{cd}) e valores de cálculo de tensões de cedência ou de tensões limite de proporcionalidade a 0,2% em tracção do aço das armaduras ordinárias (f_{syd}) ou valores característicos da tensão de rotura em tracção do aço das armaduras de pré-esforço (f_{puk}).

Os materiais mais utilizados apresentam os seguintes valores de tensões:

Betão armado e pré-esforçado:	Betão C35/45: $f_{ck} = 35$ Mpa $f_{cd} = 23.3$ MPa
Fundações:	Betão C20/25: $f_{ck} = 20$ Mpa $f_{cd} = 13.3$ MPa
Pilares, vigas de travação e enchimento:	Betão C25/30: $f_{ck} = 25$ Mpa $f_{cd} = 16.7$ MPa

Armaduras:

Armaduras passivas A500 NR: $f_{syd} = 435$ MPa

Armaduras de pré-esforço: $f_{puk} = 1860$ Mpa

Perfis e Chapas Metálicas: Fe360





5. Processos de Cálculo

Relativamente aos elementos em betão armado pré-esforçado, procura-se estabelecer um compromisso entre os estados limites de utilização (deformação e fendilhação) e os estados limites últimos de resistência. São efectuadas as verificações de tensões para as diversas combinações de acções de serviço (quase permanentes, frequentes e raras) e é efectuada a verificação de segurança em relação ao estado limite último de resistência por comparação dos esforços solicitantes com os esforços resistentes.

O estudo dos pilares obedeceu a critérios de 1ª e 2ª ordem, uma vez que devido à sua altura e devido às ligações com os restantes elementos estruturais, o efeito de instabilidade por causa do fenómeno de encurvadura pode revelar-se importante.

Os efeitos de 2ª ordem foram contabilizados através das excentricidades adicionais, mais nomeadamente, a excentricidade accidental, a excentricidade de 2ª ordem e a excentricidade de fluência, tendo-se para o efeito respeitado todas as disposições regulamentares constantes nos artigos 61º a 64º do R.E.B.A.P..

No que diz respeito aos elementos metálicos, é efectuada a verificação de segurança em relação ao estado limite último de resistência sem plastificação, por comparação dos valores de cálculo das tensões actuantes com os valores de cálculo das tensões resistentes, conforme o estipulado no artigo 41º do R.E.A.E..

O cálculo dos diversos elementos estruturais é efectuado com o recurso a programas de cálculo automático.

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A título exemplificativo podem referir-se os seguintes:

- CÁLCULO DE ESTRUTURAS RETICULADAS PLANAS: Análise de elementos porticados, utilizado o programa global dos elementos estruturais principais (pilares e vigas).
- CÁLCULO DE VIGAS CONTÍNUAS: Análise de vigas contínuas com indicação de esforços e de armaduras.
- CÁLCULO DE MOMENTOS RESISTENTES: Estudo de secções de com uma determinada geometria (com eixo vertical de simetria), apresentando as características geométricas da secção e os esforços resistentes de cálculo.

6. Bibliografia

Da bibliografia utilizada para a execução do presente projecto destacam-se os seguintes elementos:

- “RSA - Regulamento de Segurança e Acções para Estruturas de Edif. e Pontes”
- “REBAP - Regulamento de Estruturas de Betão Armado e Pré-esforçado”
- “EC2 – Eurocódigo 2 - Estruturas de Betão Armado e Pré-esforçado”
- “PCI Design Handbook – Precast and Prestressed Concrete”
- “Construções de Concreto, Volume 3” – Leonhardt
- “EC8 – Eurocódigo 8 – Dimensionamento Sísmico de Estruturas”

Maia, 30 de Agosto de 2004



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ANEXOS



CONCEPÇÃO, PRÉ-FABRICAÇÃO
E MONTAGEM DE PRODUTOS EM
BETÃO ARMADO E PRÉ-ESFORÇADO



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Elementos de cálculo

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DELEG.: Av. Engº Arantes Oliveira, 5 1ºB - 1990-221 LISBOA – lisboa@civibral.pt Telf: 218 471 061 Fax: 218 471 059



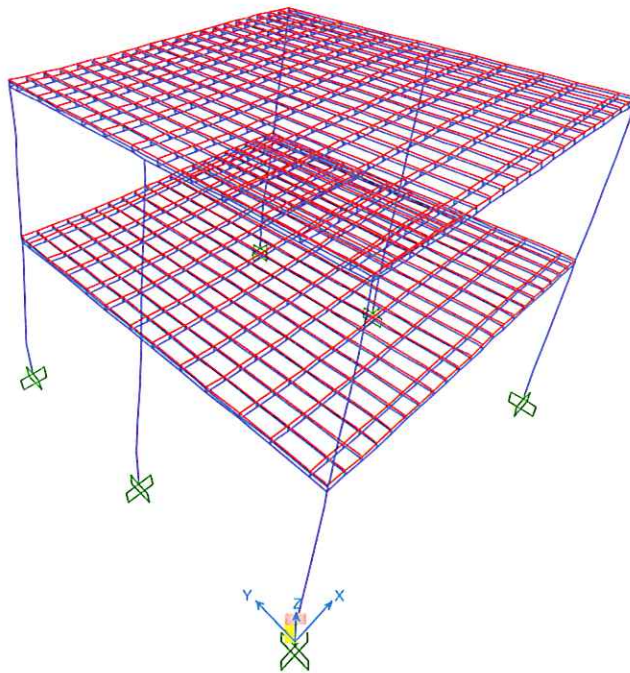
CONCEPÇÃO, PRÉ-FABRICAÇÃO
E MONTAGEM DE PRODUTOS EM
BETÃO ARMADO E PRÉ-ESFORÇADO

Períodos e frequências de vibração

OutputCase Text	StepType Text	StepNum Unitless	Period Sec	Frequency Cyc/sec	CircFreq rad/sec	Eigenvalue rad2/sec2
MODAL	Mode	1.000000	0.901381	1.1094E+00	6.9706E+00	4.8590E+01
MODAL	Mode	2.000000	0.837751	1.1937E+00	7.5001E+00	5.6251E+01
MODAL	Mode	3.000000	0.501775	1.9929E+00	1.2522E+01	1.5680E+02
MODAL	Mode	4.000000	0.274818	3.6388E+00	2.2863E+01	5.2272E+02
MODAL	Mode	5.000000	0.268081	3.7302E+00	2.3438E+01	5.4932E+02
MODAL	Mode	6.000000	0.259055	3.8602E+00	2.4254E+01	5.8827E+02

Modos de vibração

1º modo

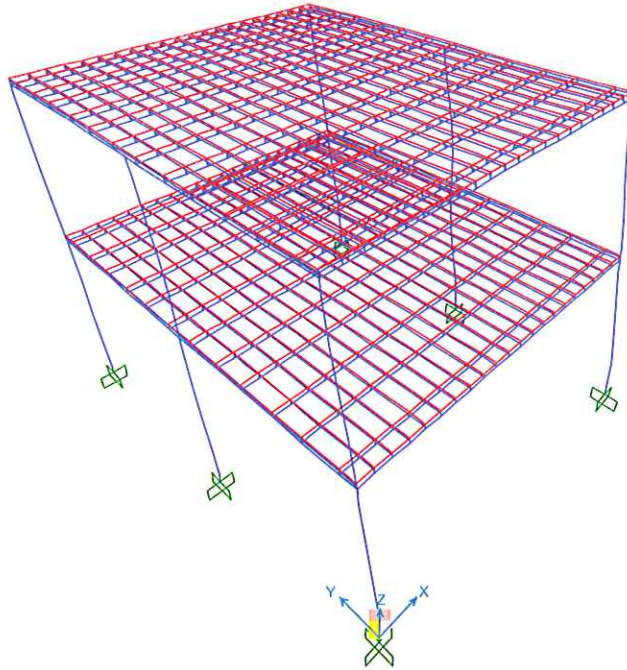


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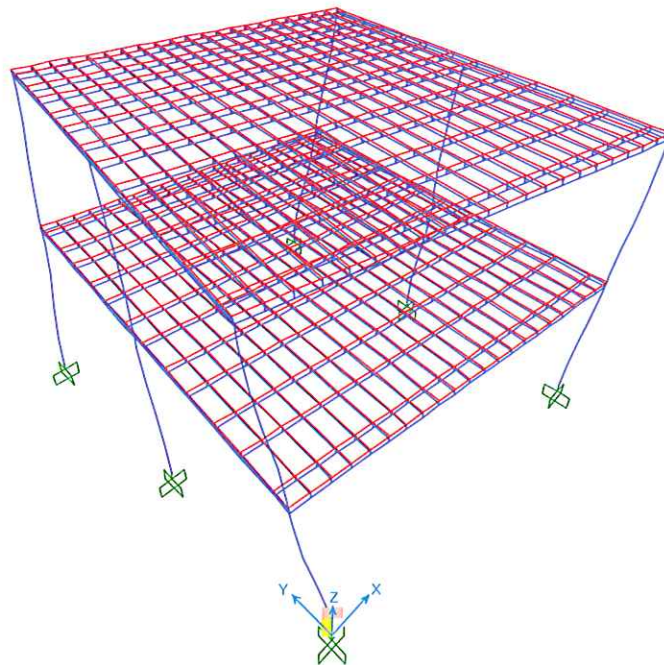
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2º modo



3º modo



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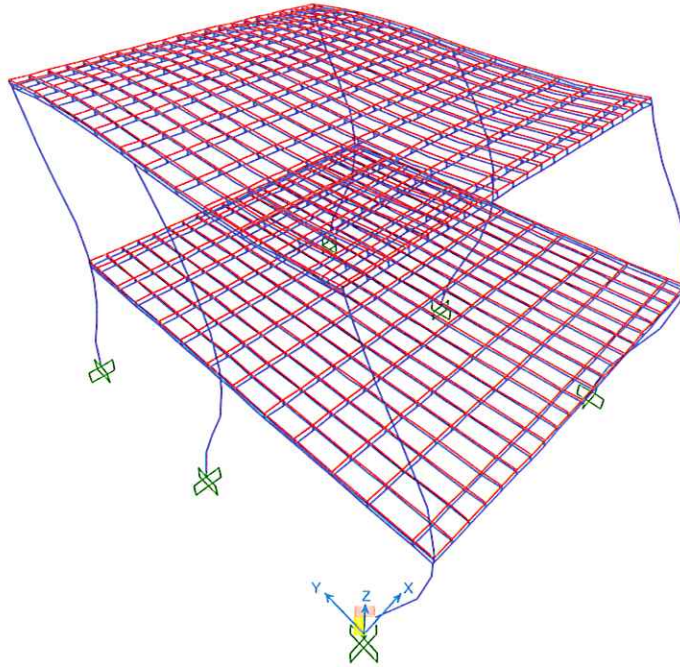
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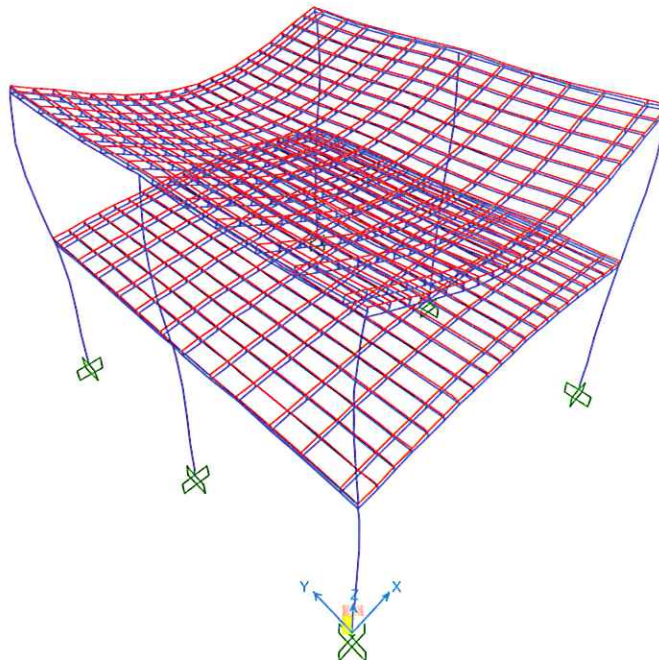




4º modo



5º modo



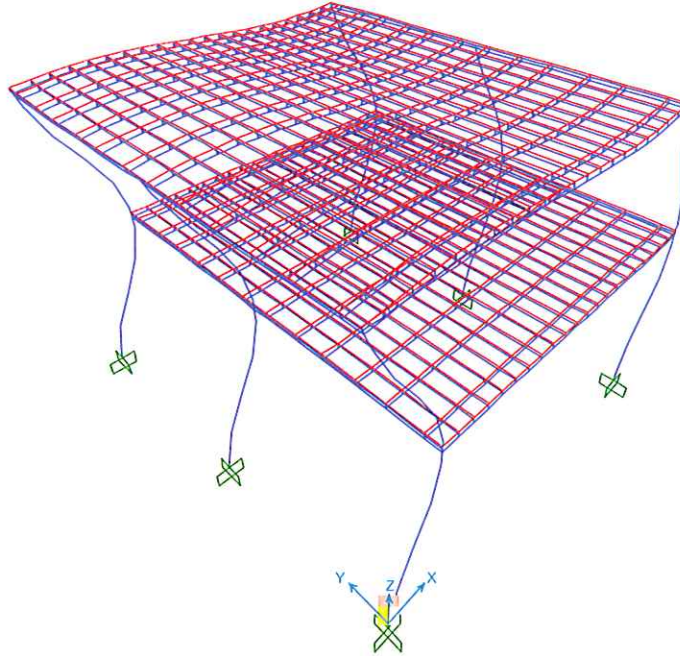
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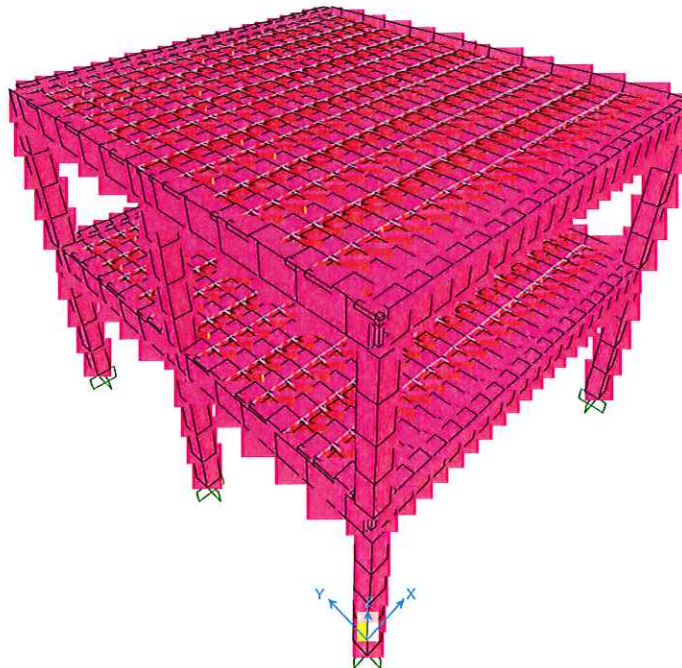
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6º modo



Modelo estrutural

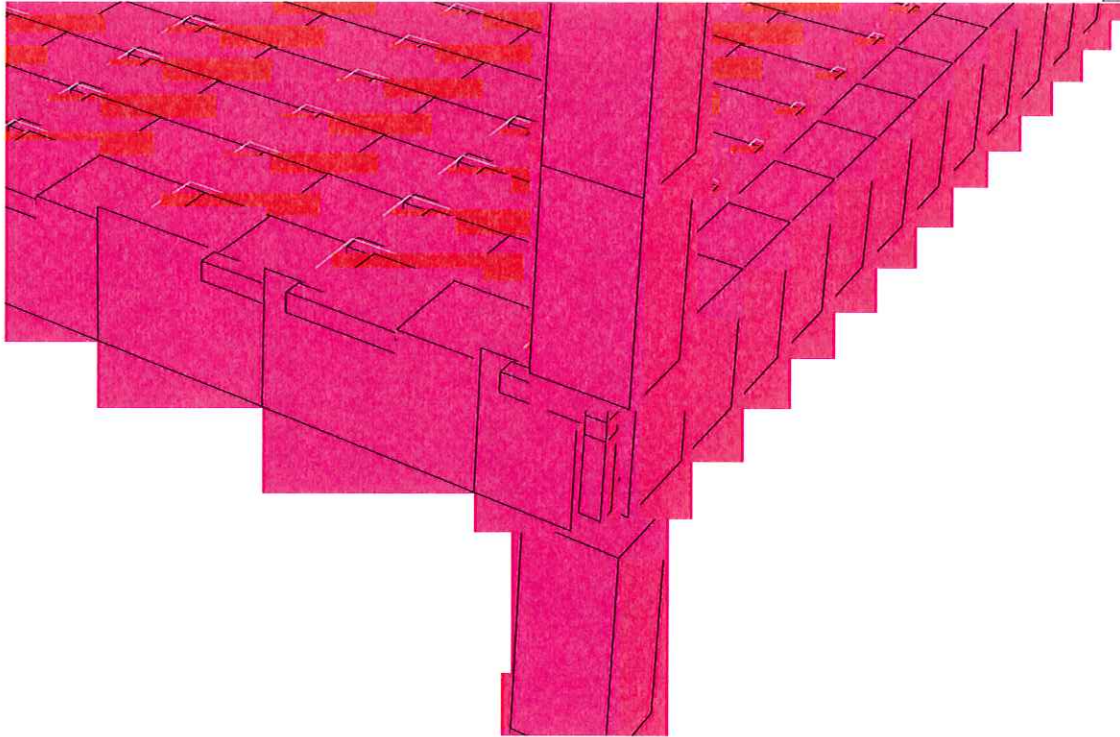


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Esforços de dimensionamento dos pilares (sem consideração de excentricidades)

Joint Text	OutputCase Text	CaseType Text	StepType Text	U1 KN	U2 KN	U3 KN	R1 KN-m	R2 KN-m	R3 KN-m
21	ELUx	Combination	Max	74.612	17.763	360.436	23.4297	181.8942	0.1972
21	ELUx	Combination	Min	-33.735	-7.133	271.409	-44.7480	-110.0419	-0.4785
21	ELUy	Combination	Max	34.194	48.745	415.632	99.6614	73.7657	1.0069
21	ELUy	Combination	Min	-0.829	-34.693	212.554	-128.4206	-20.0292	-1.2629
27	ELUx	Combination	Max	44.246	18.364	357.367	22.4413	135.4250	0.0355
27	ELUx	Combination	Min	-64.073	-6.827	268.001	-45.0805	-156.4637	-0.0751
27	ELUy	Combination	Max	4.079	49.796	413.784	97.7525	27.6569	0.1404
27	ELUy	Combination	Min	-31.433	-34.671	213.002	-128.2004	-66.8498	-0.1942
31	ELUx	Combination	Max	80.897	20.135	581.100	50.6492	191.1537	0.0094
31	ELUx	Combination	Min	-33.163	-24.876	466.056	-48.3884	-109.0657	-0.1919
31	ELUy	Combination	Max	37.231	74.568	571.914	162.8244	77.0724	0.2594
31	ELUy	Combination	Min	2.931	-75.889	471.783	-167.8540	-13.1213	-0.4584
35	ELUx	Combination	Max	43.981	19.300	575.451	48.8956	134.9890	0.1762
35	ELUx	Combination	Min	-70.082	-24.076	461.138	-46.5283	-165.2356	-0.0452
35	ELUy	Combination	Max	0.330	71.927	568.476	156.6773	20.9349	0.4499
35	ELUy	Combination	Min	-34.012	-73.087	471.576	-161.9761	-69.3335	-0.3346
39	ELUx	Combination	Max	75.116	10.821	362.045	31.4957	183.1080	0.3981
39	ELUx	Combination	Min	-34.137	-12.420	272.411	-34.1282	-110.9208	-0.2428
39	ELUy	Combination	Max	34.564	39.732	384.953	104.5304	74.5835	1.1904
39	ELUy	Combination	Min	-0.872	-38.001	248.704	-114.5246	-20.0287	-1.0060
43	ELUx	Combination	Max	44.803	10.680	358.384	31.5159	136.7172	0.1418
43	ELUx	Combination	Min	-64.487	-12.772	268.410	-33.3320	-157.3687	-0.1015
43	ELUy	Combination	Max	4.295	39.966	384.068	103.5092	28.2511	0.1917
43	ELUy	Combination	Min	-31.272	-38.525	247.204	-113.0507	-66.5266	-0.1773

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Esforços de dimensionamento dos pilares (considerando excentricidades)

OBRA: Estrutura LNEC

PILAR: P1

.....

bx = 0.45, by = 0.45, l = 4.64, Recobrimento = 0.040 (m)
 Dir. X: lo/i = 81.0 (NM) Dir. Y: lo/i = 81.0 (NM)

Comb.	Sec.	ey (cm)	ex (cm)	Nsd (kN)	M'sdx (kN.m)	M'sdy (kN.m)	As (cm2)	As (min) (cm2)
1	Inf	16.8	21.5	360.62	83.75	259.39	29.58	12.15
1	Sup	16.8	21.5	360.53	83.73	259.37	29.58	12.15
2	Inf	16.8	21.5	271.61	90.75	168.29	18.97	12.15
2	Sup	16.8	21.5	271.51	90.73	168.27	18.97	12.15
3	Inf	16.8	21.5	415.83	170.04	163.24	25.25	12.15
3	Sup	16.8	21.5	415.73	170.03	163.22	25.25	12.15
4	Inf	16.8	21.5	212.74	163.84	55.66	17.08	12.15
4	Sup	16.8	21.5	212.65	163.82	55.64	17.08	12.15

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Prémio IDICT "Programa Trabalho Seguro" - 2002





OBRA: Estrutura LNEC
 PILAR: P1

.....

bx = 0.45, by = 0.45, l = 4.64, Recobrimento = 0.040 {m}
 Dir. X: lo/i = 81.0 (NM) Dir. Y: lo/i = 81.0 (NM)

Comb.	Sec.	ey (cm)	ex (cm)	Nsd (kN)	M'sdx (kN.m)	M'sdy (kN.m)	As (cm ²)	As(min) (cm ²)
1	Inf	17.1	21.5	362.24	92.87	260.86	30.50	12.15
1	Sup	17.1	21.5	362.15	92.85	260.84	30.50	12.15
2	Inf	17.1	21.5	272.61	80.56	169.59	18.40	12.15
2	Sup	17.1	21.5	272.51	80.54	169.57	18.40	12.15
3	Inf	17.1	21.5	385.14	170.78	157.78	25.08	12.15
3	Sup	17.1	21.5	385.05	170.76	157.76	25.08	12.15
4	Inf	17.1	21.5	248.88	157.51	63.50	16.06	12.15
4	Sup	17.1	21.5	248.79	157.49	63.48	16.06	12.15

OBRA: Estrutura LNEC
 PILAR: P3

.....

bx = 0.45, by = 0.45, l = 4.64, Recobrimento = 0.040 {m}
 Dir. X: lo/i = 80.0 (NM) Dir. Y: lo/i = 80.0 (NM)

Comb.	Sec.	ey (cm)	ex (cm)	Nsd (kN)	M'sdx (kN.m)	M'sdy (kN.m)	As (cm ²)	As(min) (cm ²)
1	Inf	17.6	21.8	581.18	153.23	317.56	44.43	12.15
1	Sup	17.6	21.8	581.09	153.22	317.54	44.44	12.15
2	Inf	17.6	21.8	466.24	130.02	210.53	27.01	12.15
2	Sup	17.6	21.8	466.15	130.00	210.51	27.01	12.15
3	Inf	17.6	21.8	574.10	263.99	202.02	43.22	12.15
3	Sup	17.6	21.8	574.01	263.97	202.00	43.21	12.15
4	Inf	17.6	21.8	471.96	251.02	115.78	31.32	12.15
4	Sup	17.6	21.8	471.87	251.00	115.75	31.32	12.15

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Dimensionamento da laje TT e viga de apoio VL

Peças Pré-esforçadas

Título : Laje TT40 sob 5

Secção tipo : TT40x2.00

Resumo das análises efectuadas

i) Aplicação da pré-tensão "in situ".	Tracção:	Verifica
	Compressão:	Verifica
ii) Armazenamento e transporte.	Tracção:	Verifica
	Compressão:	Verifica
iii) Betonagem da laje "in situ".	Tracção:	
	Compressão:	
iv) Aplicação do pós-esforço "in situ".	Tracção:	
	Compressão:	
v) Fase de utilização.	Descompressão (Comb.Q.P.):	Verifica
	Abertura de fendas (Comb.F.):	Verifica
	Compressão (Comb.Q.P.):	Verifica
	Compressão (Comb.Raras):	Verifica
vi) Estado limite último.	Momentos (em x):	Verifica
	Esforço transversal máximo:	Verifica

Características dos materiais

Betão = C35/45	$f_{cd} = 23300$ kPa	$f_{ctk} = 2200$ kPa	$f_1 = 900$ kPa
	$f_{ck} = 35000$ kPa		$f_2 = 7000$ kPa
Aço = A500	$f_{syd} = 435000$ kPa	$f_{pk} = 1860$ MPa	
Betão transf. = C30/37	$f_{ck,j} = 30000$ kPa	$f_{ctm,j} = 2800$ kPa	

Ações consideradas

Vão = 10.50 m	Laje betonada "in situ"	Comprimento em consola
l inf. início = 1.000 m	Espessura no início = 0.00 m	Esquerda = 1.00 m
l inf. final = 1.000 m	Espessura no final = 0.00 m	Direita = 1.00 m
	Largura no início = 0.00 m	
	Largura no final = 0.00 m	

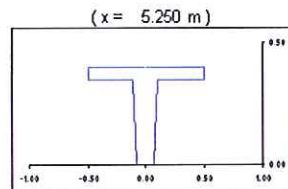
$r_{cp} = 2.00^{\circ}$ KN/m ²	$\psi_1 = 0.60$	$\psi_2 = 0.40$
Sob.(Momentos) = 5.00° KN/m ²		
Sob.(Esf.transv.) = 5.00° KN/m ²		

	pp	Δ pp	rcp	Sob.
C.equiv. 1/2 Vão	2.83	0.00	2.00	5.00
Coef.E.L.Último	1.35	1.35	1.50	1.50

Sem peso próprio extra
 Sem peso próprio da laje extra
 Sem restante carga perm. extra
 Sem sobrecarga extra

Características mecânicas da secção transversal

Fase 1	Fase 2
A = 0.1130 m ²	A = 0.1130 m ²
I = 0.0017 m ⁴	I = 0.0017 m ⁴
$v_i = 0.2689$ m	$v_i = 0.2689$ m
$v_s = 0.1311$ m	$v_s = 0.1311$ m
h = 0.4000 m	h = 0.4000 m
bs = 0.1500 m	bs = 0.1500 m
bmáx = 1.0000 m	



Ambiente = Moderadamente agressivo

Pré-esforço

Pré-tensão (P1)

Nº de strands:	4
Diâmetro:	0.6"
$P_{1,del} =$	600 kN

Pós-tensão (P2)

Nº	$P_{2,del}$	
	B	[kN]
Alinhamento 1	0	0.6"
Alinhamento 2	0	0.6"
Alinhamento 3	0	0.6"

Estado limite último

Máximos fase de montagem	Máximos ELÚltimo	Valores na secção corrente (x=5.25m)
Msd = 52.6 kNm	Msd = 197 kNm	Montagem Msd = 53 kNm S.Inicial
Vsd = 20.0 kN	Vsd = 75 kN	Mrd = 216 kNm S.Inicial
		E.L.Último Msd = 197 kNm S.Final
		Mrd = 216 kNm S.Final

Armadura de tracção

$A_{s,inf} =$	0.00 cm ²	y	0.05	0.35	0.00	0.00	0.00	0.00
$A_{s,sup} =$	0.30 cm ²	As	0.00	1.01	0.00	0.00	0.00	0.00

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Peças Pré-esforçadas

Título: **Viga VL sob 5**

Secção tipo: **VL (30+40)x(15+30)**

Resumo das análises efectuadas

i) Aplicação da pré-tensão "in situ".	Tracção:	Verifica
	Compressão:	Verifica
ii) Armazenamento e transporte.	Tracção:	Verifica
	Compressão:	Verifica
iii) Betonagem da laje "in situ".	Tracção:	
	Compressão:	
iv) Aplicação do pós-esforço "in situ".	Tracção:	
	Compressão:	
iv) Fase de utilização.	Descompressão (Comb.Q.P.):	Verifica
	Abertura de fendas (Comb.F.):	Verifica
	Compressão (Comb.Q.P.):	Verifica
	Compressão (Comb.Raras):	Verifica
v) Estado limite último.	Momentos (em x):	Verifica
	Esforço transversal máximo:	Verifica

Características dos materiais

Betão = C35/45	fcd = 23300 kPa	fck = 35000 kPa	fctk = 2200 kPa	$\tau_1 = 900$ kPa
Aço = A500	fscy = 435000 kPa			$\tau_2 = 7000$ kPa
Betão transf. = C30/37	fck,j = 30000 kPa	fpk = 1860 MPa	fctm,j = 2600 kPa	

Acções consideradas

Vão = 6.00 m	Laje betonada "in situ"	Comprimento em consola
l inf. início = 5.250 m	Espessura no início = 0.00 m	Esquerda = 1.00 m
l inf. final = 5.250 m	Espessura no final = 0.00 m	Direita = 1.00 m
	Largura no início = 0.00 m	
	Largura no final = 0.00 m	

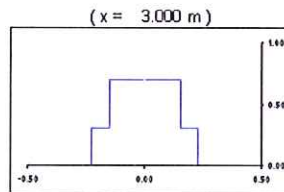
rcp = 4.83³ kN/m²
 Sob.(Momentos) = 5.00⁴ kN/m⁴ $\psi_1 = 0.60$ $\psi_2 = 0.40$
 Sob.(Esf.transv.) = 5.00⁴ kN/m⁴

	pp	Δ pp	rcp	Sob.
C.equiv. 1/2 Vão	6.38	0.00	25.36	26.25
Coef.E.L.Último	1.35	1.35	1.50	1.50

Sem peso próprio extra
 Sem peso próprio da laje extra
 Sem restante carga perm. extra
 Sem sobrecarga extra

Características mecânicas da secção transversal

Fase 1	Fase 2
A = 0.2550 m ²	A = 0.2550 m ²
I = 0.0104 m ⁴	I = 0.0104 m ⁴
vi = 0.3147 m	vi = 0.3147 m
vs = 0.3853 m	vs = 0.3853 m
h = 0.7000 m	h = 0.7000 m
bs = 0.3000 m	bs = 0.3000 m
bmáx = 0.4500 m	



Ambiente = Moderadamente agressivo

Pré-esforço

Pré-tensão (P1)	Pós-tensão (P2)
Nº de strands: 4	Alinhamento 1
Diâmetro: 0.6"	Alinhamento 2
$P_{1,ext} = 600$ kN	Alinhamento 3

Estado limite último

Máximos fase de montagem	Máximos ELÚltimo	Valores na secção corrente (x=3.00m)
Msd = 38.7 kNm	Msd = 387 kNm	Montagem Msd = 39 kNm S.Inicial
Vsd = 25.8 kN	Vsd = 258 kN	Mrd = 569 kNm S.Inicial
		E.L.Último Msd = 387 kNm S.Final
		Mrd = 569 kNm S.Final

Armadura de tracção

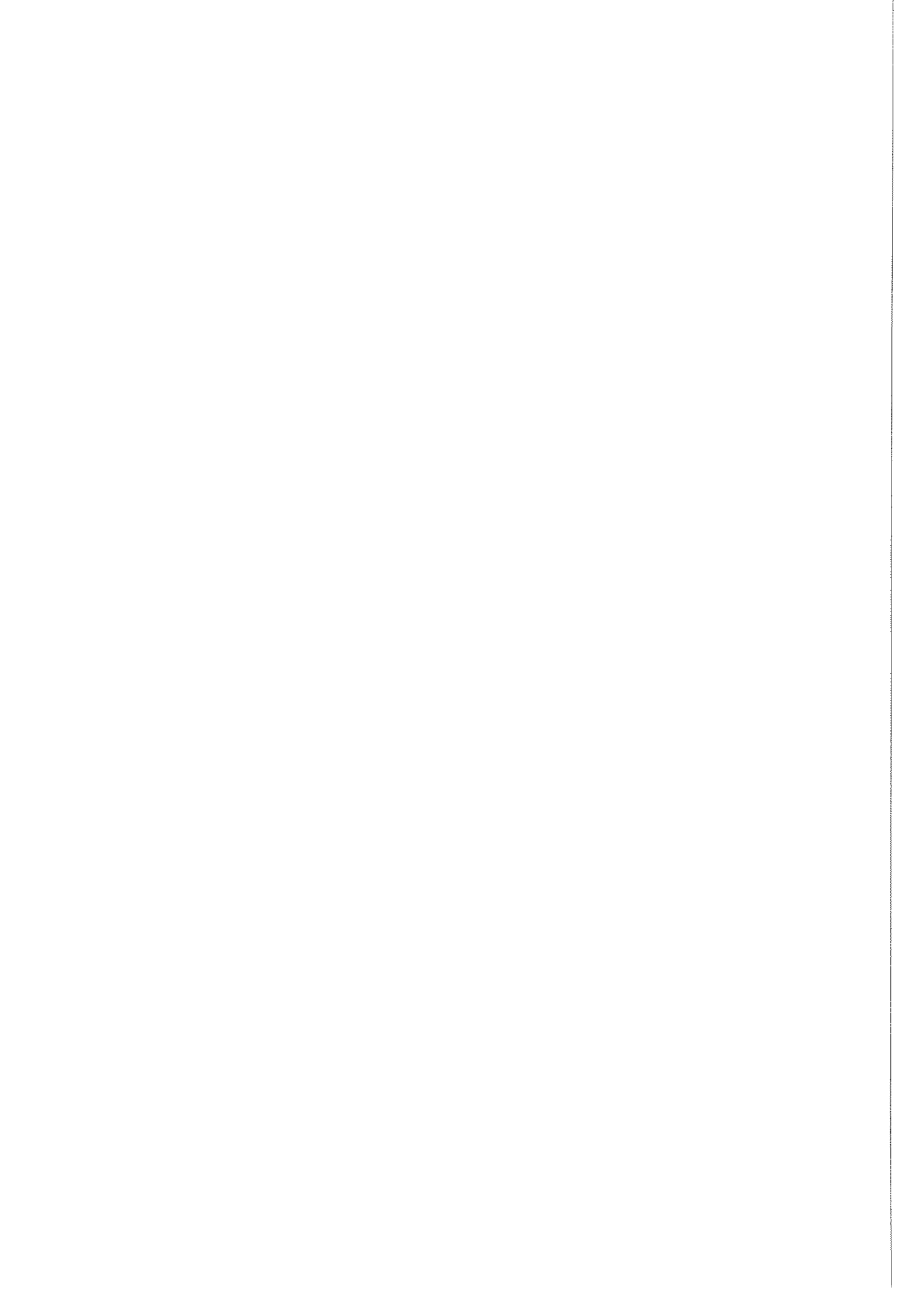
$A_{s,inf} = 0.00$ cm ²	
$A_{s,sup} = 2.89$ cm ²	

y	0.05	0.35	0.00	0.00	0.00	0.00
As	5.65	3.83	0.00	0.00	0.00	0.00

Prémio IDICT "Prevenir Mais Viver Melhor no Trabalho" – 2003 - Menção Honrosa

Prémio IDICT "Programa Trabalho Seguro" - 2002

IMP – ADM 010



ANNEX B PROTOTYPE DRAWINGS

List of Drawings:

Drawing nº 1 – Prototype – Story Plan

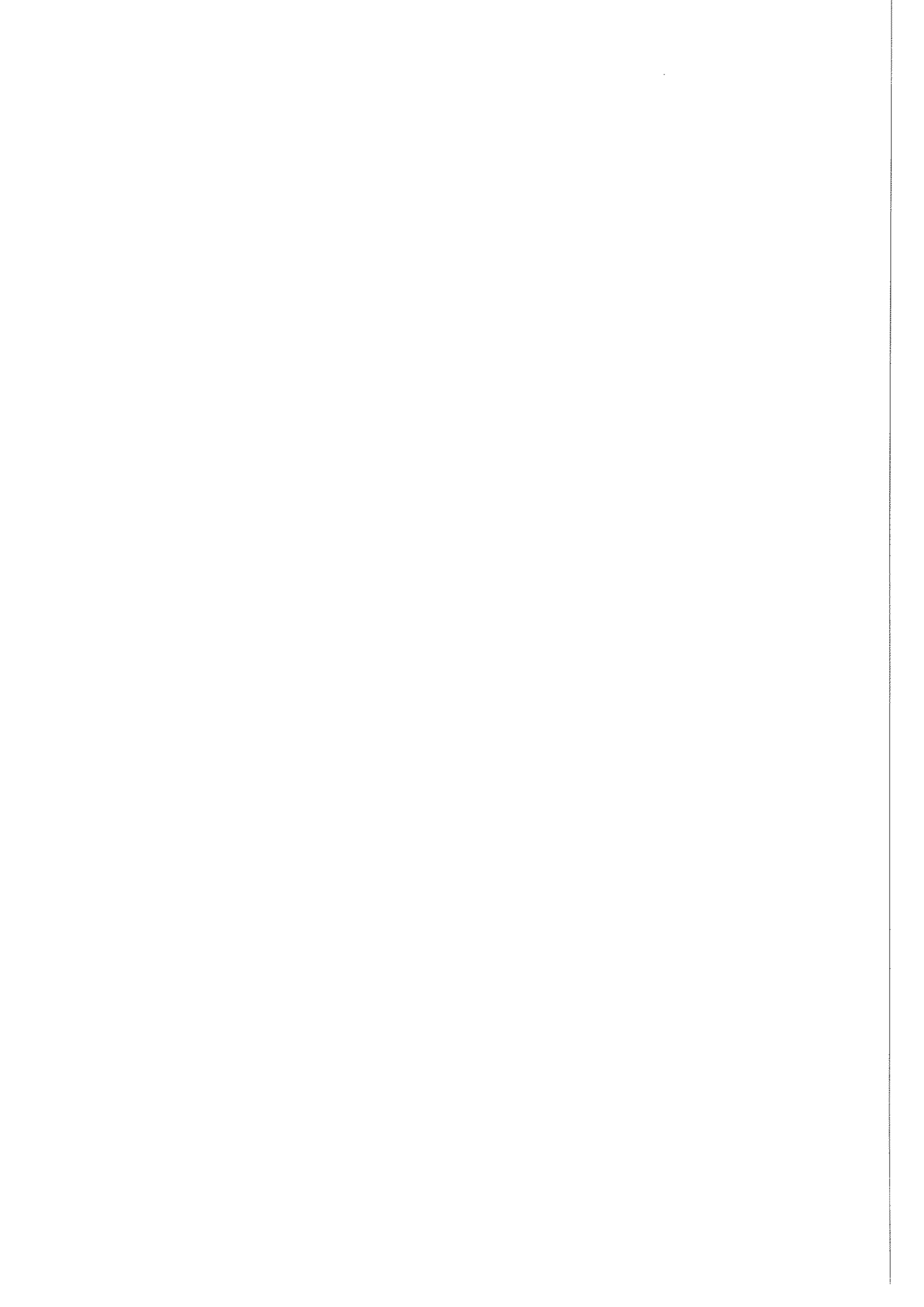
Drawing nº 2 – Prototype – Section A-B

Drawing nº 3 – Prototype – Section C-D

Drawing nº 4 – Prototype – Foundations Plan

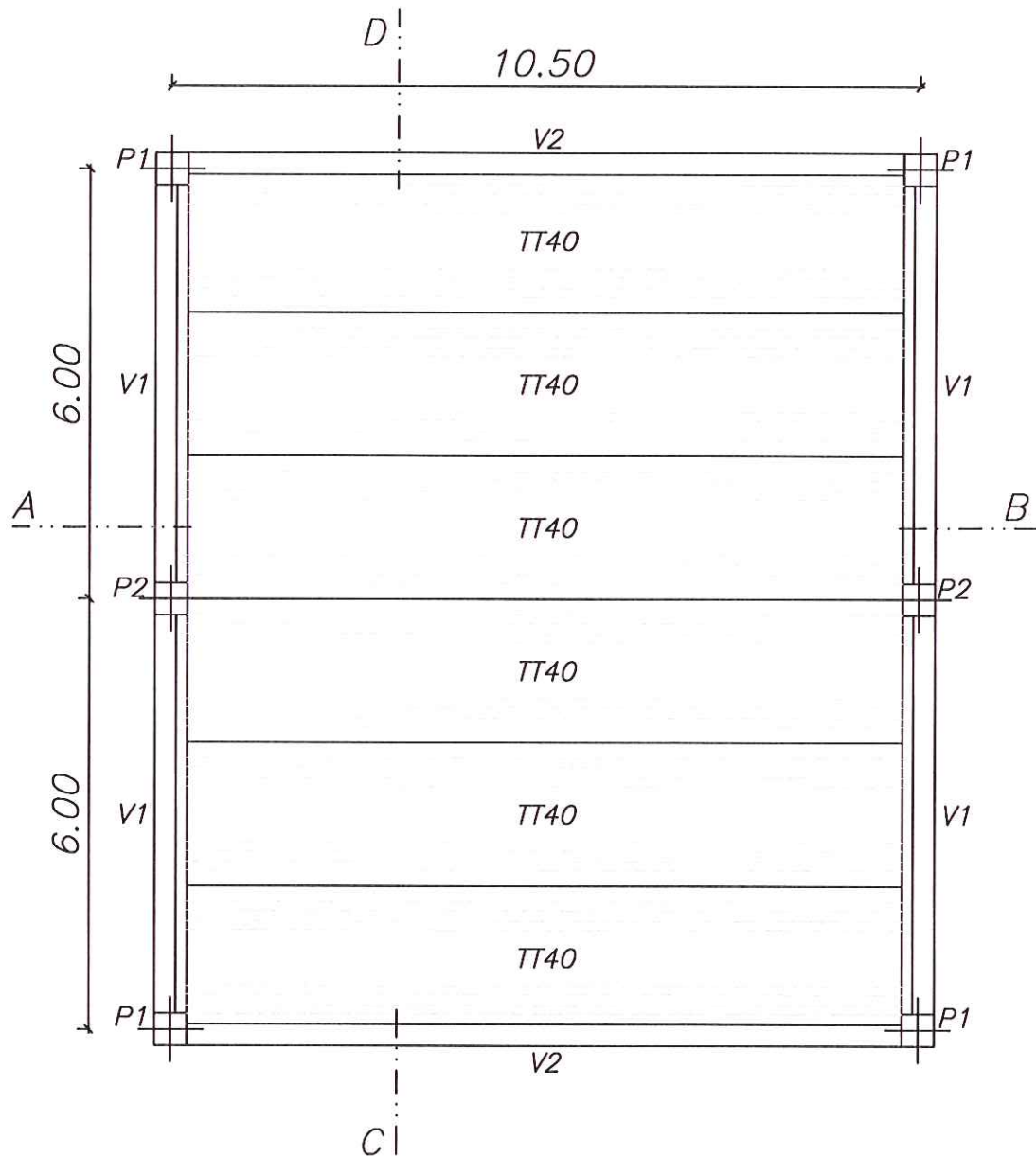
Drawing nº 5 – Prototype – Slabs and Beams Details

Drawing nº 6 – Prototype – Columns Details



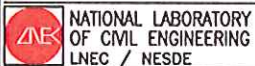
Plan View

Scale: 1/100



Dimensions in meters

This drawing is based on the Civibril S.A. system drawings



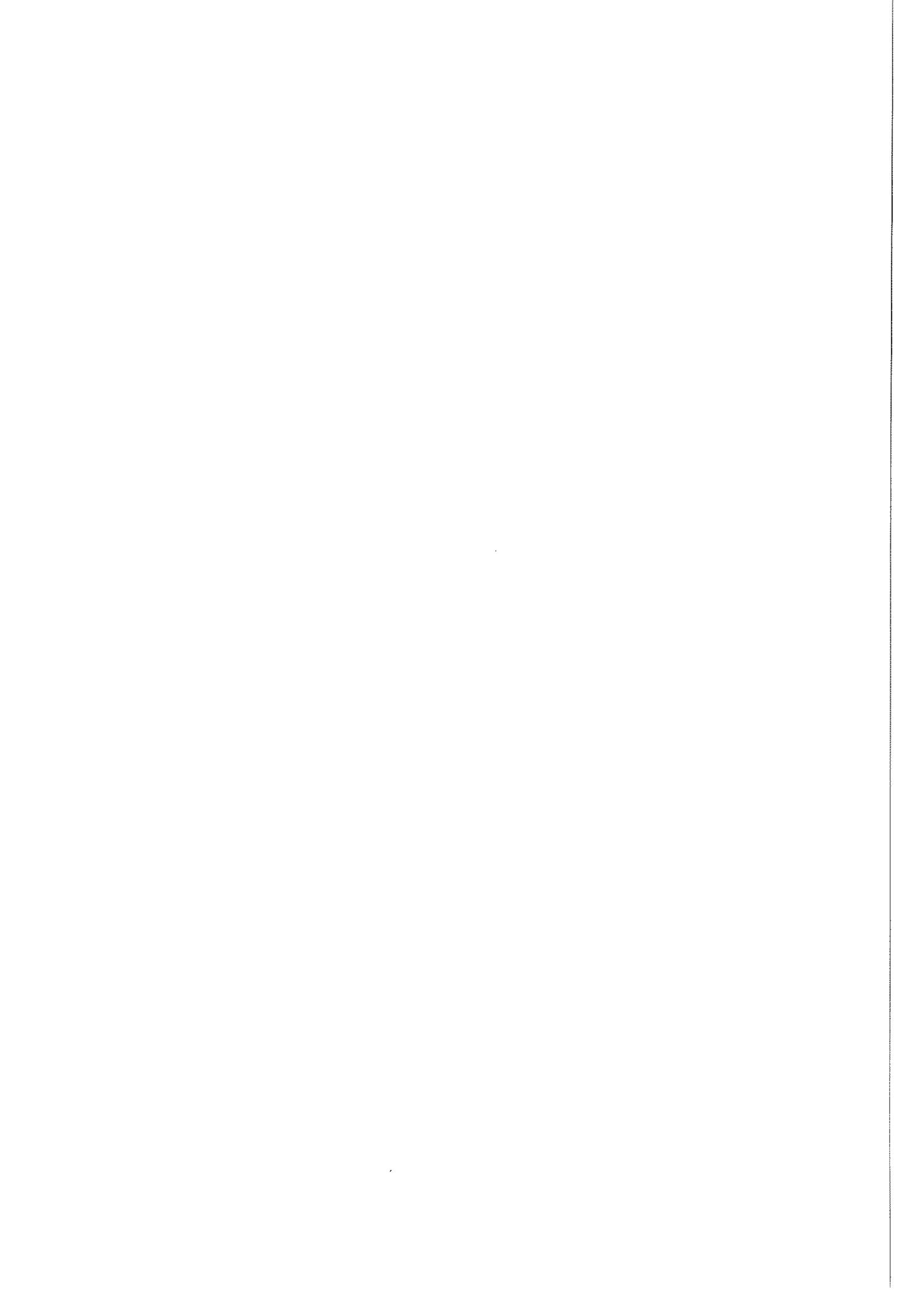
Scale: 1:100
(except if indicated)

PRECAST STRUCTURES EC8

Prototype – Story Plan

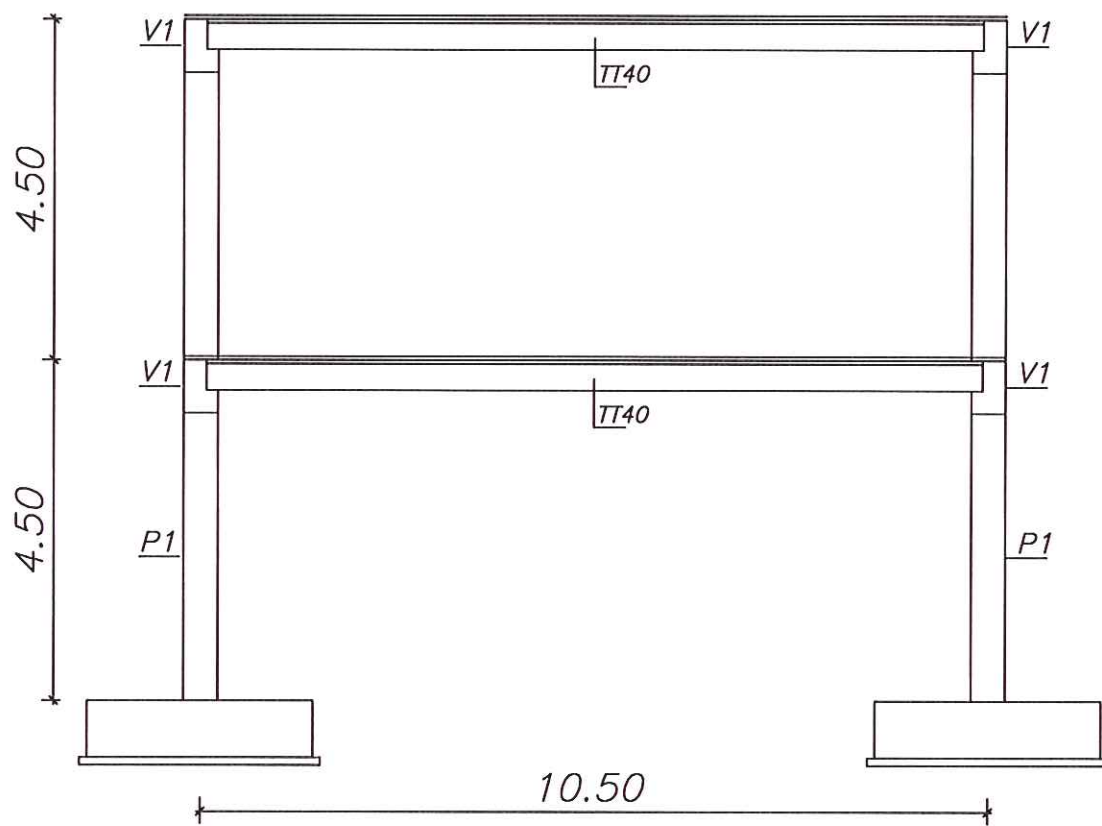
Dec 2005

Drawing N°
1



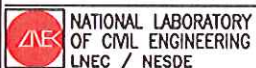
Section A-B

Scale: 1/100



Dimensions in meters

This drawing is based on the Civibril S.A. system drawings



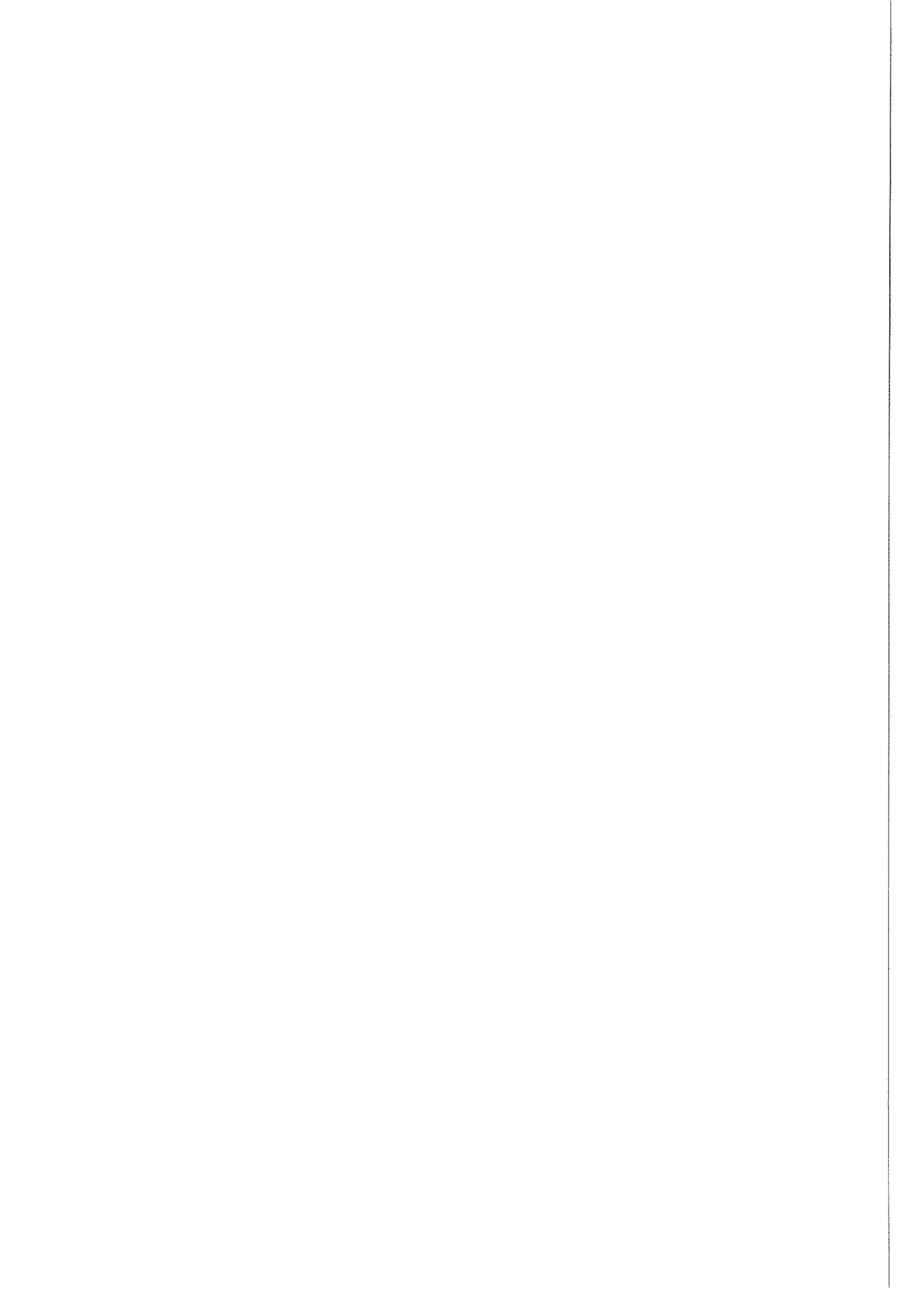
PRECAST STRUCTURES EC8

Dec 2005

Scale: 1:100
(except if indicated)

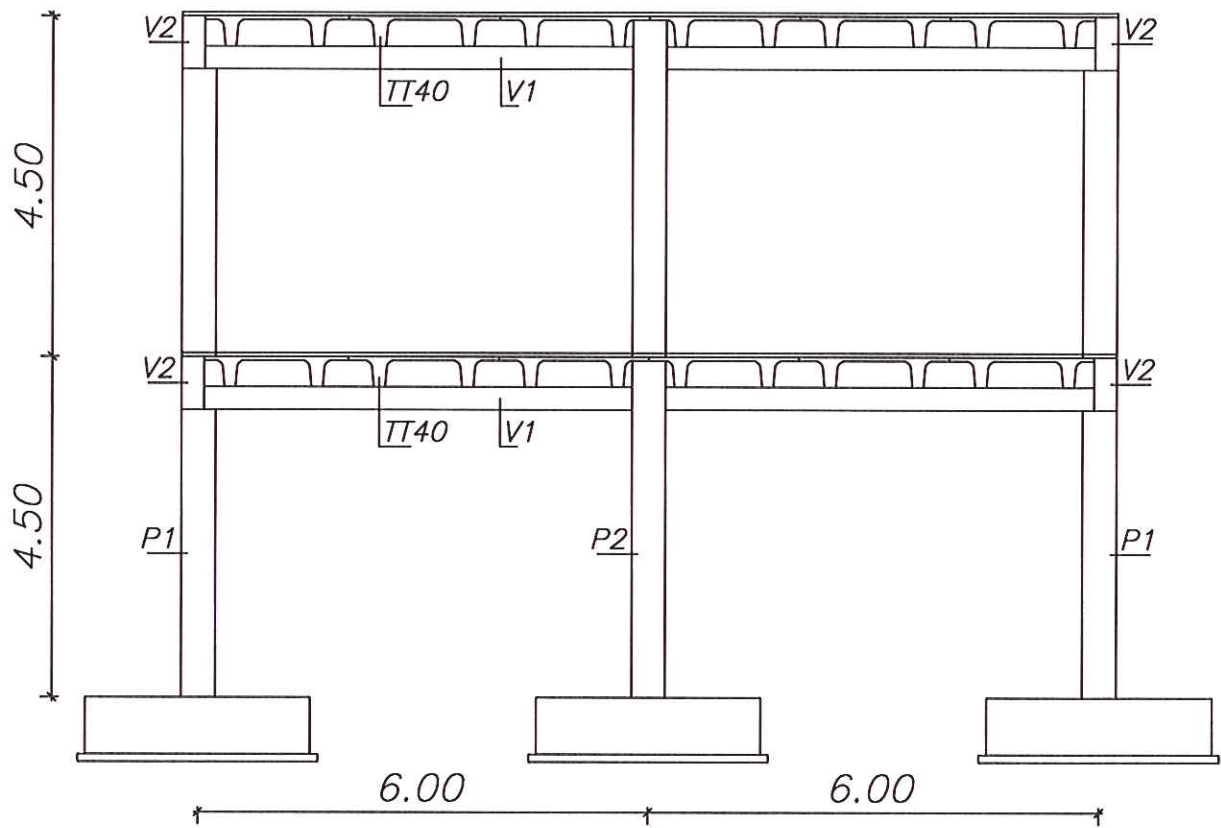
Prototype - Section A-B

Drawing N°
2



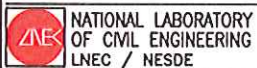
Section C-D

Scale: 1/100



Dimensions in meters

This drawing is based on the Civibril S.A. system drawings



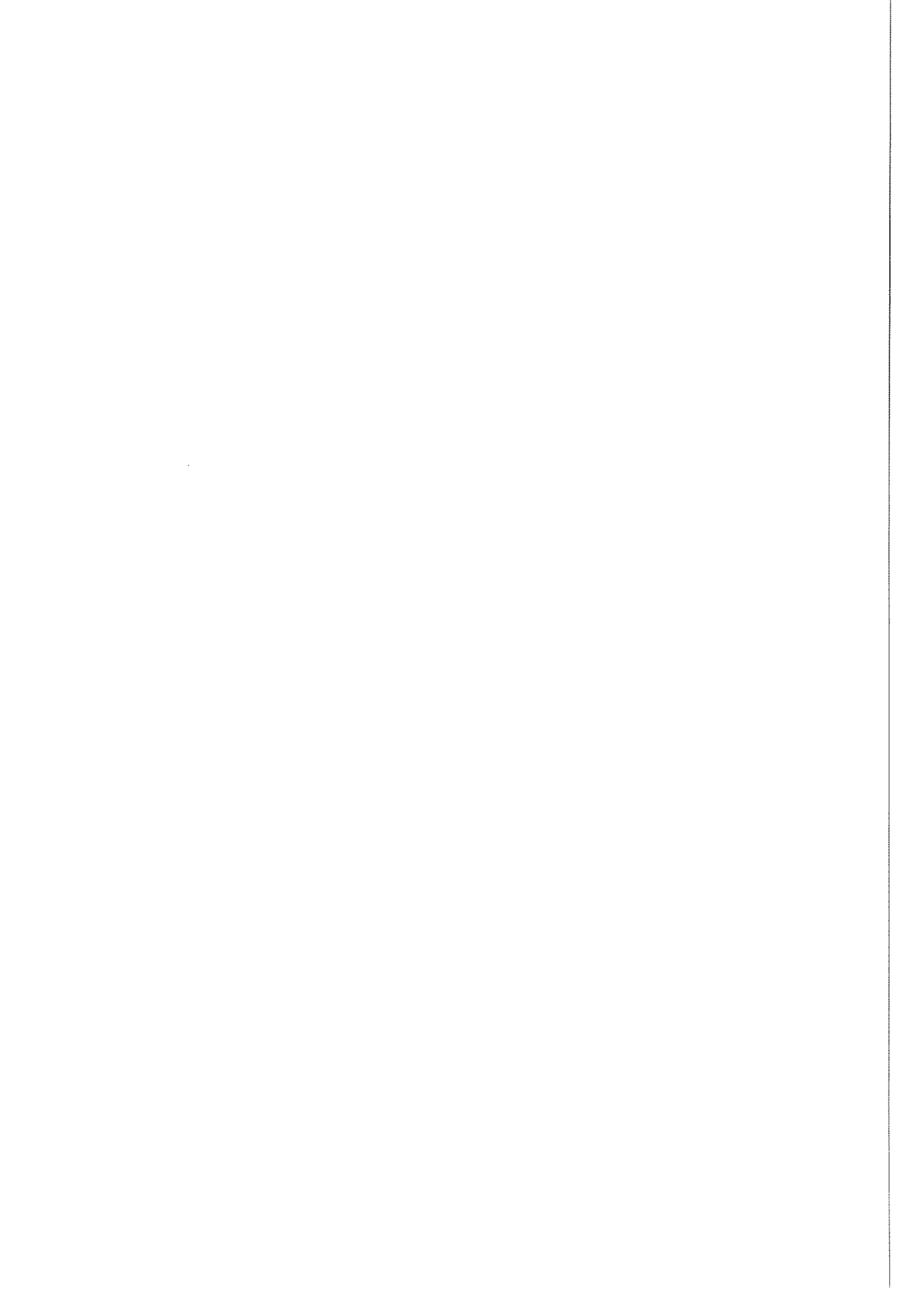
Scale: 1:100
(except if indicated)

PRECAST STRUCTURES EC8

Prototype - Section C-D

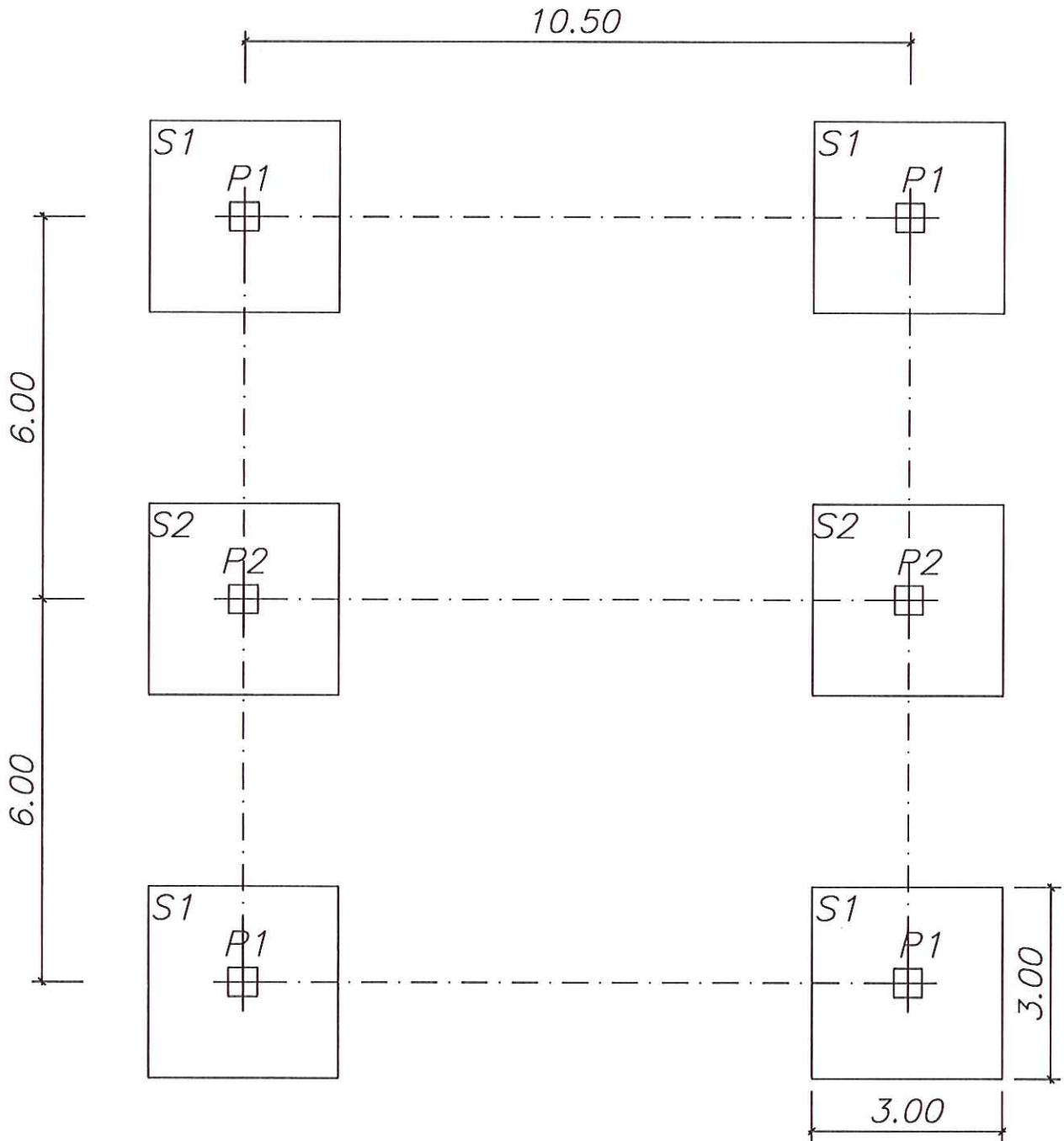
Dec 2005

Drawing N°
3



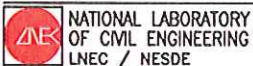
Foundations Plan

Scale: 1/100



Dimensions in meters

This drawing is based on the Civibral S.A. system drawings



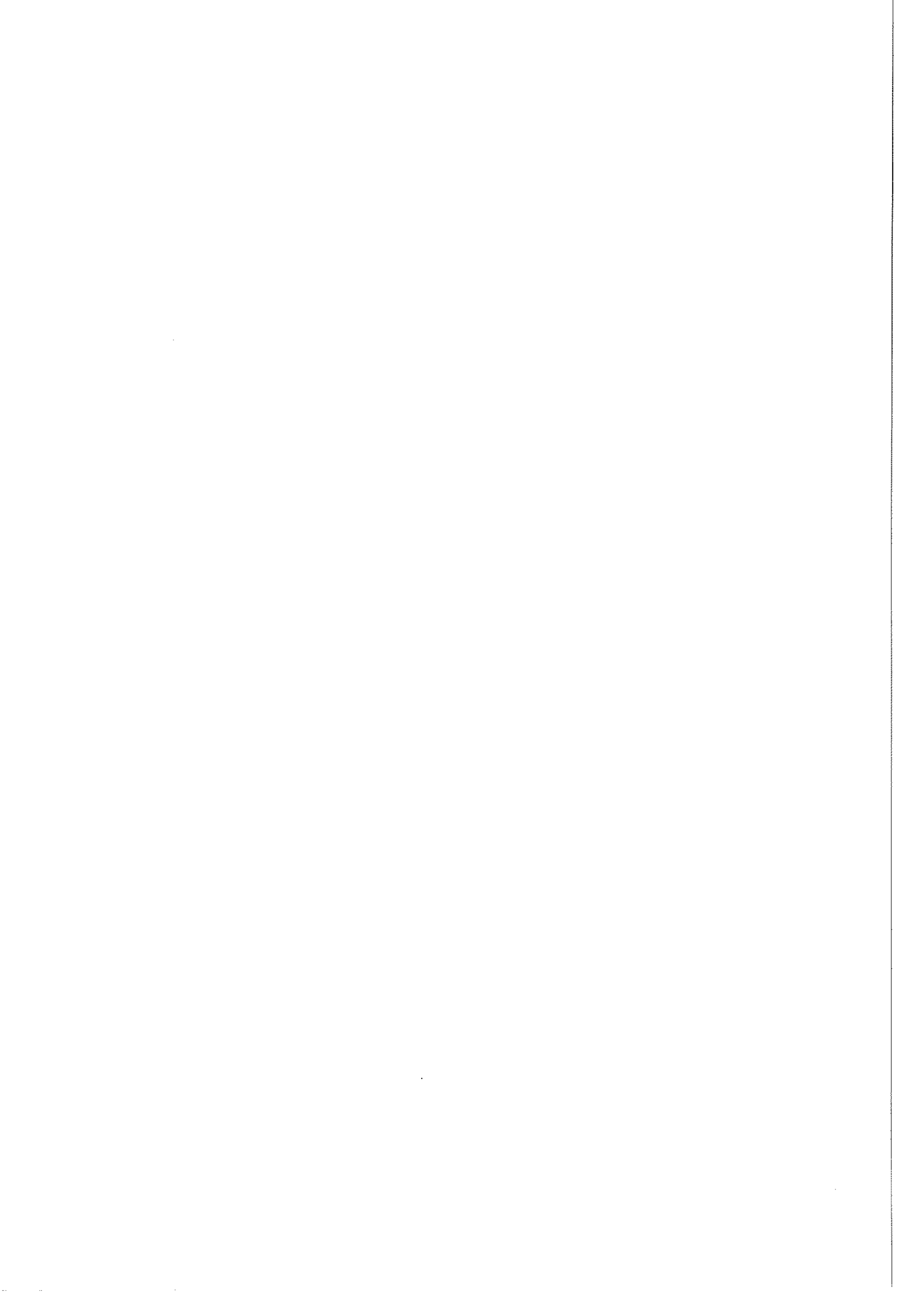
PRECAST STRUCTURES EC8

Dec 2005

Scale: 1:100
(except if indicated)

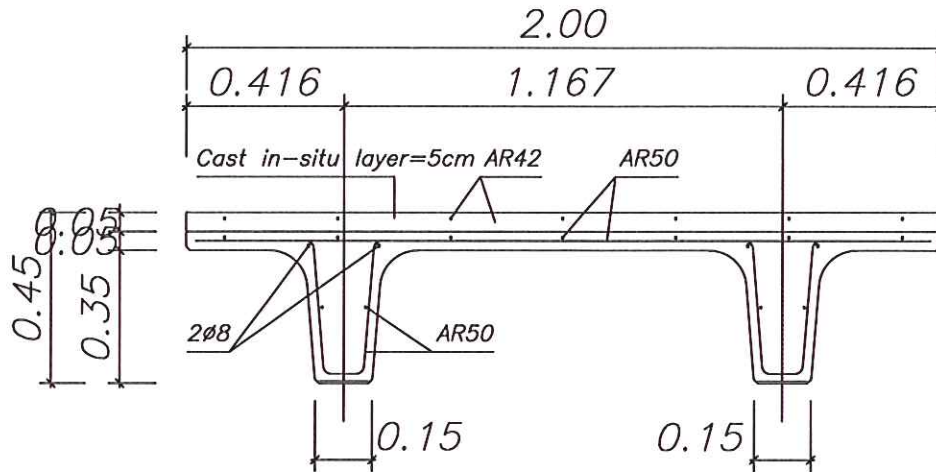
Prototype – Foundations Plan

Drawing N°
4



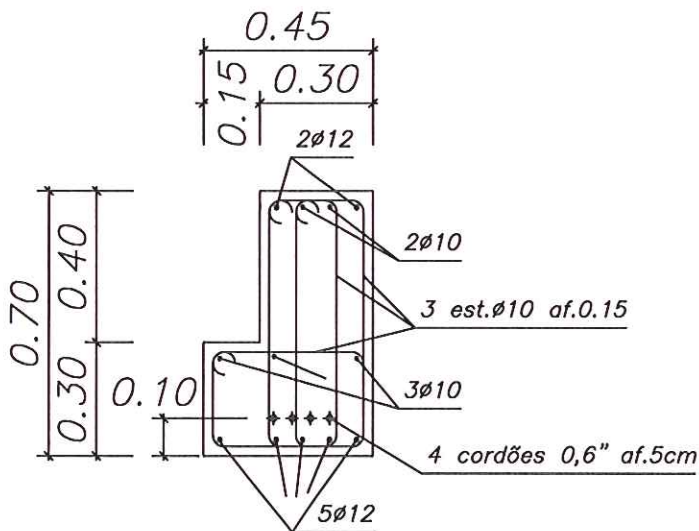
Slab TT40

Scale: 1/20



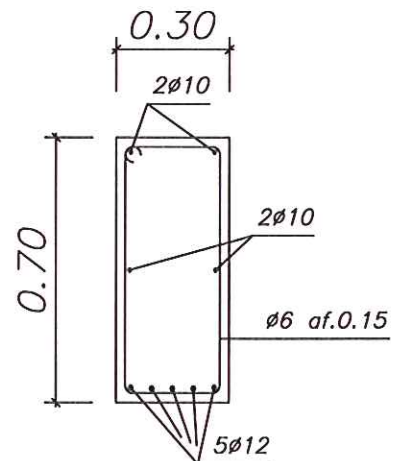
Beam V1

Scale: 1/20



Beam V2

Scale: 1/20

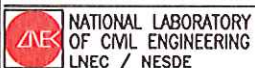


MATERIALS

Concrete:	
Prestressed Elements	C35/45
Other Elements	C25/30
Steel:	
Reinforcing Steel	A500NR
Welded Mesh	A500EL
Prestressing Steel	$F_{puK}=1860\text{Mpa}$
Cover:	2.5cm

Dimensions in meters

This drawing is based on the Civibril S.A. system drawings



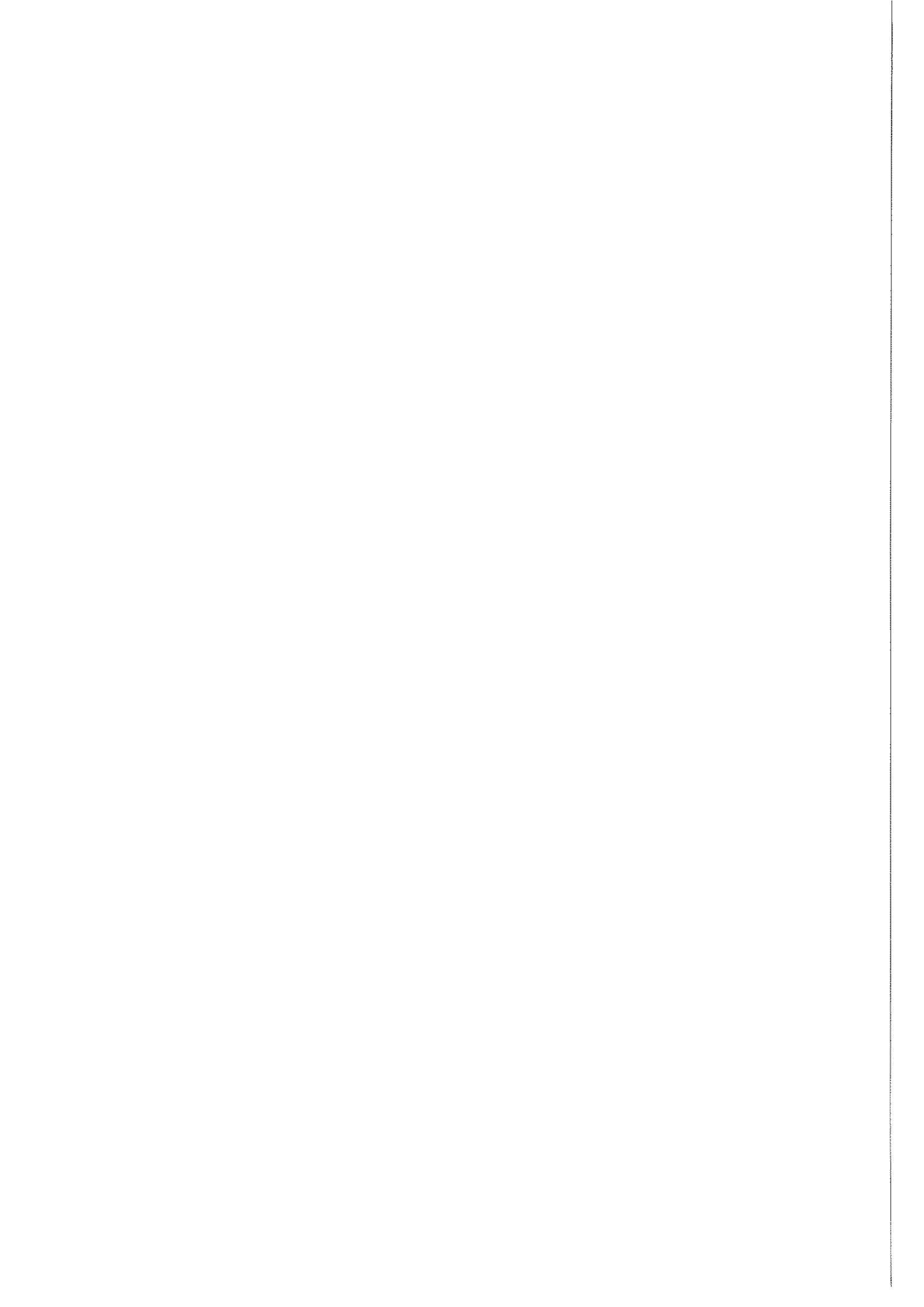
Scale: 1:100
(except if indicated)

PRECAST STRUCTURES EC8

Prototype – Slab and Beams Details

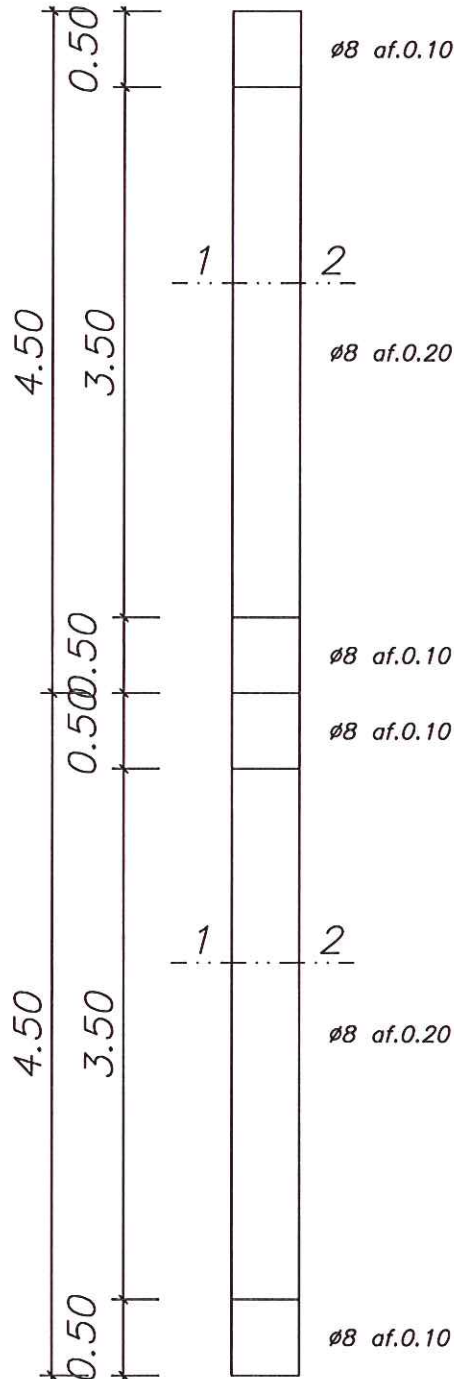
Dec 2005

Drawing N°
5



Columns Stirrups

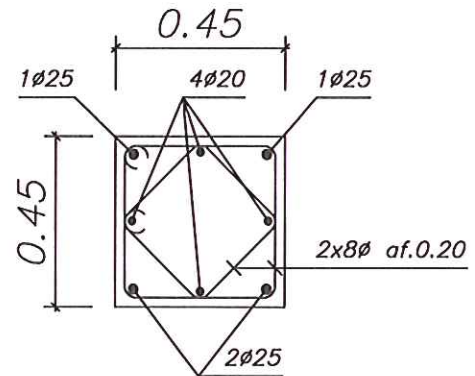
Scale: 1/50



Column P1

Section 1-2

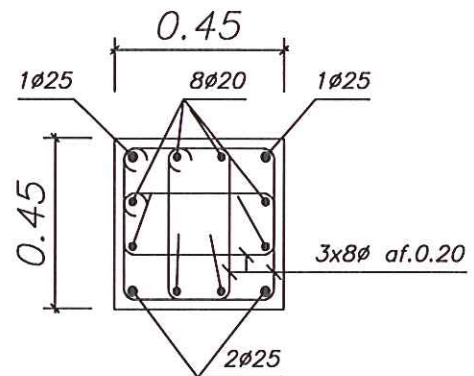
Scale: 1/20



Column P2

Section 1-2

Scale: 1/20

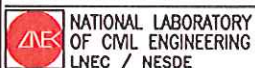


MATERIALS

Concrete:	
Prestressed Elements	C35/45
Other Elements	C25/30
Steel:	
Reinforcing Steel	A500NR
Welded Mesh	A500EL
Prestressing Steel	$F_{puK}=1860\text{Mpa}$
Cover:	2.5cm

Dimensions in meters

This drawing is based on the Civibril S.A. system drawings



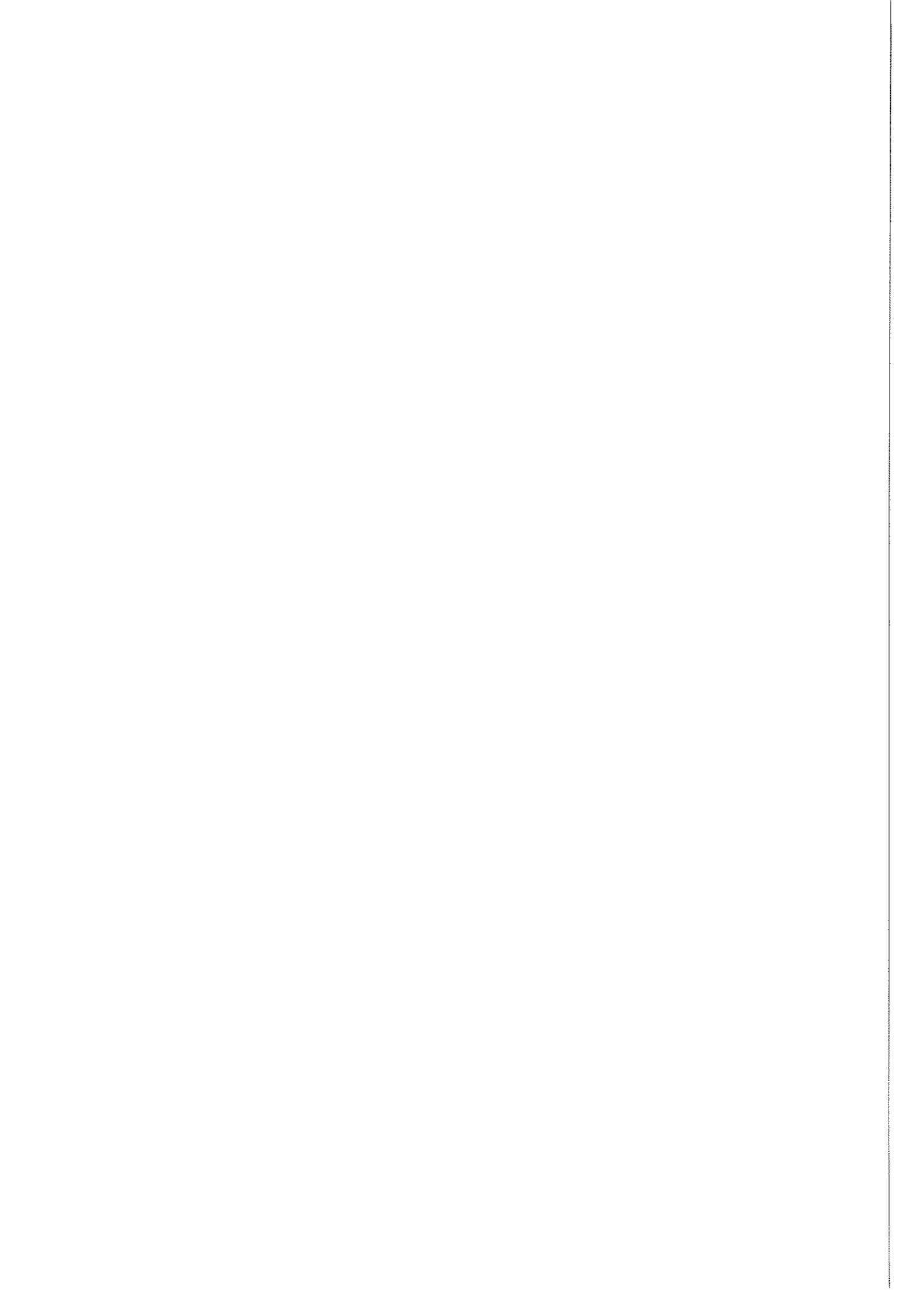
PRECAST STRUCTURES EC8

Dec 2005

Scale: 1:100
(except if indicated)

Prototype – Columns Details

Drawing N°
6



ANNEX C SHAKING TABLE CHARACTERISTICS

LNEC's TRIAXIAL SHAKING TABLE

Platform Dimensions:
 Size: 5.6m x 4.6m
 Mass: 400kN
 Max. Payload: 600kN
Degrees of freedom:
 Translation T, V, L
 Rotation N/A
Hydraulic system:
 Max. oil flow: 690 l/min
 Operating pressure: 210 bar

Axis	T	V	L
SERVO-ACTUATORS			
Quantity	2	1	1
Total force (kN)	600	300	1000
Max. displacement (cm)	20	20	20
Max. velocity (cm/s)	70	40	40
Max. acceleration (g)	1.2	0.6	2.0
PASSIVE GAS ACTUATORS			
Quantity	N/A	2	N/A
Total force (kN)	N/A	1000	N/A

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 Earthquake Engineering and Structural Dynamics Division

Full Name of the Shaking Table	MESA SÍSMICA TRIAXIAL DO LNEC
Abbreviated Name	LNEC-3G
Designer/Manufacturer	LNEC e INSTRON
Year of Installation	1995

Table C-1: General information

	Long. X	Lateral Y	Vertical Z	Pitch	Roll	Yaw
Multiaxial	Y	Y	Y	N/A	N/A	N/A

Table C-2: Type of Shaking Table

Size (m×m)	4,6 x 5,6
Weight (kN)	392
Material	Steel

Table C-3: Characteristics of the platform

Type of Actuation	Hydraulic
Electric Power (kW)	330
Flow Rate (kN)	690
Pressure (MPa)	20.7

Table C-4: Hydraulic System

	Manufacturer	Total Force (kN)	Number of units/axis
Longitudinal	INSTRON	1250	1
Lateral	INSTRON	750	2
Vertical	INSTRON	375	1

Table C-5: Actuators technical data

Frequency Range		Hz	0,1 – 40,0
Stroke (effective/maximum)	Horizontal	mm _{pp}	290/400
	Vertical	mm _{pp}	290/400
Max Velocity (nominal/limit)	Horizontal	Transversal	cm/s 70,1/121,5
		Longitudinal	cm/s 41,9/72,6
	Vertical		cm/s 42,4/73,5
	Max Acceleration at bare table	Horizontal	Transversal
Longitudinal			m/s ² 9,38
Vertical		m/s ² 31,25	
Yaw/Pitch/Roll	Rotation degrees		° N/A
	Velocity		rad/s N/A
Max Overturning Moment			kN×m N/A
Max Specimen Dead Weight			kN 392
Max Compensated Dead Weight			kN 392

Table C-6: Shaking table performance

Type of Control	Mixed (Analogue/Digital)
-----------------	--------------------------

Table C-7: Type of Control

Manufacturer	Instron
Type	Uni-Variable Units

Table C-8: Characteristics of the Analogue Part

Hardware	Computer	Instron 8580 Control Tower
	D/A Channels	8 ADC channels – 16 bit
	A/D Channels	
Software	Designer	Instron

Table C-9: Characteristics of the Digital Part

ANNEX D MODEL DRAWINGS

List of Drawings:

Drawing nº 1 – Model – Story Plan

Drawing nº 2 – Model – Section A-B

Drawing nº 3 – Model – Section C-D

Drawing nº 4 – Model – Foundations Plan

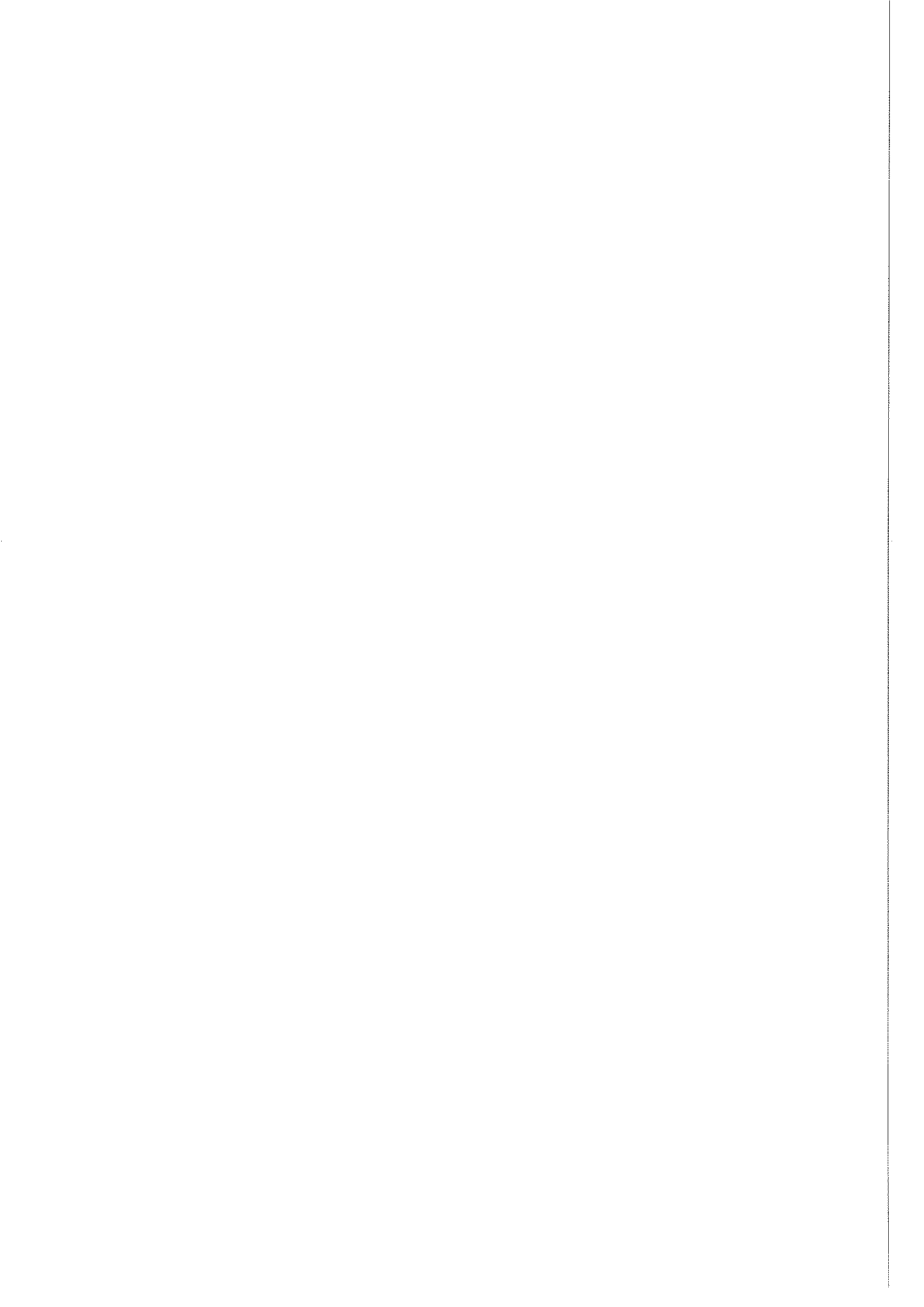
Drawing nº 5 – Model – Beam V1

Drawing nº 6 – Model – Beam V2

Drawing nº 7 – Model – Columns

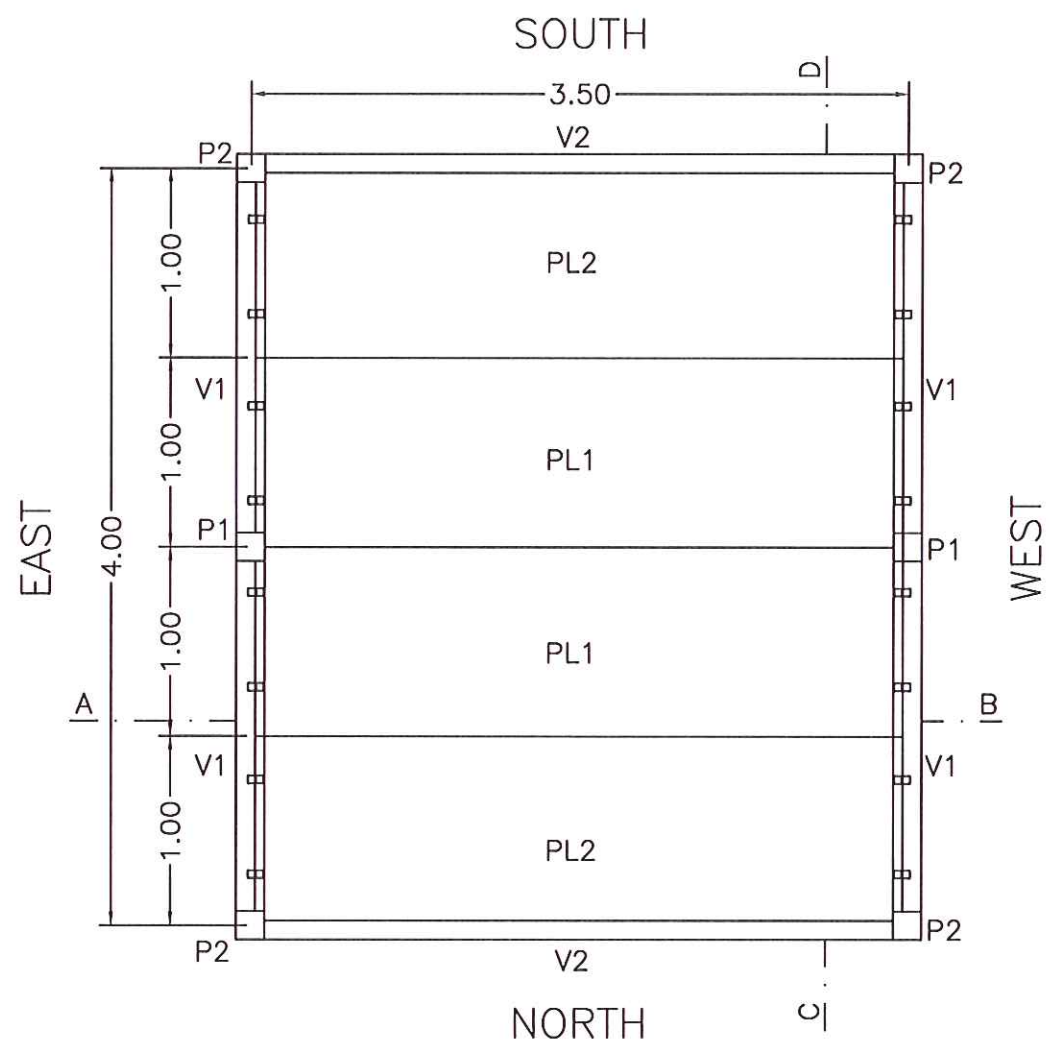
Drawing nº 8 – Model – Slabs

Drawing nº 9 – Model – Position of the Additional Masses




Plan View

Scale: 1/40



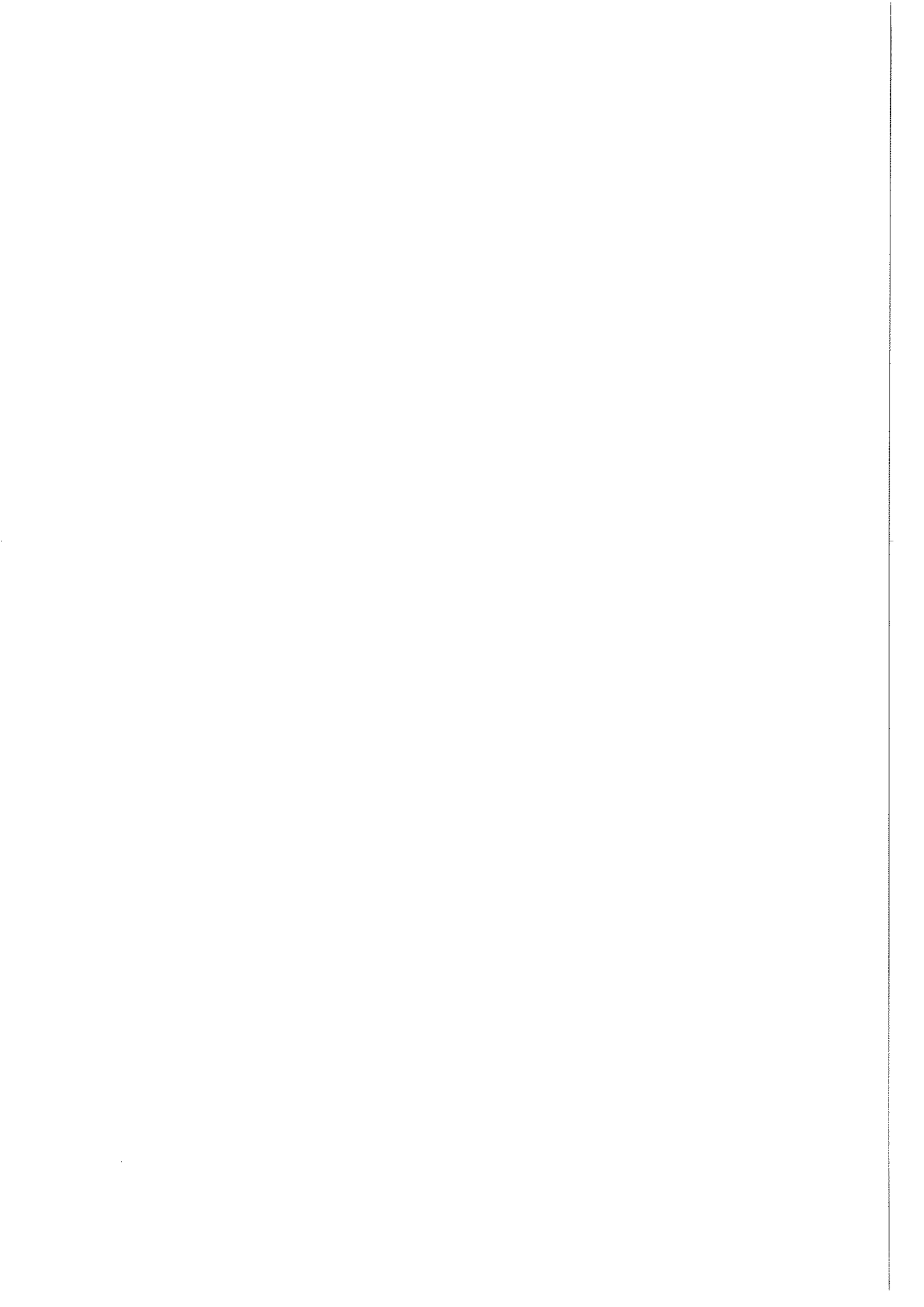
Dimensions in meters

This drawing is based on the Civibril S.A. system drawings


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 Scale: 1:100
(except if indicated)

PRECAST STRUCTURES EC8
 Model – Story Plan

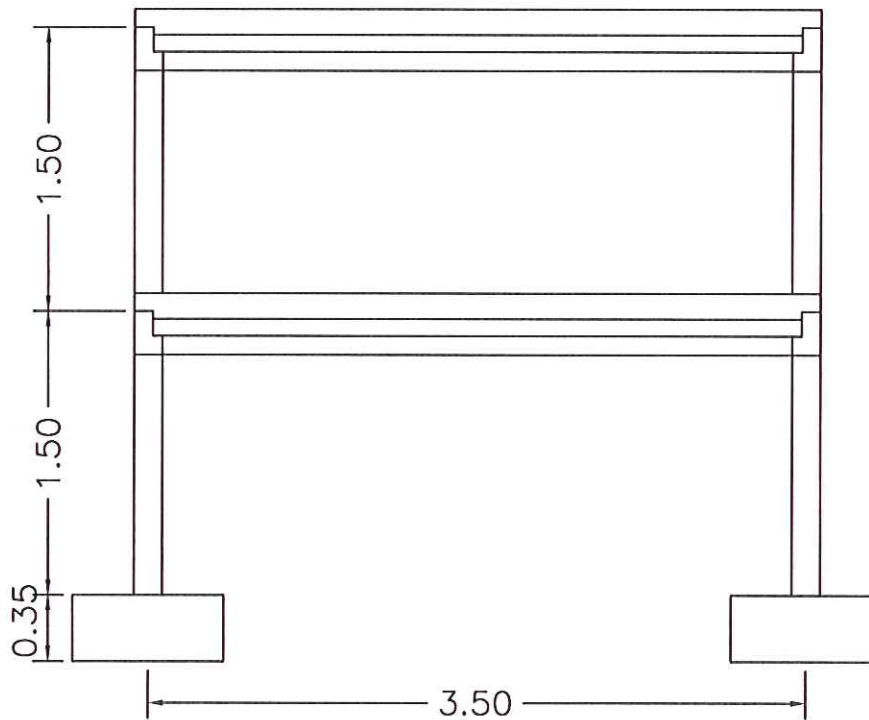
Dec 2005
 Drawing N°
1



Section A-B

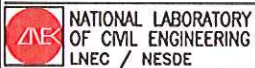
North View

Scale: 1/40



Dimensions in meters

This drawing is based on the Civibril S.A. system drawings



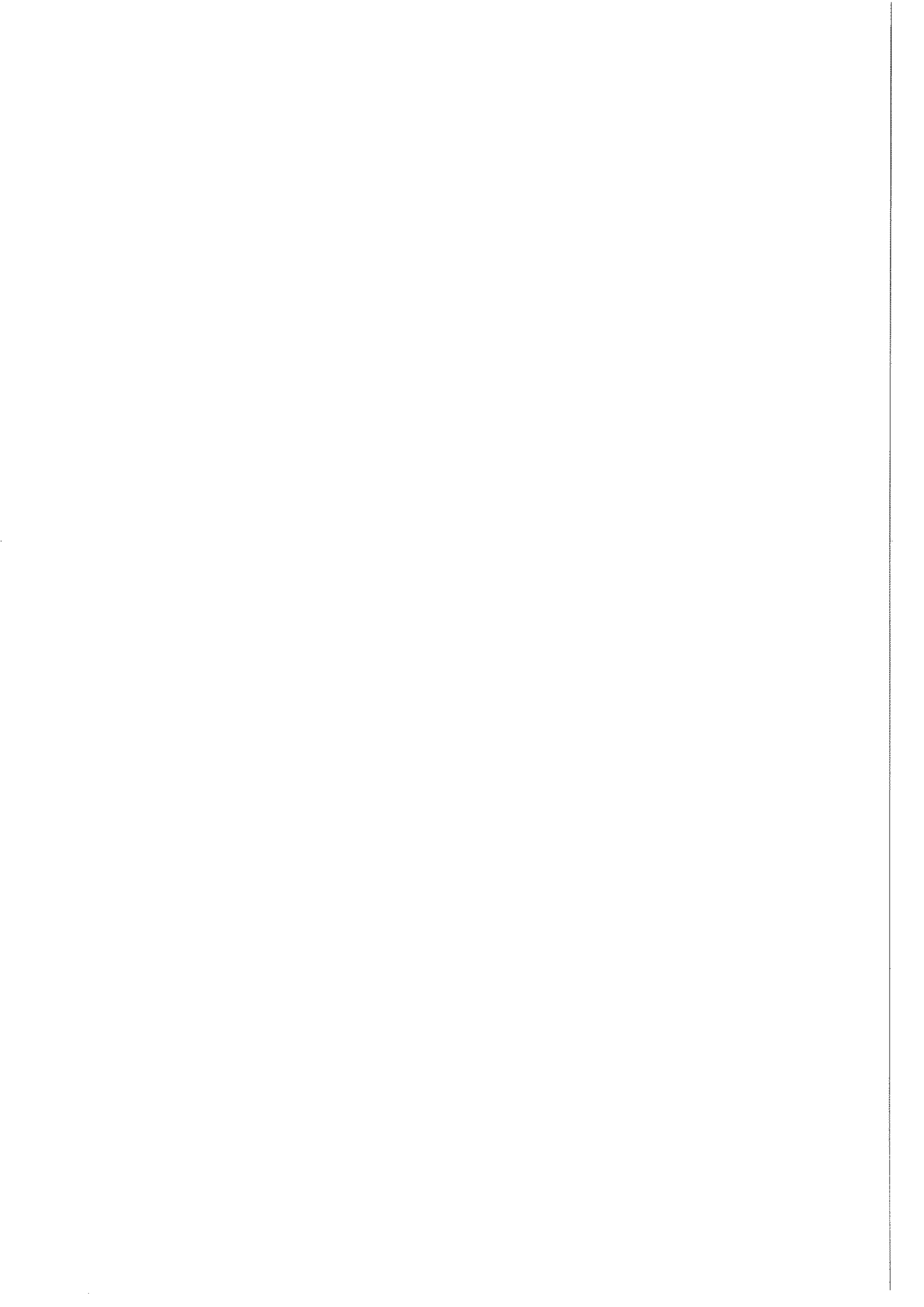
PRECAST STRUCTURES EC8

Dec 2005

Scale: 1:100
(except if indicated)

Model - Section A-B

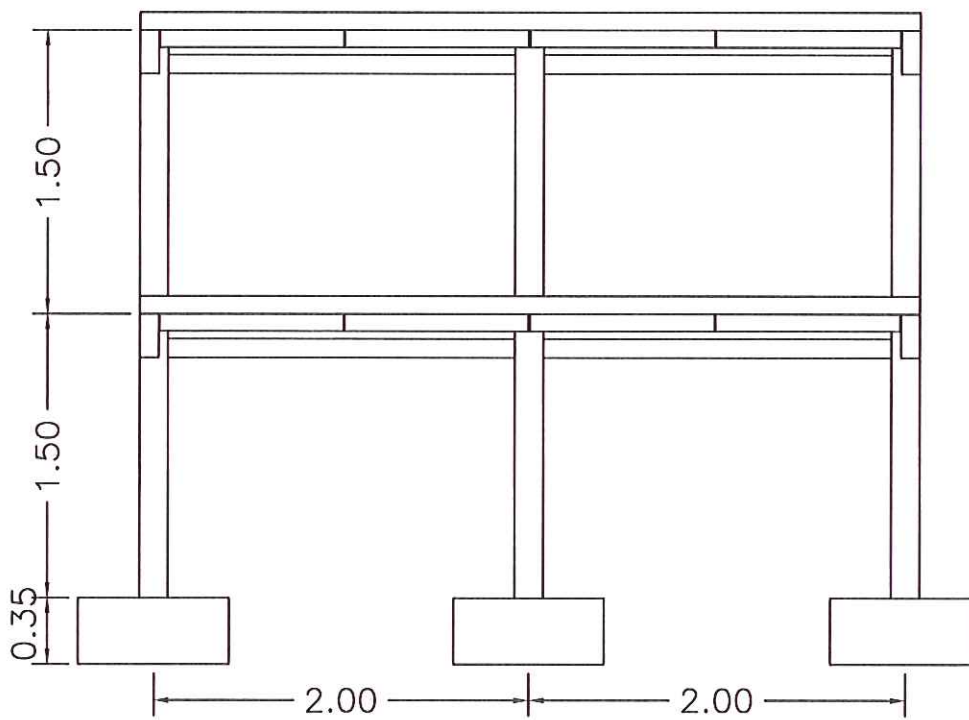
Drawing N°
2



Section C-D

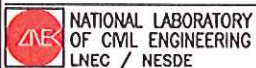
East View

Scale: 1/40



Dimensions in meters

This drawing is based on the Civibril S.A. system drawings



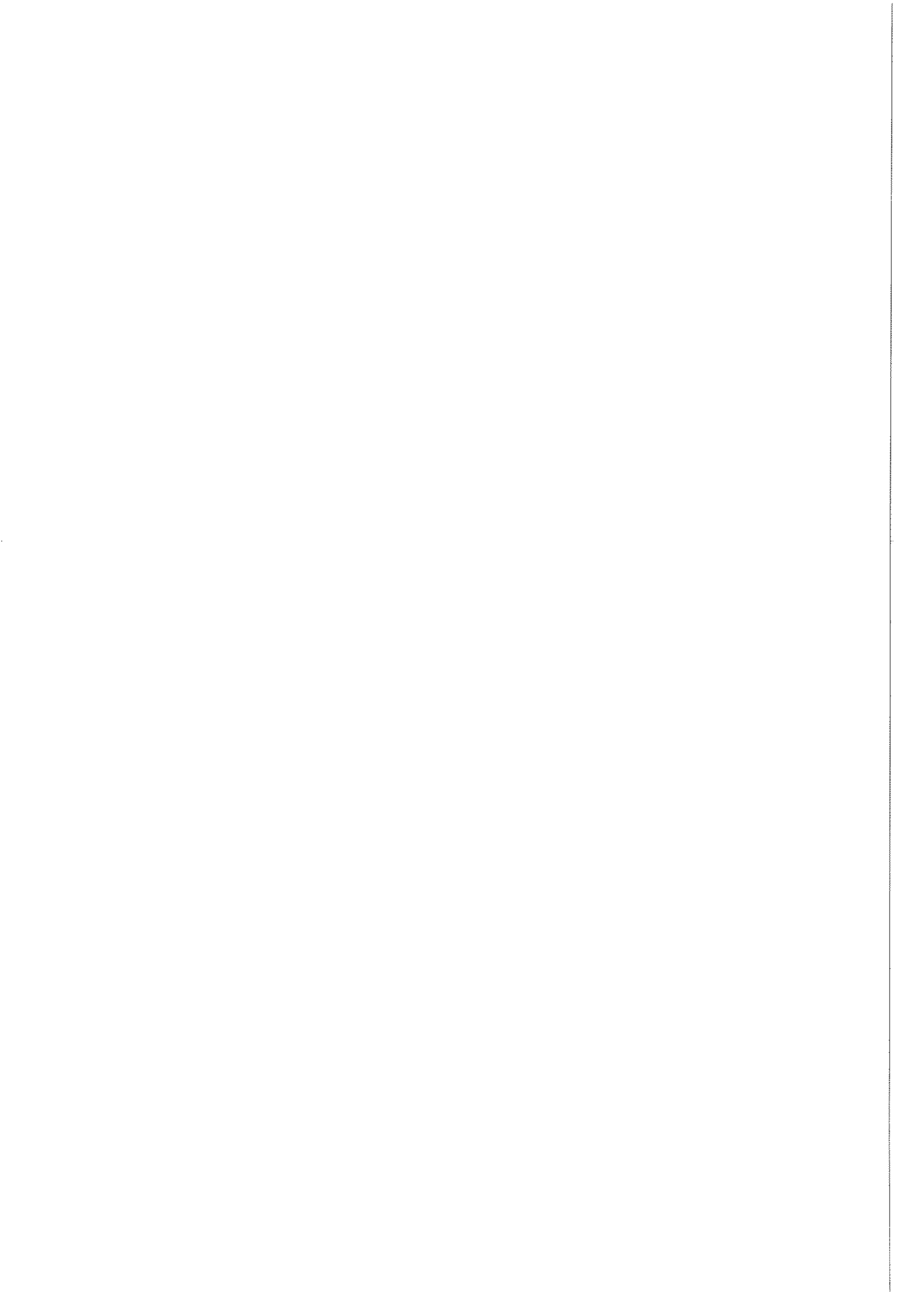
PRECAST STRUCTURES EC8

Dec 2005

Scale: 1:100
(except if indicated)

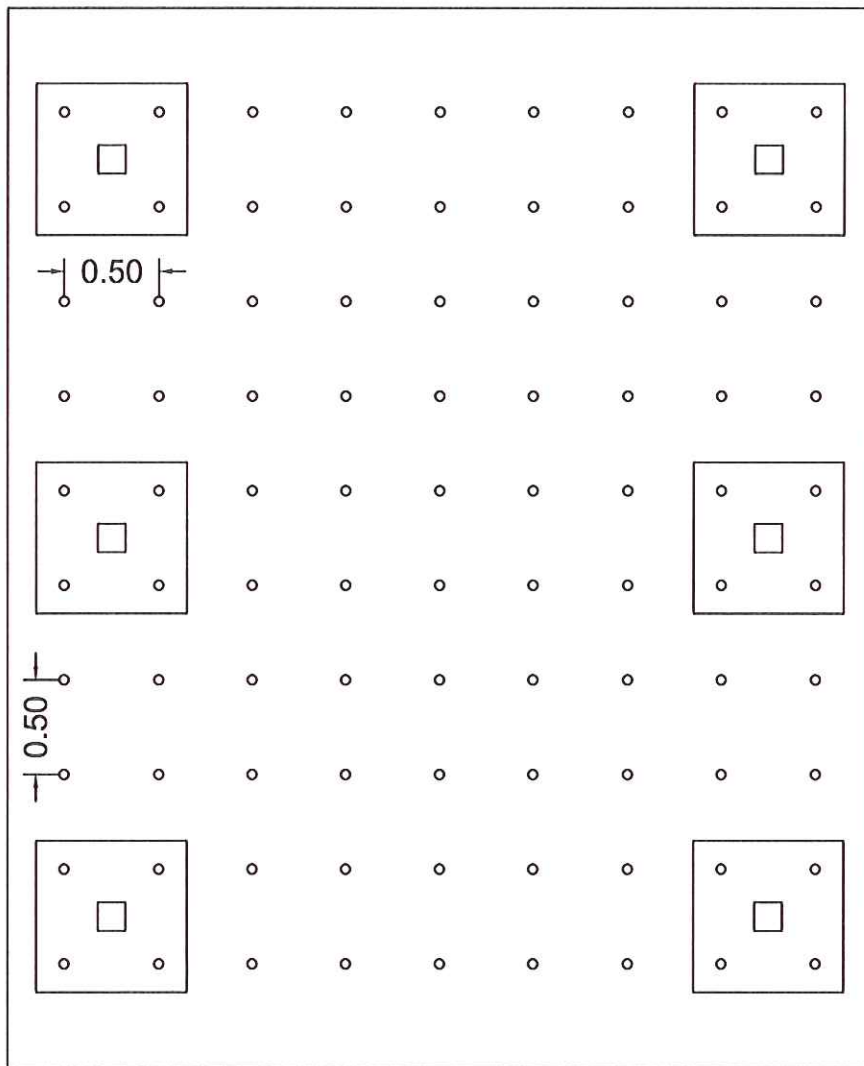
Model - Section C-D

Drawing N°
3



Foundations Plan

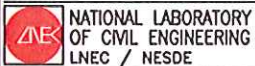
Scale: 1/40



Shaking Table Platform

Dimensions in meters

This drawing is based on the Civibril S.A. system drawings



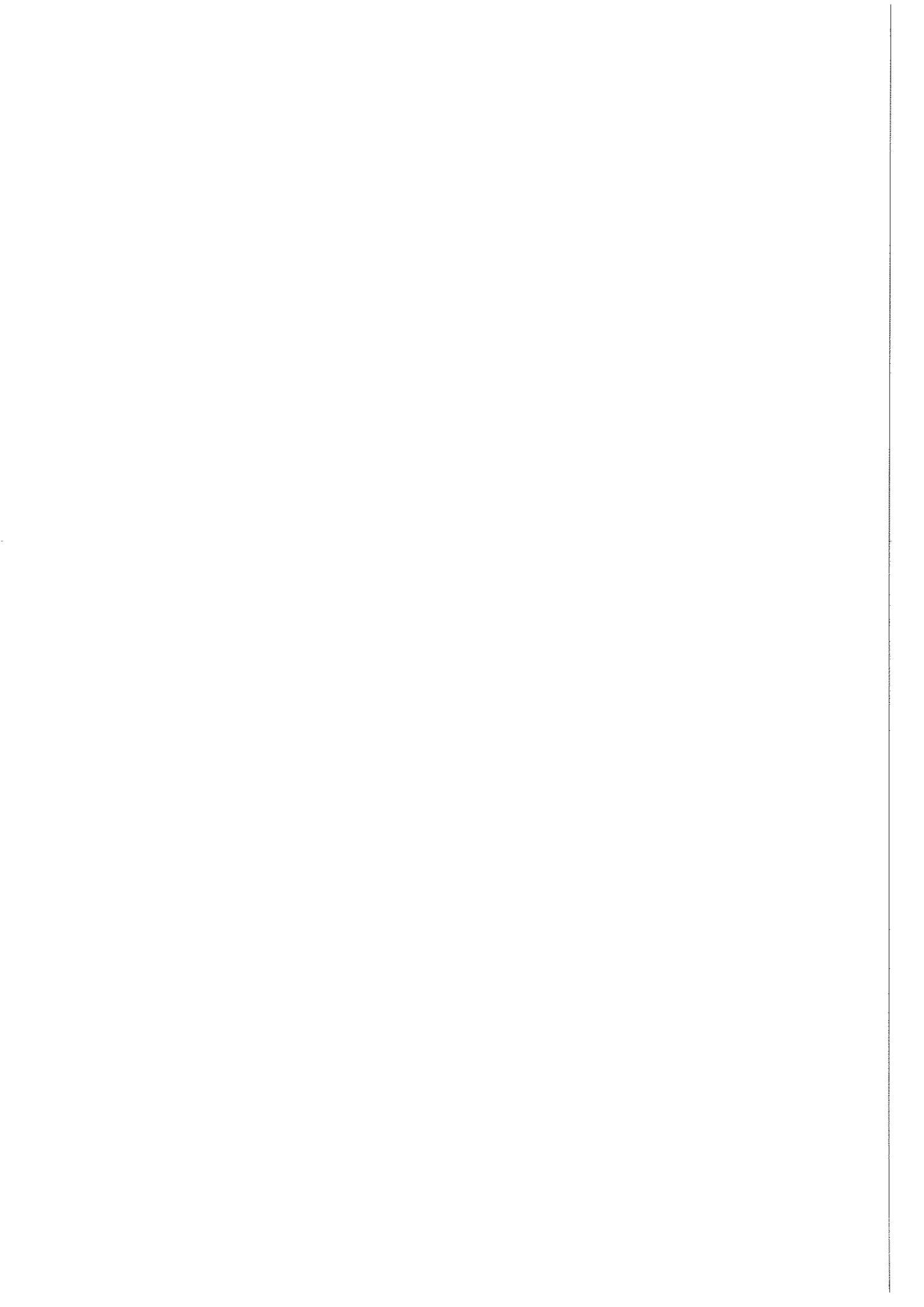
PRECAST STRUCTURES EC8

Dec 2005

Scale: 1:100
(except if indicated)

Model – Foundations Plan

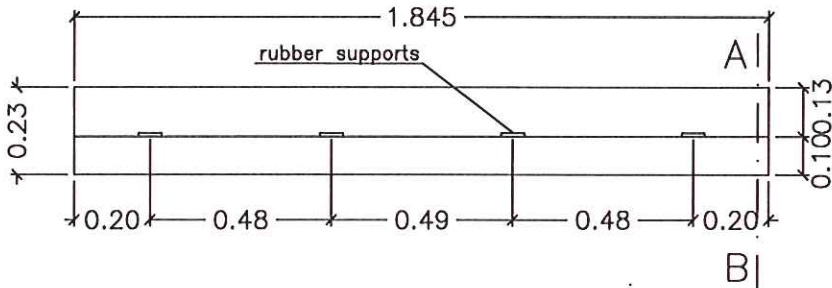
Drawing N°
4



Beam V1

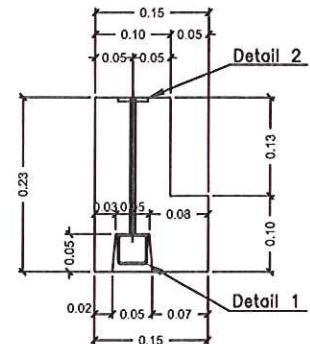
View

Scale: 1/20



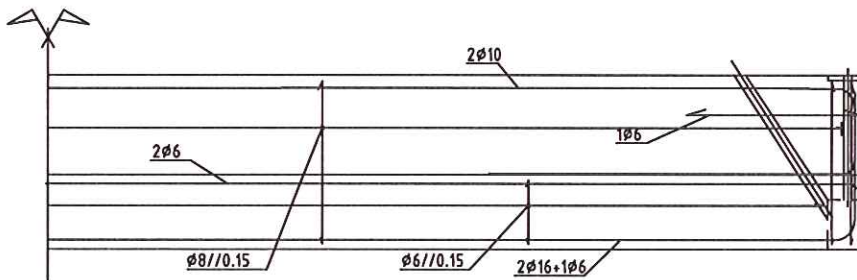
Section A-B

Scale: 1/10



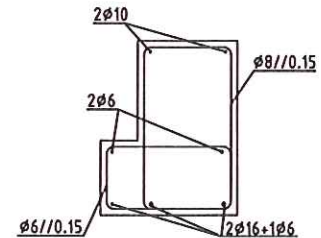
Details View

Scale: 1/10



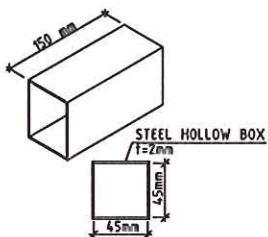
Details Section

Scale: 1/10



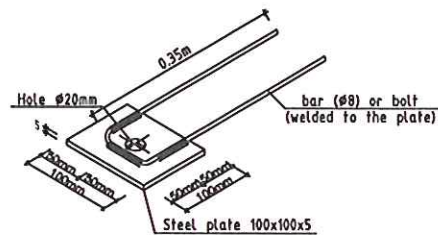
Detail 1

Scale: -



Detail 2

Scale: -

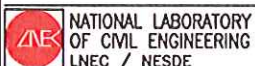


MATERIALS

Concrete:	
Prestressed Elements	C35/45
Other Elements	C25/30
Steel:	
Reinforcing Steel	A500NR
Welded Mesh	A500EL
Prestressing Steel	$F_{puK}=1860\text{Mpa}$
Cover:	2.5cm

Dimensions in meters

This drawing is based on the Civibril S.A. system drawings



PRECAST STRUCTURES EC8

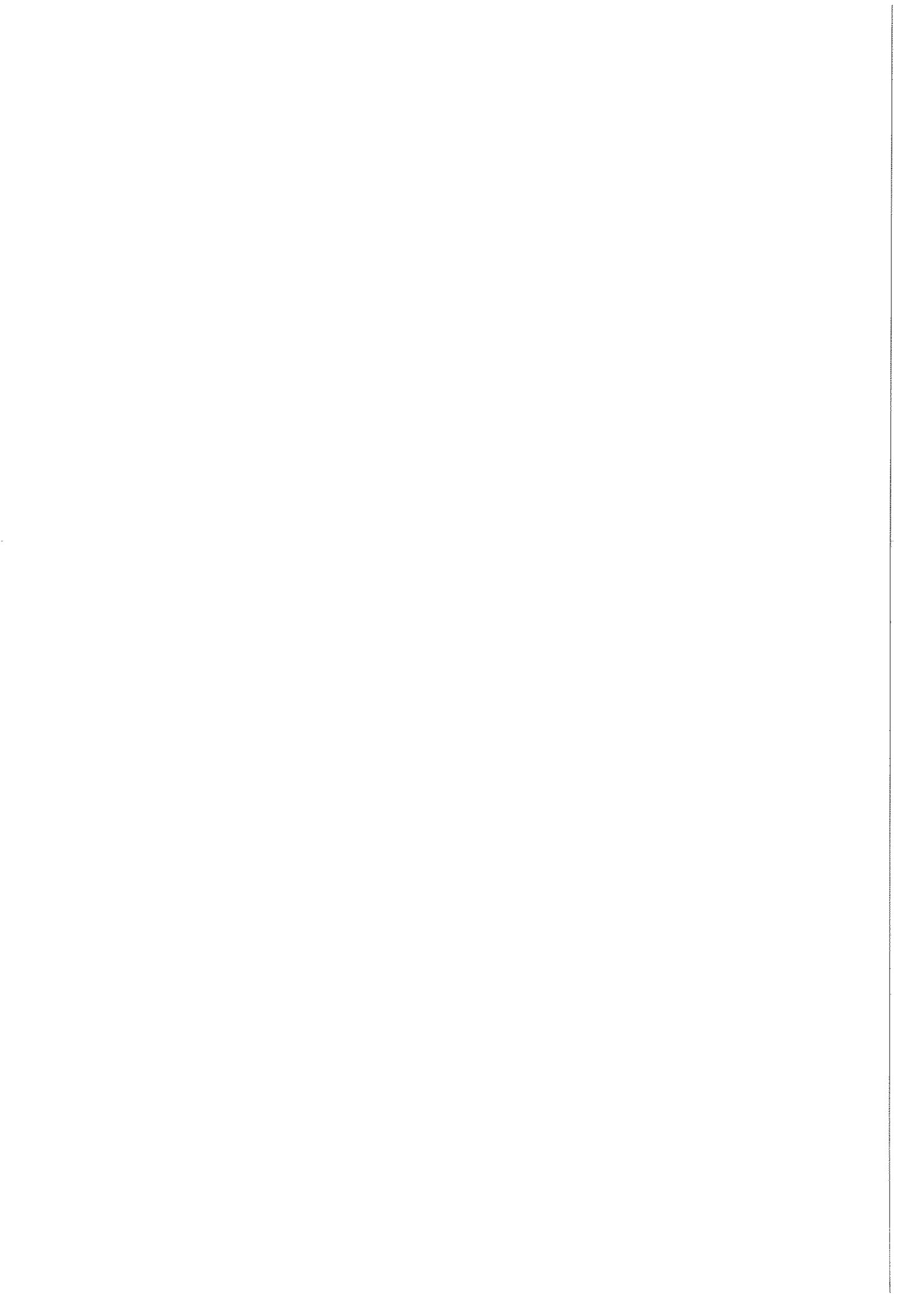
Dec 2005

Scale: 1:100
(except if indicated)

Model - Beam V1

Drawing N°

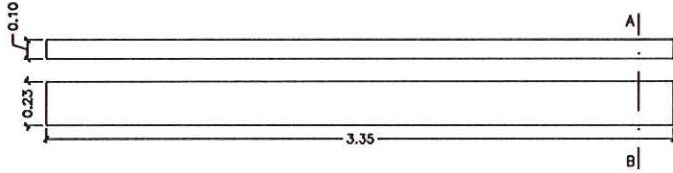
5



Beam V2

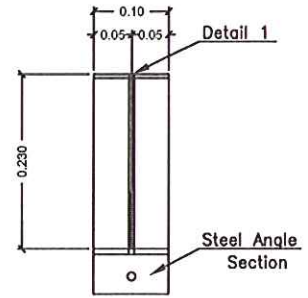
View

Scale: 1/40



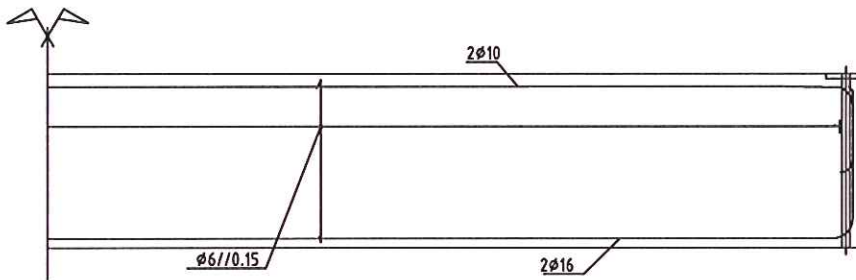
Section A-B

Scale: 1/10



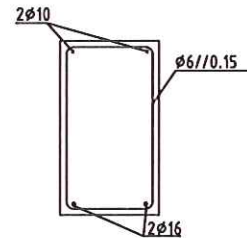
Details View

Scale: 1/10



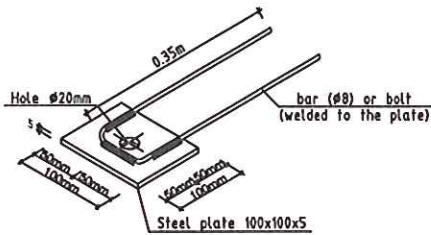
Details Section

Scale: 1/10



Detail 1


Scale: -



MATERIALS	
Concrete:	
Prestressed Elements	C35/45
Other Elements	C25/30
Steel:	
Reinforcing Steel	A500NR
Welded Mesh	A500EL
Prestressing Steel	$F_{puK}=1860\text{Mpa}$
Cover:	2.5cm

Dimensions in meters

This drawing is based on the Civibril S.A. system drawings

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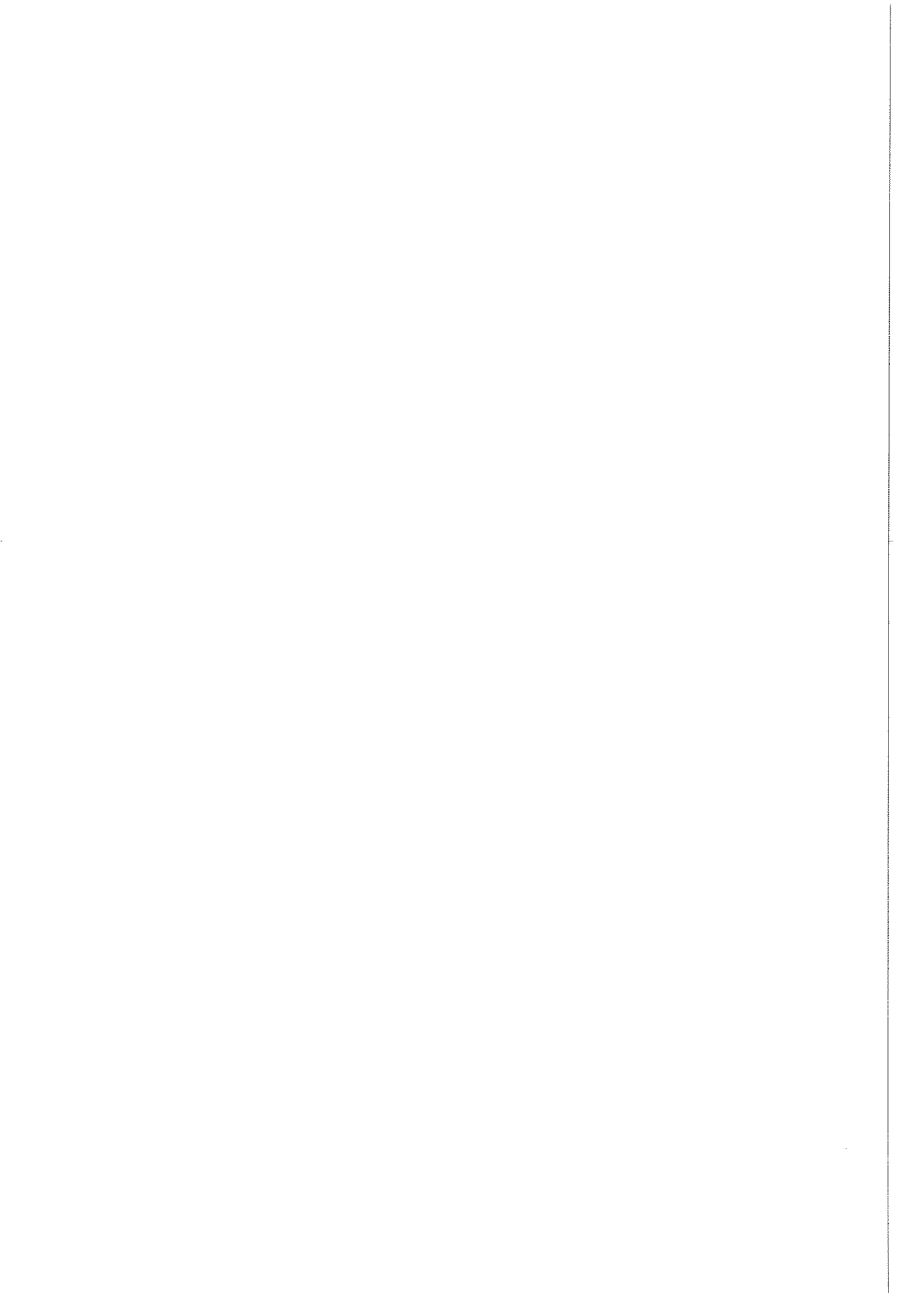
Scale: 1:100
(except if indicated)

PRECAST STRUCTURES EC8

Model - Beam V2

Dec 2005

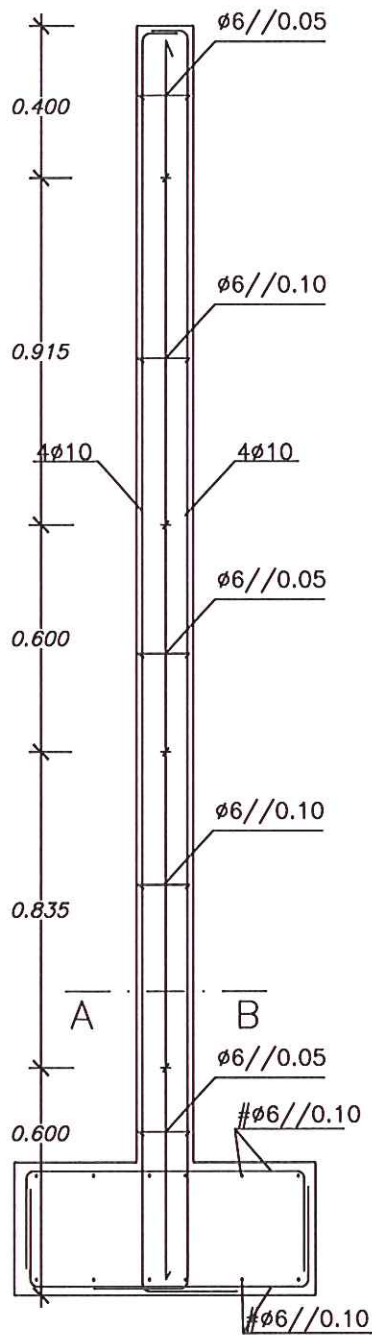
Drawing N°
6



Column P1

Details

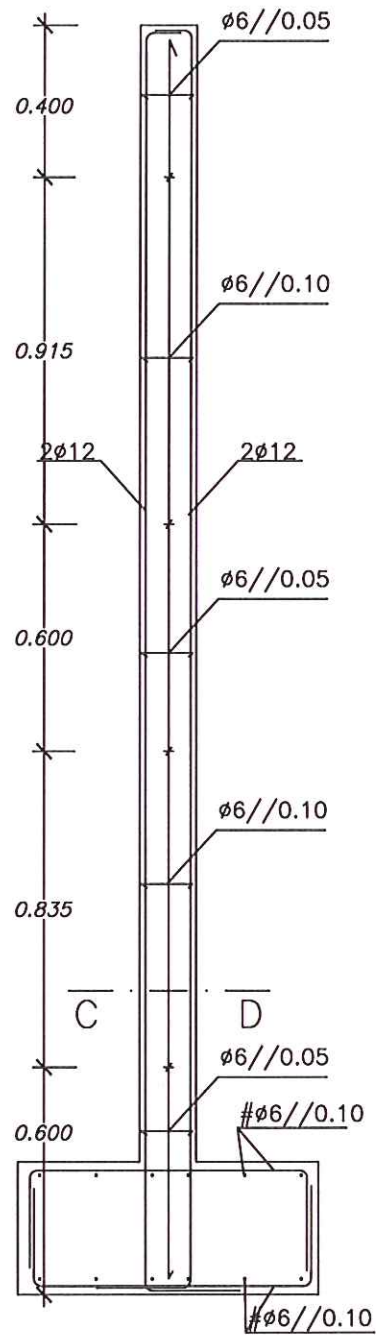
Scale: 1/20



Column P2

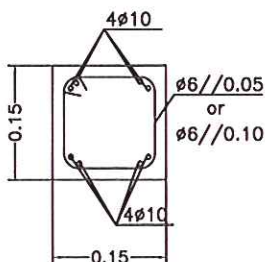
Details

Scale: 1/20



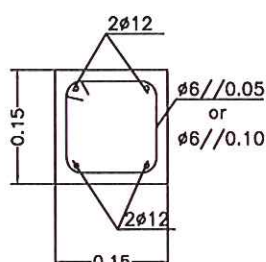
Section A-B

Scale: 1/10



Section C-D

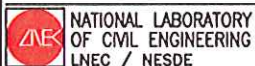
Scale: 1/10



MATERIALS	
Concrete:	
Prestressed Elements	C35/45
Other Elements	C25/30
Steel:	
Reinforcing Steel	A500NR
Welded Mesh	A500EL
Prestressing Steel	FpuK=1860Mpa
Cover:	2.5cm

Dimensions in meters

This drawing is based on the Civibril S.A. system drawings



PRECAST STRUCTURES EC8

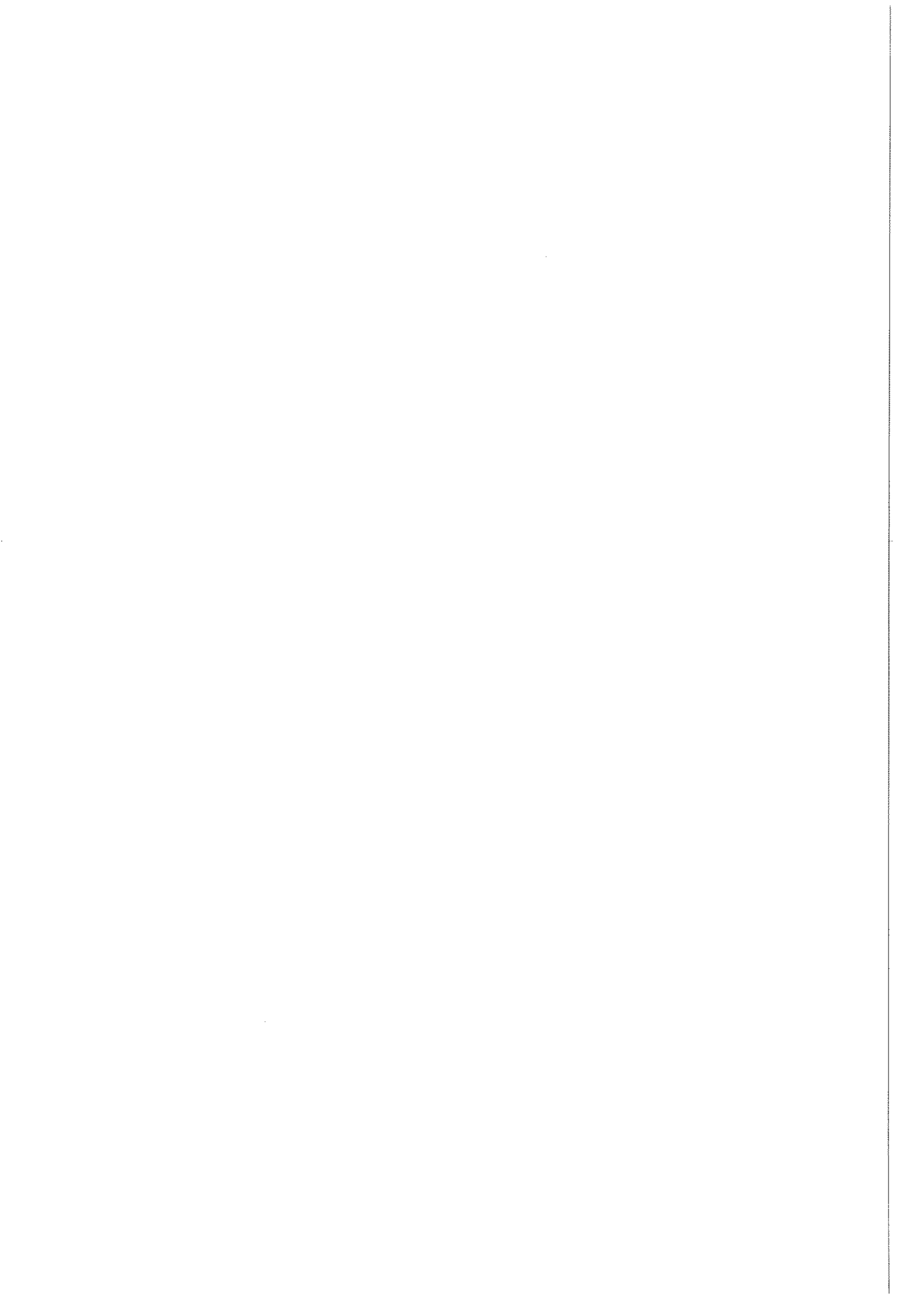
Dec 2005

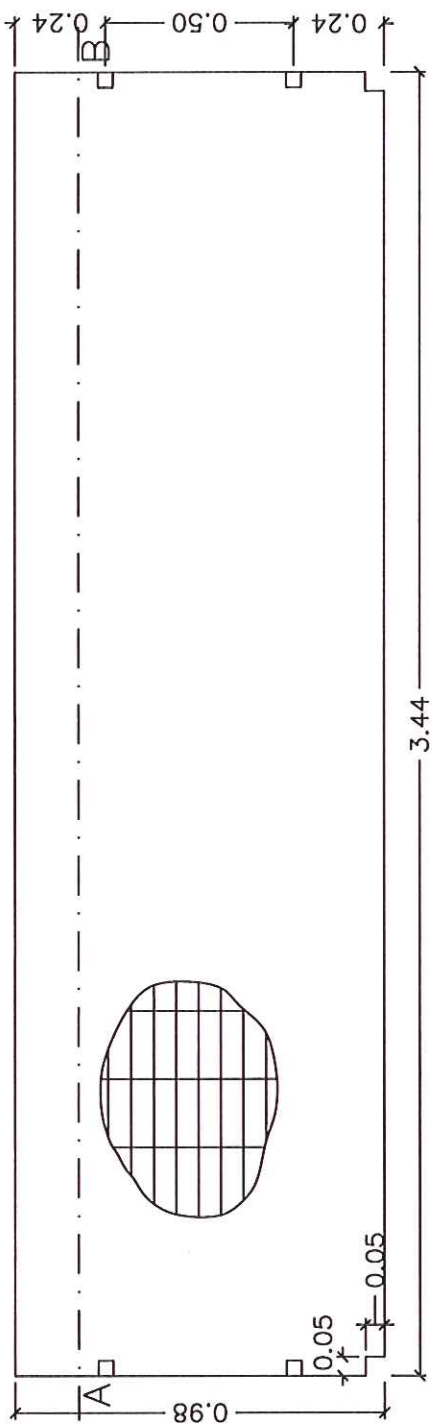
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(except if indicated)

Model – Columns

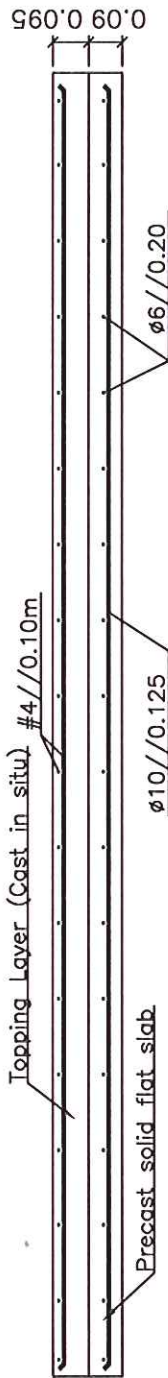
Drawing N°

7

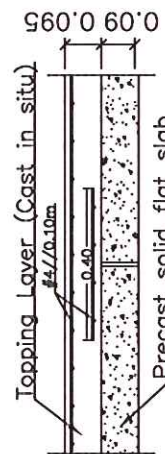




Slab
Plan View
Scale: 1/20



Section A-B
Scale: 1/20




Detail Between Slabs
Scale: 1/20

MATERIALS	
Concrete:	
Prestressed Elements	C35/45
Other Elements	C25/30
Steel:	
Reinforcing Steel	A500NR
Welded Mesh	A500EL
Prestressing Steel	FpuK=1860Mpa
Cover:	2.5cm

Dimensions in meters

This drawing is based on the Civibril S.A. system drawings

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Scale: 1:100
(except if Indicated)

PRECAST STRUCTURES EC8

Model - Slabs

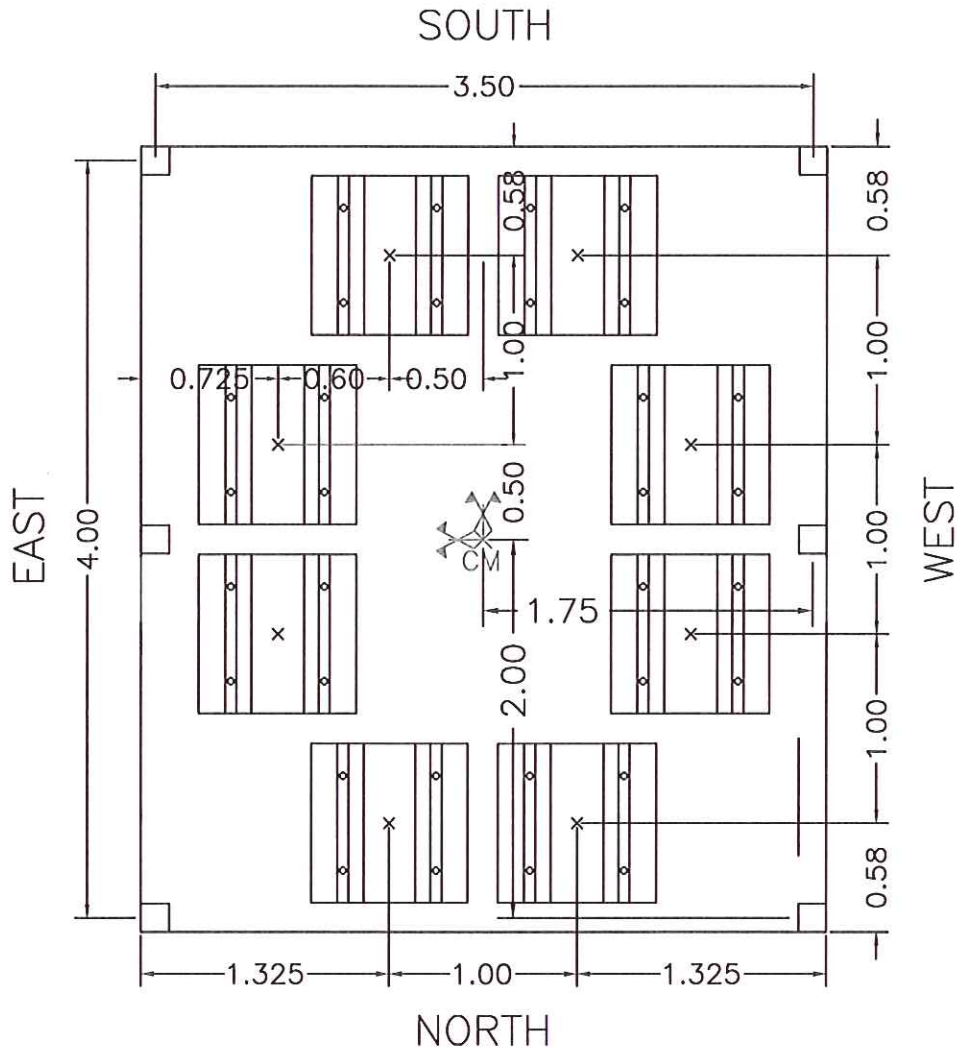
Dec 2005

Drawing N°
8

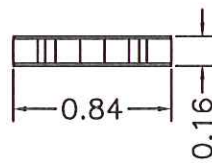
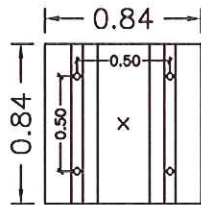


Plan View

Scale: 1/40

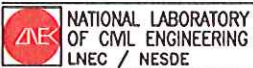


DIMENSIONS OF THE MASSES



Weight: 600 kgf

Dimensions in meters



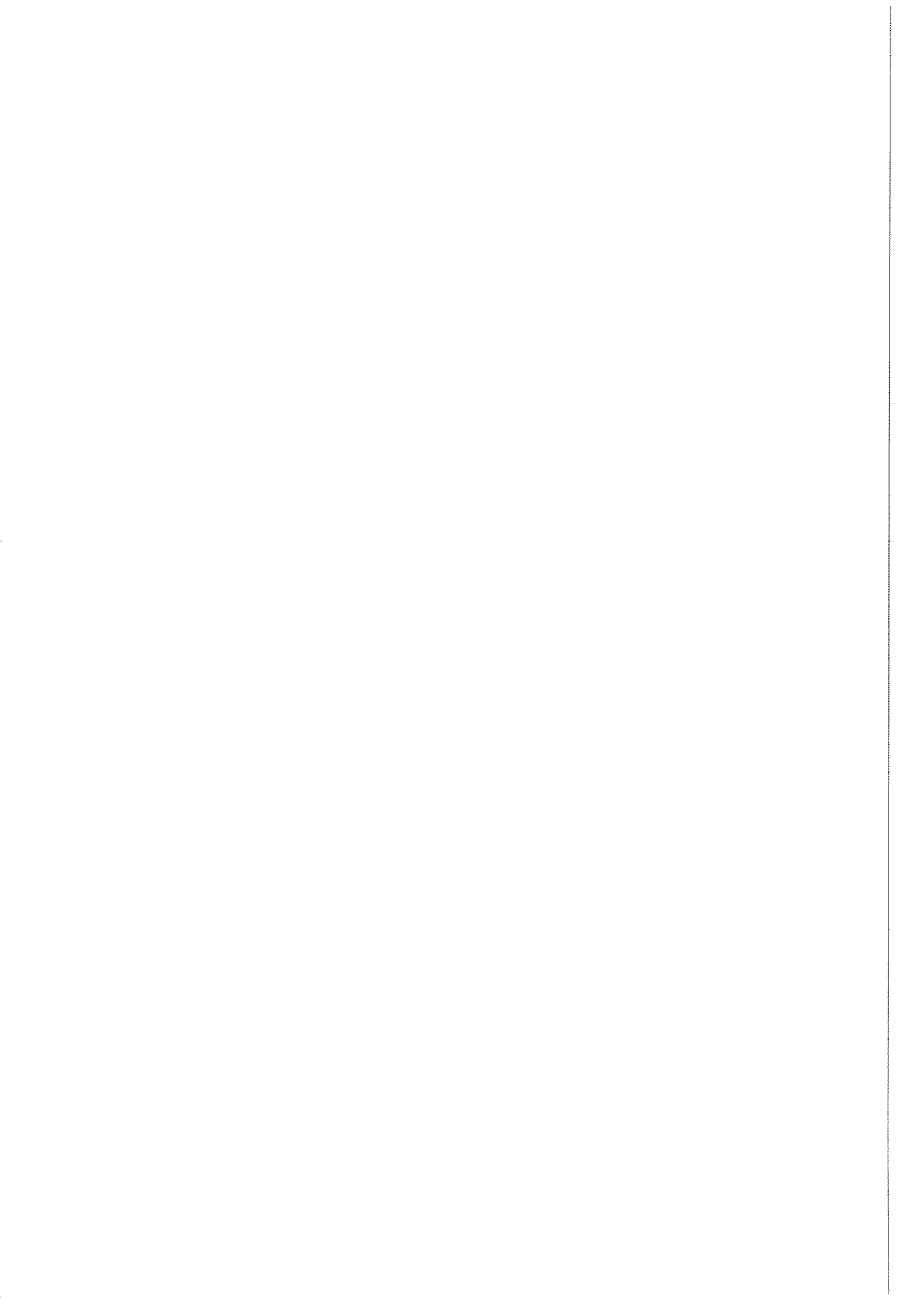
Scale: 1:100
(except if indicated)

PRECAST STRUCTURES EC8

Model – Position of the Additional Masses

Dec 2005

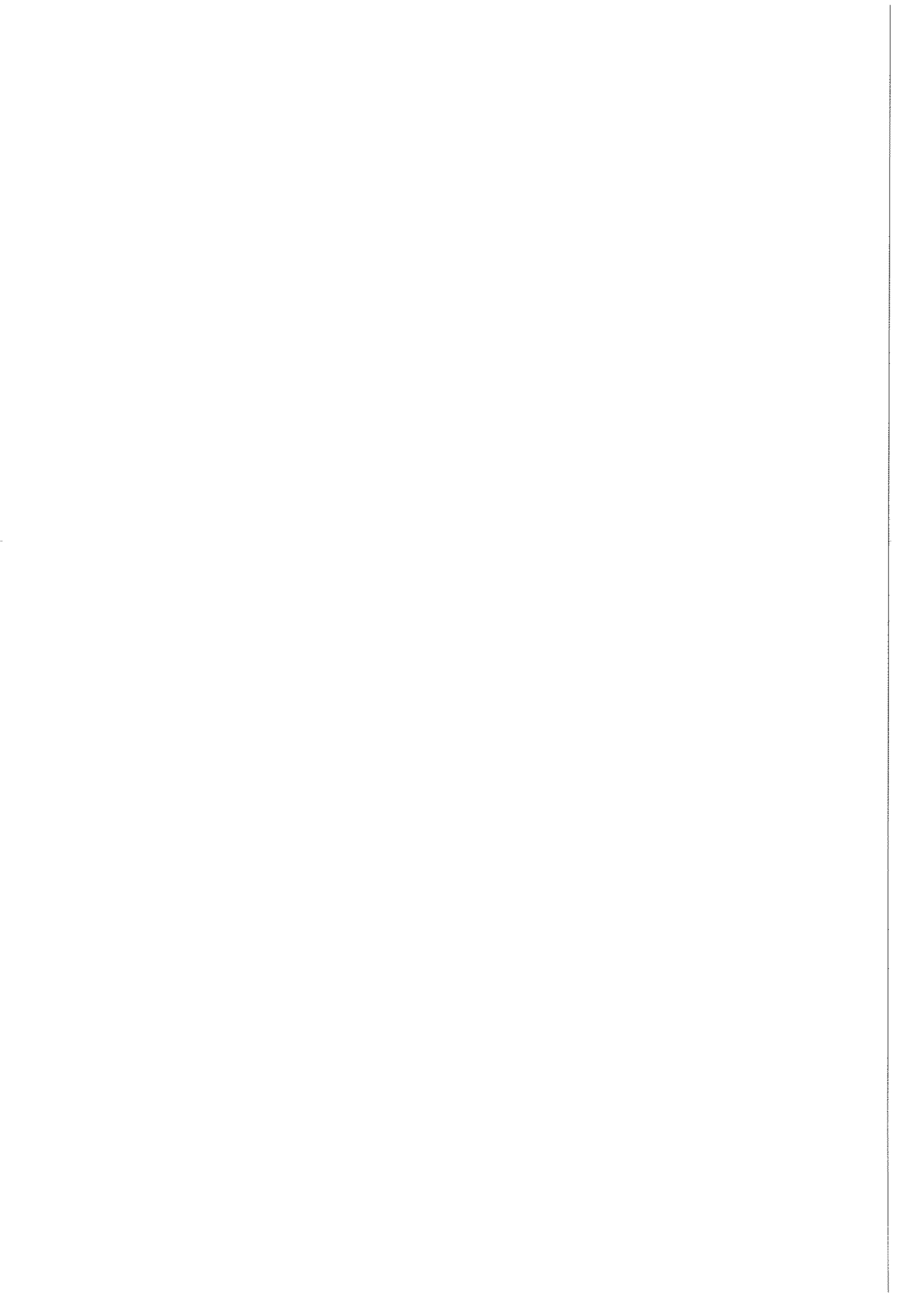
Drawing N°
9



ANNEX E INSTRUMENTATION AND TEST SETUP

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E.1 List of Channels

STAGE SERIES			CAT SERIES		
N°	Name (ID-POS-TYPE-DIR)	Units	N°	Name (ID-POS-TYPE-DIR)	Units
0	ST1-STNE-Acc-Trans	g	0	ST1-STNE-Acc-Trans	g
1	ST2-STNE-Acc-Long	g	1	ST2-STNE-Acc-Long	g
2	ST3-STNW-Acc-Vert	g	2	ST3-STNW-Acc-Vert	g
3	ST4-STSE-Acc-Vert	g	3	ST4-STSE-Acc-Vert	g
4	ST5-ST-ADisp-Trans	mm	4	ST5-ST-ADisp-Trans	mm
5	ST6-ST-ADisp-Long	mm	5	ST6-ST-ADisp-Long	mm
6	DA1-1FNW-ADisp-Trans	mm	6	A1-2FNE-Acc-Trans	g
7	DA1-1FNW-ADisp-Long	mm	7	A2-2FNE-Acc-Long	g
8	DA2-1FSE-ADisp-Trans	mm	8	A3-2FE-Acc-Trans	g
9	DA2-1FSE-ADisp-Long	mm	9	A4-2FE-Acc-Long	g
10	DA3-2FNE-ADisp-Trans	mm	10	A5-2FSE-Acc-Trans	g
11	DA3-2FNE-ADisp-Long	mm	11	A6-2FSE-Acc-Long	g
12	DA4-2FNW-ADisp-Trans	mm	12	A7-2FSW-Acc-Trans	g
13	DA4-2FNW-ADisp-Long	mm	13	A8-2FSW-Acc-Long	g
14	DA5-2FSE-ADisp-Trans	mm	14	A9-2FW-Acc-Trans	g
15	DA5-2FSE-ADisp-Long	mm	15	A10-2FW-Acc-Long	g
16	DA6-2FSW-ADisp-Trans	mm	16	A11-2FNW-Acc-Trans	g
17	DA6-2FSW-ADisp-Long	mm	17	A12-2FNW-Acc-Long	g
18	DR21-1FNE-RDisp-Trans	mm	18	A13-1FNE-Acc-Trans	g
19	DR22-1FNE-RDisp-Long	mm	19	A14-1FNE-Acc-Long	g
20	DR23-1FSW-RDisp-Trans	mm	20	A15-1FE-Acc-Trans	g
21	DR24-1FSW-RDisp-Long	mm	21	A16-1FE-Acc-Long	g
22	DR1-1FNE-RDisp	mm	22	A17-1FSE-Acc-Trans	g
23	DR2-1FNW-RDisp	mm	23	A18-1FSE-Acc-Long	g
24	DR3-1FSE-RDisp	mm	24	A19-1FSW-Acc-Trans	g
25	DR4-1FSW-RDisp	mm	25	A20-1FSW-Acc-Long	g
26	A1-2FNE-Acc-Trans	g	26	A21-1FW-Acc-Trans	g
27	A2-2FNE-Acc-Long	g	27	A22-1FW-Acc-Long	g
28	A3-2FE-Acc-Trans	g	28	A23-1FNW-Acc-Trans	g
29	A4-2FE-Acc-Long	g	29	A24-1FNW-Acc-Long	g
30	A5-2FSE-Acc-Trans	g	30	A25-2FNN-Acc-Vert	g
31	A6-2FSE-Acc-Long	g	31	A26-2FN-Acc-Vert	g
32	A7-2FSW-Acc-Trans	g	32	A27-2FC-Acc-Vert	g
33	A8-2FSW-Acc-Long	g			
34	A9-2FW-Acc-Trans	g			
35	A10-2FW-Acc-Long	g			
36	A11-2FNW-Acc-Trans	g			
37	A12-2FNW-Acc-Long	g			
38	A13-1FNE-Acc-Trans	g			
39	A14-1FNE-Acc-Long	g			
40	A15-1FE-Acc-Trans	g			
41	A16-1FE-Acc-Long	g			
42	A17-1FSE-Acc-Trans	g			
43	A18-1FSE-Acc-Long	g			
44	A19-1FSW-Acc-Trans	g			
45	A20-1FSW-Acc-Long	g			

46 A21-1FW-Acc-Trans	g
47 A22-1FW-Acc-Long	g
48 A23-1FNW-Acc-Trans	g
49 A24-1FNW-Acc-Long	g
50 A25-2FNN-Acc-Vert	g
51 A26-2FN-Acc-Vert	g
52 A27-2FC-Acc-Vert	g
53 DR5-1FNE-RDisp-BTrans	mm
54 DR6-1FNE-RDisp-TTrans	mm
55 DR7-2FNE-RDisp-BTrans	mm
56 DR8-2FNE-RDisp-TTrans	mm
57 DR9-1FNE-RDisp-BLong	mm
58 DR10-1FNE-RDisp-TLong	mm
59 DR11-2FNE-RDisp-BLong	mm
60 DR12-2FNE-RDisp-TLong	mm
61 DR13-1FE-RDisp-BLong	mm
62 DR14-1FE-RDisp-TLong	mm
63 DR15-2FE-RDisp-BLong	mm
64 DR16-2FE-RDisp-TLong	mm
65 DR17-1FE-RDisp-BLong	mm
66 DR18-1FE-RDisp-TLong	mm
67 DR19-2FE-RDisp-BLong	mm
68 DR20-2FE-RDisp-TLong	mm
69 MC DA1-1FNW-RDisp-Trans	mm
70 MC DA1-1FNW-RDisp-Long	mm
71 MC DA2-1FSE-RDisp-Trans	mm
72 MC DA2-1FSE-RDisp-Long	mm
73 MC DA3-2FNE-RDisp-Trans	mm
74 MC DA3-2FNE-RDisp-Long	mm
75 MC DA4-2FNW-RDisp-Trans	mm
76 MC DA4-2FNW-RDisp-Long	mm
77 MC DA5-2FSE-RDisp-Trans	mm
78 MC DA5-2FSE-RDisp-Long	mm
79 MC DA6-2FSW-RDisp-Trans	mm
80 MC DA6-2FSW-RDisp-Long	mm
81 MC DA7-1FNE-ADisp-Trans	mm
82 MC DA7-1FNE-ADisp-Long	mm
83 MC DA8-1FSW-ADisp-Trans	mm
84 MC DA8-1FSW-ADisp-Long	mm
85 MC DA7-1FNE-RDisp-Trans	mm
86 MC DA7-1FNE-RDisp-Long	mm
87 MC DA8-1FSW-RDisp-Trans	mm
88 MC DA8-1FSW-RDisp-Long	mm
89 MC RD1-2FNE-Elong-Diag	%
90 MC RD2-2FNW-Elong-Diag	%
91 MC RD3-2FSE-Elong-Diag	%
92 MC RD4-2FSW-Elong-Diag	%
93 MC ID1-1FNW-ID-Trans	%
94 MC ID1-1FNW-ID-Long	%
95 MC ID2-1FSE-ID-Trans	%
96 MC ID2-1FSE-ID-Long	%
97 MC ID3-1FNE-ID-Trans	%

98 MC ID3-1FNE-ID-Long	%
99 MC ID4-1FSW-ID-Trans	%
100 MC ID4-1FSW-ID-Long	%
101 MC ID5-2FNE-ID-Trans	%
102 MC ID5-2FNE-ID-Long	%
103 MC ID6-2FNW-ID-Trans	%
104 MC ID6-2FNW-ID-Long	%
105 MC ID7-2FSE-ID-Trans	%
106 MC ID7-2FSE-ID-Long	%
107 MC ID8-2FSW-ID-Trans	%
108 MC ID8-2FSW-ID-Long	%
109 MC J3-1FNE-RDisp-Trans	mm
110 MC J3-1FNE-Rot-Trans	rad
111 MC J9-2FNE-RDisp-Trans	mm
112 MC J9-2FNE-Rot-Trans	rad
113 MC J3-1FNE-RDisp-Long	mm
114 MC J3-1FNE-Rot-Long	rad
115 MC J9-2FNE-RDisp-Long	mm
116 MC J9-2FNE-Rot-Long	rad
117 MC J2-1FEN-RDisp-Long	mm
118 MC J2-1FEN-Rot-Long	rad
119 MC J8-2FEN-RDisp-Long	mm
120 MC J8-2FEN-Rot-Long	rad
121 MC J2-1FES-RDisp-Long	mm
122 MC J2-1FES-Rot-Long	rad
123 MC J8-2FES-RDisp-Long	mm
124 MC J8-2FES-Rot-Long	rad
125 MC CM1-CM1F-RDisp-Trans	mm
126 MC CM2-CM1F-RDisp-Long	mm
127 MC CM3-CM1F-RDisp-Rot	rad
128 MC CM4-CM2F-RDisp-Trans	mm
129 MC CM5-CM2F-RDisp-Long	mm
130 MC CM6-CM2F-RDisp-Rot	rad
131 MC CM7-CM1F-ADisp-Trans	g
132 MC CM8-CM1F-ADisp-Long	g
133 MC CM9-CM1F-ADisp-Rot	rad/s ²
134 MC CM10-CM2F-ADisp-Trans	g
135 MC CM11-CM2F-ADisp-Long	g
136 MC CM12-CM2F-ADisp-Rot	rad/s ²
137 MC AAV1-CM1F-AccAvg-Trans	g
138 MC AAV2-CM1F-AccAvg-Long	g
139 MC AAV3-CM1F-RotAccAvg-Trans	rad/s ²
140 MC AAV3-CM1F-RotAccAvg-Long	rad/s ²
141 MC AAV3-CM1F-RotAccAvg-DirAvg	rad/s ²
142 MC AAV4-CM2F-AccAvg-Trans	g
143 MC AAV5-CM2F-AccAvg-Long	g
144 MC AAV6-CM2F-RotAccAvg-Trans	rad/s ²
145 MC AAV6-CM2F-RotAccAvg-Long	rad/s ²
146 MC AAV6-CM2F-RotAccAvg-DirAvg	rad/s ²
147 MC IF1-DOF1-X1	kN
148 MC IF2-DOF1-X2	kN
149 MC IF3-DOF1-Tor	kN.m

150 MC IF4-DOF2-X1	kN
151 MC IF5-DOF2-X2	kN
152 MC IF6-DOF2-Tor	kN.m
153 MC SF1-DOF1-X1	kN
154 MC SF2-DOF1-X2	kN
155 MC SF3-DOF1-Tor	kN.m
156 MC SF4-DOF2-X1	kN
157 MC SF5-DOF2-X2	kN
158 MC SF6-DOF2-Tor	kN.m
159 MC BS1-ST-X1	kN
160 MC BS2-ST-X2	kN
161 MC BT-ST-Tor	kN.m
162 MC BOM1-ST-X1	kN.m
163 MC BOM2-ST-X2	kN.m
164 MC ID9-1FCM-ID-Trans	%
165 MC ID9-1FCM-ID-Long	%
166 MC ID10-2FCM-ID-Trans	%
167 MC ID10-2FCM-ID-Long	%
168 MC CM13-CM2F-CM1F-RDisp-Trans	mm
169 MC CM14-CM2F-CM1F-RDisp-Long	mm

Name Conventions:

MC-Math Channel;

POS: ST-Base Shaking Table; 1F-First Floor; 2F-Second Floor; CM-Center of Mass; DOF-Degree of freedom;
NW-Northwest etc

TYPE: Acc-Acceleration; ADisp-Absolute Displacement; RDisp-Relative Displacement;
ID-Interstorey Drift; Rot-Rotation; Avg - Average; IF-Inertia Forces; SF-Storey Forces; BS-Base Shear
BT-Base Torsion; BOM-Base Overturning Moment; EG-Energy

DIR: Trans=X1-Transversal direction; Long=X2-Longitudinal direction; Vert=Tor-Vertical direction

E.2 Sensors Technical Data


	<p>Manufacture: PCB PIEZOTRONICS (www.pcb.com) Model: 337A26 Sensitivity: 100 mV/g Measurement range: ± 50 g pk Broadband Resolution: 0.0001 g rms Frequency range: 0.5 to 5000 Hz</p>
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Table E-1: Piezoelectric accelerometers

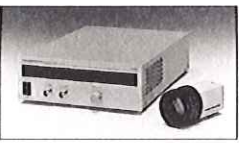
	<p>Manufacture: HAMAMATSU PHOTONICS (www.hamamatsu.com) Model: C5949 and C2399-00 Measurement points: 1 to 7 Spectral response: 700 to 1150 nm Sampling frequency: 300 Hz Position detection error: $\pm 1\%$ Error due to light: $\pm 1\%$</p>
-----------------------------------------------------------------------------------	--------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------

Table E-2: Optical displacement transducers



	<p>Manufacture: RDP ELECTRONICS (www.rdpe.com) Model: ACT2000, ACT4000 and ACT6000 Stroke: ± 50mm (ACT2000); ± 100mm (ACT4000); ± 150mm (ACT6000) Sensitivity: 15 mV/V/mm (ACT6000) to 30 mV/V/mm (ACT2000) Energising supply: 5 Vrms, 5 kHz Linearity deviation: 0.08% (ACT2000) to 0.3% (ACT6000)</p>
	<p>Manufacture: HOTTINGER BALDWIN M. (www.hbm.com) Model: WA300 and WA500 Sensitivity: 80 mV/V Linearity deviation: $\pm 0.1\%$ to $\pm 0.2\%$ Frequency range: 5 to 65 Hz</p>

Table E-3: Inductive displacement transducers

E.3 Math Channels Calculations

E.3.1 Relative Displacements

MATH CHANNELS REPORT
LNEC-SPA - Signal Processing and Analysis Program
File: 1-MC DA1_to_DA6-RDisp.bin

USED CHANNELS

- 4 - ST5-ST-ADisp-Trans
- 5 - ST6-ST-ADisp-Long
- 6 - DA1-1FNW-ADisp-Trans
- 7 - DA1-1FNW-ADisp-Long
- 8 - DA2-1FSE-ADisp-Trans
- 9 - DA2-1FSE-ADisp-Long
- 10 - DA3-2FNE-ADisp-Trans
- 11 - DA3-2FNE-ADisp-Long
- 12 - DA4-2FNW-ADisp-Trans
- 13 - DA4-2FNW-ADisp-Long
- 14 - DA5-2FSE-ADisp-Trans
- 15 - DA5-2FSE-ADisp-Long
- 16 - DA6-2FSW-ADisp-Trans
- 17 - DA6-2FSW-ADisp-Long

LEVEL 0 - Type: Operations between channels

OPERATIONS

- 0 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 4,6,; Parameters:
- 1 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 5,7,; Parameters:
- 2 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 4,8,; Parameters:
- 3 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 5,9,; Parameters:
- 4 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 4,10,; Parameters:
- 5 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 5,11,; Parameters:
- 6 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 4,12,; Parameters:
- 7 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 5,13,; Parameters:
- 8 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 4,14,; Parameters:
- 9 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 5,15,; Parameters:
- 10 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 4,16,;

Parameters:

- 11 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 5,17,;

Parameters:

CHANNELS OUT NAMES

- 0 - MC DA1-1FNW-RDisp-Trans
- 1 - MC DA1-1FNW-RDisp-Long
- 2 - MC DA2-1FSE-RDisp-Trans
- 3 - MC DA2-1FSE-RDisp-Long
- 4 - MC DA3-2FNE-RDisp-Trans
- 5 - MC DA3-2FNE-RDisp-Long
- 6 - MC DA4-2FNW-RDisp-Trans
- 7 - MC DA4-2FNW-RDisp-Long
- 8 - MC DA5-2FSE-RDisp-Trans
- 9 - MC DA5-2FSE-RDisp-Long
- 10 - MC DA6-2FSW-RDisp-Trans
- 11 - MC DA6-2FSW-RDisp-Long

CHANNELS OUT UNITS

- mm
- mm
- mm
- mm
- mm
- mm
- mm
- mm
- mm
- mm
- mm
- mm

E.3.2 DA7 & DA8 Absolute and Relative Displacements

MATH CHANNELS REPORT

LNEC-SPA - Signal Processing and Analysis Program

File: 2-MC DA7_to_DA8-ADisp&RDisp.bin

USED CHANNELS

4 - ST5-ST-ADisp-Trans
5 - ST6-ST-ADisp-Long
18 - DR21-1FNE-RDisp-Trans
19 - DR22-1FNE-RDisp-Long
20 - DR23-1FSW-RDisp-Trans
21 - DR24-1FSW-RDisp-Long

LEVEL 0 - Type: Operations to individual channels

OPERATIONS

0 - ID: Add scalar; Channels n°: 18,; Parameters: 3.764070,
1 - ID: Add scalar; Channels n°: 19,; Parameters: 2.432740,
2 - ID: Add scalar; Channels n°: 20,; Parameters: 3.764070,
3 - ID: Add scalar; Channels n°: 21,; Parameters: 2.432740,
4 - ID: Unchanged; Channels n°: 4,; Parameters: 1.000000,
5 - ID: Unchanged; Channels n°: 5,; Parameters: 1.000000,

CHANNELS OUT NAMES

0 - DR21-1FNE-RDisp-Trans (Add scalar)
1 - DR22-1FNE-RDisp-Long (Add scalar)
2 - DR23-1FSW-RDisp-Trans (Add scalar)
3 - DR24-1FSW-RDisp-Long (Add scalar)
4 - ST-BS-ADisp-Trans (Unchanged)
5 - ST-BS-ADisp-Long (Unchanged)

LEVEL 1 - Type: Operations to individual channels

OPERATIONS

0 - ID: Multiply by scalar; Channels n°: 0,; Parameters: 0.929841,
1 - ID: Multiply by scalar; Channels n°: 1,; Parameters: 0.822144,
2 - ID: Multiply by scalar; Channels n°: 2,; Parameters: 0.929841,
3 - ID: Multiply by scalar; Channels n°: 3,; Parameters: 0.822144,
4 - ID: Multiply by scalar; Channels n°: 4,; Parameters: 1.000000,
5 - ID: Multiply by scalar; Channels n°: 5,; Parameters: 1.000000,

CHANNELS OUT NAMES

0 - DR21-1FNE-RDisp-Trans (Add scalar) (Multiply by scalar)
1 - DR22-1FNE-RDisp-Long (Add scalar) (Multiply by scalar)
2 - DR23-1FSW-RDisp-Trans (Add scalar) (Multiply by scalar)
3 - DR24-1FSW-RDisp-Long (Add scalar) (Multiply by scalar)
4 - ST-BS-ADisp-Trans (Unchanged) (Multiply by scalar)
5 - ST-BS-ADisp-Long (Unchanged) (Multiply by scalar)

LEVEL 2 - Type: Operations to individual channels

OPERATIONS

3.500000,
2.000000,
3.500000,
2.000000,
4 - ID: Multiply by scalar; Channels n°: 4,; Parameters: 1.000000,
5 - ID: Multiply by scalar; Channels n°: 5,; Parameters: 1.000000,

CHANNELS OUT NAMES

0 - DR21-1FNE-RDisp-Trans (Add scalar) (Multiply by scalar) (Subtract scalar
(chan-scalar))
1 - DR22-1FNE-RDisp-Long (Add scalar) (Multiply by scalar) (Subtract scalar
(chan-scalar))
2 - DR23-1FSW-RDisp-Trans (Add scalar) (Multiply by scalar) (Subtract scalar
(chan-scalar))
3 - DR24-1FSW-RDisp-Long (Add scalar) (Multiply by scalar) (Subtract scalar
(chan-scalar))
4 - ST-BS-ADisp-Trans (Unchanged) (Multiply by scalar) (Multiply by scalar)
5 - ST-BS-ADisp-Long (Unchanged) (Multiply by scalar) (Multiply by scalar)

LEVEL 3 - Type: Operations to individual channels

OPERATIONS

0 - ID: Unchanged; Channels n°: 0,; Parameters:
1 - ID: Multiply by scalar; Channels n°: 1,; Parameters: -1.000000,
2 - ID: Multiply by scalar; Channels n°: 2,; Parameters: -1.000000,
3 - ID: Unchanged; Channels n°: 3,; Parameters:

4 - ID: Unchanged; Channels n°: 4,; Parameters:
5 - ID: Unchanged; Channels n°: 5,; Parameters:

CHANNELS OUT NAMES

0 - DR21-1FNE-RDisp-Trans (Add scalar) (Multiply by scalar) (Subtract scalar (chan-scalar)) (Unchanged)
1 - DR22-1FNE-RDisp-Long (Add scalar) (Multiply by scalar) (Subtract scalar (chan-scalar)) (Multiply by scalar)
2 - DR23-1FSW-RDisp-Trans (Add scalar) (Multiply by scalar) (Subtract scalar (chan-scalar)) (Multiply by scalar)
3 - DR24-1FSW-RDisp-Long (Add scalar) (Multiply by scalar) (Subtract scalar (chan-scalar)) (Unchanged)
4 - ST-BS-ADisp-Trans (Unchanged) (Multiply by scalar) (Multiply by scalar) (Unchanged)
5 - ST-BS-ADisp-Long (Unchanged) (Multiply by scalar) (Multiply by scalar) (Unchanged)

LEVEL 4 - Type: Operations between channels

OPERATIONS

0 - ID: Unchanged; Channels n°: 0,4,; Parameters:
1 - ID: Add channels; Channels n°: 1,5,; Parameters:
2 - ID: Add channels; Channels n°: 2,4,; Parameters:
3 - ID: Add channels; Channels n°: 3,5,; Parameters:
4 - ID: Unchanged; Channels n°: 0,; Parameters:
5 - ID: Unchanged; Channels n°: 1,; Parameters:
6 - ID: Unchanged; Channels n°: 2,; Parameters:
7 - ID: Unchanged; Channels n°: 3,; Parameters:

CHANNELS OUT NAMES

0 - DR21-1FNE-RDisp-Trans (Add scalar) (Multiply by scalar) (Subtract scalar (chan-scalar)) (Unchanged) (Unchanged) ST-BS-ADisp-Trans (Unchanged) (Multiply by scalar) (Multiply by scalar) (Unchanged)
1 - DR22-1FNE-RDisp-Long (Add scalar) (Multiply by scalar) (Subtract scalar (chan-scalar)) (Multiply by scalar) (Add channels) ST-BS-ADisp-Long (Unchanged) (Multiply by scalar) (Multiply by scalar) (Unchanged)
2 - DR23-1FSW-RDisp-Trans (Add scalar) (Multiply by scalar) (Subtract scalar (chan-scalar)) (Multiply by scalar) (Add channels) ST-BS-ADisp-Trans (Unchanged) (Multiply by scalar) (Multiply by scalar) (Unchanged)
3 - DR24-1FSW-RDisp-Long (Add scalar) (Multiply by scalar) (Subtract scalar (chan-scalar)) (Unchanged) (Add channels) ST-BS-ADisp-Long (Unchanged) (Multiply by scalar) (Multiply by scalar) (Unchanged)
4 - DR21-1FNE-RDisp-Trans (Add scalar) (Multiply by scalar) (Subtract scalar (chan-scalar)) (Unchanged) (Unchanged)
5 - DR22-1FNE-RDisp-Long (Add scalar) (Multiply by scalar) (Subtract scalar (chan-scalar)) (Multiply by scalar) (Unchanged)
6 - DR23-1FSW-RDisp-Trans (Add scalar) (Multiply by scalar) (Subtract scalar (chan-scalar)) (Multiply by scalar) (Unchanged)
7 - DR24-1FSW-RDisp-Long (Add scalar) (Multiply by scalar) (Subtract scalar (chan-scalar)) (Unchanged) (Unchanged)

LEVEL 5 - Type: Operations to individual channels

OPERATIONS

0 - ID: Unchanged; Channels n°: 0,; Parameters:
1 - ID: Unchanged; Channels n°: 1,; Parameters:
2 - ID: Unchanged; Channels n°: 2,; Parameters:
3 - ID: Unchanged; Channels n°: 3,; Parameters:
4 - ID: Unchanged; Channels n°: 4,; Parameters:
5 - ID: Unchanged; Channels n°: 5,; Parameters:
6 - ID: Unchanged; Channels n°: 6,; Parameters:
7 - ID: Unchanged; Channels n°: 7,; Parameters:

CHANNELS OUT NAMES

0 - MC DA7-1FNE-ADisp-Trans
1 - MC DA7-1FNE-ADisp-Long
2 - MC DA8-1FSW-ADisp-Trans
3 - MC DA8-1FSW-ADisp-Long
4 - MC DA7-1FNE-RDisp-Trans
5 - MC DA7-1FNE-RDisp-Long
6 - MC DA8-1FSW-RDisp-Trans
7 - MC DA8-1FSW-RDisp-Long

CHANNELS OUT UNITS

mm
mm
mm
mm
mm
mm
mm
mm

E.3.3 Rigid Diaphragm

MATH CHANNELS REPORT
LNEC-SPA - Signal Processing and Analysis Program
File: 3-MC RigidDiaphragm.bin

USED CHANNELS

22 - DR1-1FNE-RDisp
23 - DR2-1FNW-RDisp
24 - DR3-1FSE-RDisp
25 - DR4-1FSW-RDisp

LEVEL 0 - Type: Operations to individual channels

OPERATIONS

0 - ID: Divide by scalar (chan/scalar); Channels n°: 22,; Parameters:
3950.000000,
1 - ID: Divide by scalar (chan/scalar); Channels n°: 23,; Parameters:
3950.000000,
2 - ID: Divide by scalar (chan/scalar); Channels n°: 24,; Parameters:
3950.000000,
3 - ID: Divide by scalar (chan/scalar); Channels n°: 25,; Parameters:
3950.000000,

CHANNELS OUT NAMES

0 - DR1-1FNE-RDisp(Divide by scalar (chan/scalar))
1 - DR2-1FNW-RDisp(Divide by scalar (chan/scalar))
2 - DR3-1FSE-RDisp(Divide by scalar (chan/scalar))
3 - DR4-1FSW-RDisp(Divide by scalar (chan/scalar))

LEVEL 1 - Type: Operations to individual channels

OPERATIONS

0 - ID: Multiply by scalar; Channels n°: 0,; Parameters: 100.000000,
1 - ID: Multiply by scalar; Channels n°: 1,; Parameters: 100.000000,
2 - ID: Multiply by scalar; Channels n°: 2,; Parameters: 100.000000,
3 - ID: Multiply by scalar; Channels n°: 3,; Parameters: 100.000000,

CHANNELS OUT NAMES

0 - MC RD1-2FNE-Elong-Diag
1 - MC RD2-2FNW-Elong-Diag
2 - MC RD3-2FSE-Elong-Diag
3 - MC RD4-2FSW-Elong-Diag

CHANNELS OUT UNITS

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%
%

E.3.4 Interstorey Drift

MATH CHANNELS REPORT
LNEC-SPA - Signal Processing and Analysis Program
File: 4-MC Interstorey Drifts.bin

USED CHANNELS

69 - MC DA1-1FNW-RDisp-Trans
70 - MC DA1-1FNW-RDisp-Long
71 - MC DA2-1FSE-RDisp-Trans
72 - MC DA2-1FSE-RDisp-Long
73 - MC DA3-2FNE-RDisp-Trans
74 - MC DA3-2FNE-RDisp-Long
75 - MC DA4-2FNW-RDisp-Trans
76 - MC DA4-2FNW-RDisp-Long
77 - MC DA5-2FSE-RDisp-Trans
78 - MC DA5-2FSE-RDisp-Long
79 - MC DA6-2FSW-RDisp-Trans
80 - MC DA6-2FSW-RDisp-Long
85 - MC DA7-1FNE-RDisp-Trans
86 - MC DA7-1FNE-RDisp-Long
87 - MC DA8-1FSW-RDisp-Trans
88 - MC DA8-1FSW-RDisp-Long

LEVEL 0 - Type: Operations between channels

OPERATIONS

0 - ID: Unchanged; Channels n°: 69,; Parameters:
1 - ID: Unchanged; Channels n°: 70,; Parameters:
2 - ID: Unchanged; Channels n°: 71,; Parameters:

3 - ID: Unchanged; Channels n°: 72,; Parameters:
 4 - ID: Unchanged; Channels n°: 85,; Parameters:
 5 - ID: Unchanged; Channels n°: 86,; Parameters:
 6 - ID: Unchanged; Channels n°: 87,; Parameters:
 7 - ID: Unchanged; Channels n°: 88,; Parameters:
 8 - ID: Subtract channels (1st ch - 2nd ch); Channels n°: 73,85,;
 Parameters:
 9 - ID: Subtract channels (1st ch - 2nd ch); Channels n°: 74,86,;
 Parameters:
 10 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 69,75,;
 Parameters:
 11 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 70,76,;
 Parameters:
 12 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 71,77,;
 Parameters:
 13 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 72,78,;
 Parameters:
 14 - ID: Subtract channels (1st ch - 2nd ch); Channels n°: 79,87,;
 Parameters:
 15 - ID: Subtract channels (1st ch - 2nd ch); Channels n°: 80,88,;
 Parameters:
 CHANNELS OUT NAMES
 0 - MC DA1-1FNW-RDisp-Trans (Unchanged)
 1 - MC DA1-1FNW-RDisp-Long (Unchanged)
 2 - MC DA2-1FSE-RDisp-Trans (Unchanged)
 3 - MC DA2-1FSE-RDisp-Long (Unchanged)
 4 - MC DA7-1FNE-RDisp-Trans (Unchanged)
 5 - MC DA7-1FNE-RDisp-Long (Unchanged)
 6 - MC DA8-1FSW-RDisp-Trans (Unchanged)
 7 - MC DA8-1FSW-RDisp-Long (Unchanged)
 8 - MC DA3-2FNE-RDisp-Trans (Subtract channels (1st ch - 2nd ch))MC DA7-1FNE-
 RDisp-Trans
 9 - MC DA3-2FNE-RDisp-Long (Subtract channels (1st ch - 2nd ch))MC DA7-1FNE-
 RDisp-Long
 10 - MC DA1-1FNW-RDisp-Trans (Subtract channels (2st ch - 1nd ch))MC DA4-
 2FNW-RDisp-Trans
 11 - MC DA1-1FNW-RDisp-Long (Subtract channels (2st ch - 1nd ch))MC DA4-2FNW-
 RDisp-Long
 12 - MC DA2-1FSE-RDisp-Trans (Subtract channels (2st ch - 1nd ch))MC DA5-
 2FSE-RDisp-Trans
 13 - MC DA2-1FSE-RDisp-Long (Subtract channels (2st ch - 1nd ch))MC DA5-2FSE-
 RDisp-Long
 14 - MC DA6-2FSW-RDisp-Trans (Subtract channels (1st ch - 2nd ch))MC DA8-
 1FSW-RDisp-Trans
 15 - MC DA6-2FSW-RDisp-Long (Subtract channels (1st ch - 2nd ch))MC DA8-1FSW-
 RDisp-Long
 LEVEL 1 - Type: Operations to individual channels
 OPERATIONS
 0 - ID: Divide by scalar (chan/scalar); Channels n°: 0,; Parameters:
 1500.000000,
 1 - ID: Divide by scalar (chan/scalar); Channels n°: 1,; Parameters:
 1500.000000,
 2 - ID: Divide by scalar (chan/scalar); Channels n°: 2,; Parameters:
 1500.000000,
 3 - ID: Divide by scalar (chan/scalar); Channels n°: 3,; Parameters:
 1500.000000,
 4 - ID: Divide by scalar (chan/scalar); Channels n°: 4,; Parameters:
 1500.000000,
 5 - ID: Divide by scalar (chan/scalar); Channels n°: 5,; Parameters:
 1500.000000,
 6 - ID: Divide by scalar (chan/scalar); Channels n°: 6,; Parameters:
 1500.000000,
 7 - ID: Divide by scalar (chan/scalar); Channels n°: 7,; Parameters:
 1500.000000,
 8 - ID: Divide by scalar (chan/scalar); Channels n°: 8,; Parameters:
 1500.000000,
 9 - ID: Divide by scalar (chan/scalar); Channels n°: 9,; Parameters:
 1500.000000,
 10 - ID: Divide by scalar (chan/scalar); Channels n°: 10,; Parameters:
 1500.000000,
 11 - ID: Divide by scalar (chan/scalar); Channels n°: 11,; Parameters:
 1500.000000,
 12 - ID: Divide by scalar (chan/scalar); Channels n°: 12,; Parameters:
 1500.000000,
 13 - ID: Divide by scalar (chan/scalar); Channels n°: 13,; Parameters:
 1500.000000,

1500.000000, 14 - ID: Divide by scalar (chan/scalar); Channels n°: 14,; Parameters:
1500.000000, 15 - ID: Divide by scalar (chan/scalar); Channels n°: 15,; Parameters:

- CHANNELS OUT NAMES
- 0 - MC ID1-1FNW-ID-Trans
 - 1 - MC ID1-1FNW-IDLong
 - 2 - MC ID2-1FSE-ID-Trans
 - 3 - MC ID2-1FSE-ID-Long
 - 4 - MC ID3-1FNE-ID-Trans
 - 5 - MC ID3-1FNE-ID-Long
 - 6 - MC ID4-1FSW-ID-Trans
 - 7 - MC ID4-1FSW-ID-Long
 - 8 - MC ID5-2FNE-ID-Trans
 - 9 - MC ID5-2FNE-ID-Long
 - 10 - MC ID6-2FNW-ID-Trans
 - 11 - MC ID6-2FNW-ID-Long
 - 12 - MC ID7-2FSE-ID-Trans
 - 13 - MC ID7-2FSE-ID-Long
 - 14 - MC ID8-2FSW-ID-Trans
 - 15 - MC ID8-2FSW-ID-Long

LEVEL 2 - Type: Operations to individual channels

- OPERATIONS
- 0 - ID: Multiply by scalar; Channels n°: 0,; Parameters: 100.000000,
 - 1 - ID: Multiply by scalar; Channels n°: 1,; Parameters: 100.000000,
 - 2 - ID: Multiply by scalar; Channels n°: 2,; Parameters: 100.000000,
 - 3 - ID: Multiply by scalar; Channels n°: 3,; Parameters: 100.000000,
 - 4 - ID: Multiply by scalar; Channels n°: 4,; Parameters: 100.000000,
 - 5 - ID: Multiply by scalar; Channels n°: 5,; Parameters: 100.000000,
 - 6 - ID: Multiply by scalar; Channels n°: 6,; Parameters: 100.000000,
 - 7 - ID: Multiply by scalar; Channels n°: 7,; Parameters: 100.000000,
 - 8 - ID: Multiply by scalar; Channels n°: 8,; Parameters: 100.000000,
 - 9 - ID: Multiply by scalar; Channels n°: 9,; Parameters: 100.000000,
 - 10 - ID: Multiply by scalar; Channels n°: 10,; Parameters: 100.000000,
 - 11 - ID: Multiply by scalar; Channels n°: 11,; Parameters: 100.000000,
 - 12 - ID: Multiply by scalar; Channels n°: 12,; Parameters: 100.000000,
 - 13 - ID: Multiply by scalar; Channels n°: 13,; Parameters: 100.000000,
 - 14 - ID: Multiply by scalar; Channels n°: 14,; Parameters: 100.000000,
 - 15 - ID: Multiply by scalar; Channels n°: 15,; Parameters: 100.000000,

- CHANNELS OUT NAMES
- 0 - MC ID1-1FNW-ID-Trans
 - 1 - MC ID1-1FNW-IDLong
 - 2 - MC ID2-1FSE-ID-Trans
 - 3 - MC ID2-1FSE-ID-Long
 - 4 - MC ID3-1FNE-ID-Trans
 - 5 - MC ID3-1FNE-ID-Long
 - 6 - MC ID4-1FSW-ID-Trans
 - 7 - MC ID4-1FSW-ID-Long
 - 8 - MC ID5-2FNE-ID-Trans
 - 9 - MC ID5-2FNE-ID-Long
 - 10 - MC ID6-2FNW-ID-Trans
 - 11 - MC ID6-2FNW-ID-Long
 - 12 - MC ID7-2FSE-ID-Trans
 - 13 - MC ID7-2FSE-ID-Long
 - 14 - MC ID8-2FSW-ID-Trans
 - 15 - MC ID8-2FSW-ID-Long

CHANNELS OUT UNITS
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E.3.5 Joints Displacements

MATH CHANNELS REPORT
LNEC-SPA - Signal Processing and Analysis Program
File: 5-MC JointsDisp&Rot.bin

USED CHANNELS

53 - DR5-1FNE-RDisp-BTrans
54 - DR6-1FNE-RDisp-TTrans
55 - DR7-2FNE-RDisp-BTrans
56 - DR8-2FNE-RDisp-TTrans
57 - DR9-1FNE-RDisp-BLong
58 - DR10-1FNE-RDisp-TLong
59 - DR11-2FNE-RDisp-BLong
60 - DR12-2FNE-RDisp-TLong
61 - DR13-1FE-RDisp-BLong
62 - DR14-1FE-RDisp-TLong
63 - DR15-2FE-RDisp-BLong
64 - DR16-2FE-RDisp-TLong
65 - DR17-1FE-RDisp-BLong
66 - DR18-1FE-RDisp-TLong
67 - DR19-2FE-RDisp-BLong
68 - DR20-2FE-RDisp-TLong

LEVEL 0 - Type: Operations between channels

OPERATIONS

0 - ID: Add channels; Channels n°: 53,54,; Parameters:
1 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 53,54,;
Parameters:
2 - ID: Add channels; Channels n°: 55,56,; Parameters:
3 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 55,56,;
Parameters:
4 - ID: Add channels; Channels n°: 57,58,; Parameters:
5 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 57,58,;
Parameters:
6 - ID: Add channels; Channels n°: 59,60,; Parameters:
7 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 59,60,;
Parameters:
8 - ID: Add channels; Channels n°: 61,62,; Parameters:
9 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 61,62,;
Parameters:
10 - ID: Add channels; Channels n°: 63,64,; Parameters:
11 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 63,64,;
Parameters:
12 - ID: Add channels; Channels n°: 65,66,; Parameters:
13 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 65,66,;
Parameters:
14 - ID: Add channels; Channels n°: 67,68,; Parameters:
15 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 67,68,;

Parameters:

CHANNELS OUT NAMES

0 - DR5-1FNE-RDisp-BTrans(Add channels)DR6-1FNE-RDisp-TTrans
1 - DR5-1FNE-RDisp-BTrans(Subtract channels (2nd ch - 1st ch))DR6-1FNE-
RDisp-TTrans
2 - DR7-2FNE-RDisp-BTrans(Add channels)DR8-2FNE-RDisp-TTrans
3 - DR7-2FNE-RDisp-BTrans(Subtract channels (2nd ch - 1st ch))DR8-2FNE-
RDisp-TTrans
4 - DR9-1FNE-RDisp-BLong(Add channels)DR10-1FNE-RDisp-TLong
5 - DR9-1FNE-RDisp-BLong(Subtract channels (2nd ch - 1st ch))DR10-1FNE-
RDisp-TLong
6 - DR11-2FNE-RDisp-BLong(Add channels)DR12-2FNE-RDisp-TLong
7 - DR11-2FNE-RDisp-BLong(Subtract channels (2nd ch - 1st ch))DR12-2FNE-
RDisp-TLong
8 - DR13-1FE-RDisp-BLong(Add channels)DR14-1FE-RDisp-TLong
9 - DR13-1FE-RDisp-BLong(Subtract channels (2nd ch - 1st ch))DR14-1FE-RDisp-
TLong
10 - DR15-2FE-RDisp-BLong(Add channels)DR16-2FE-RDisp-TLong
11 - DR15-2FE-RDisp-BLong(Subtract channels (2nd ch - 1st ch))DR16-2FE-
RDisp-TLong
12 - DR17-1FE-RDisp-BLong(Add channels)DR18-1FE-RDisp-TLong
13 - DR17-1FE-RDisp-BLong(Subtract channels (2nd ch - 1st ch))DR18-1FE-
RDisp-TLong
14 - DR19-2FE-RDisp-BLong(Add channels)DR20-2FE-RDisp-TLong
15 - DR19-2FE-RDisp-BLong(Subtract channels (2nd ch - 1st ch))DR20-2FE-
RDisp-TLong

LEVEL 1 - Type: Operations to individual channels

OPERATIONS

2.000000,	0 - ID: Divide by scalar (chan/scalar); Channels n°: 0,; Parameters:
190.000000,	1 - ID: Divide by scalar (chan/scalar); Channels n°: 1,; Parameters:
2.000000,	2 - ID: Divide by scalar (chan/scalar); Channels n°: 2,; Parameters:
195.000000,	3 - ID: Divide by scalar (chan/scalar); Channels n°: 3,; Parameters:
2.000000,	4 - ID: Divide by scalar (chan/scalar); Channels n°: 4,; Parameters:
158.000000,	5 - ID: Divide by scalar (chan/scalar); Channels n°: 5,; Parameters:
2.000000,	6 - ID: Divide by scalar (chan/scalar); Channels n°: 6,; Parameters:
173.000000,	7 - ID: Divide by scalar (chan/scalar); Channels n°: 7,; Parameters:
2.000000,	8 - ID: Divide by scalar (chan/scalar); Channels n°: 8,; Parameters:
206.000000,	9 - ID: Divide by scalar (chan/scalar); Channels n°: 9,; Parameters:
2.000000,	10 - ID: Divide by scalar (chan/scalar); Channels n°: 10,; Parameters:
189.000000,	11 - ID: Divide by scalar (chan/scalar); Channels n°: 11,; Parameters:
2.000000,	12 - ID: Divide by scalar (chan/scalar); Channels n°: 12,; Parameters:
217.000000,	13 - ID: Divide by scalar (chan/scalar); Channels n°: 13,; Parameters:
2.000000,	14 - ID: Divide by scalar (chan/scalar); Channels n°: 14,; Parameters:
187.000000,	15 - ID: Divide by scalar (chan/scalar); Channels n°: 15,; Parameters:

CHANNELS OUT NAMES

0 - MC J3-1FNE-RDisp-Trans
1 - MC J3-1FNE-Rot-Trans
2 - MC J9-2FNE-RDisp-Trans
3 - MC J9-2FNE-Rot-Trans
4 - MC J3-1FNE-RDisp-Long
5 - MC J3-1FNE-Rot-Long
6 - MC J9-2FNE-RDisp-Long
7 - MC J9-2FNE-Rot-Long
8 - MC J2-1FEN-RDisp-Long
9 - MC J2-1FEN-Rot-Long
10 - MC J8-2FEN-RDisp-Long
11 - MC J8-2FEN-Rot-Long
12 - MC J2-1FES-RDisp-Long
13 - MC J2-1FES-Rot-Long
14 - MC J8-2FES-RDisp-Long
15 - MC J8-2FES-Rot-Long

LEVEL 2 - Type: Operations to individual channels

OPERATIONS

0 - ID: Unchanged; Channels n°: 0,; Parameters:
1 - ID: Apply atan (rad); Channels n°: 1,; Parameters:
2 - ID: Unchanged; Channels n°: 2,; Parameters:
3 - ID: Apply atan (rad); Channels n°: 3,; Parameters:
4 - ID: Unchanged; Channels n°: 4,; Parameters:
5 - ID: Apply atan (rad); Channels n°: 5,; Parameters:
6 - ID: Unchanged; Channels n°: 6,; Parameters:
7 - ID: Apply atan (rad); Channels n°: 7,; Parameters:
8 - ID: Unchanged; Channels n°: 8,; Parameters:
9 - ID: Apply atan (rad); Channels n°: 9,; Parameters:
10 - ID: Unchanged; Channels n°: 10,; Parameters:
11 - ID: Apply atan (rad); Channels n°: 11,; Parameters:
12 - ID: Unchanged; Channels n°: 12,; Parameters:
13 - ID: Apply atan (rad); Channels n°: 13,; Parameters:
14 - ID: Unchanged; Channels n°: 14,; Parameters:
15 - ID: Apply atan (rad); Channels n°: 15,; Parameters:

CHANNELS OUT NAMES

0 - MC J3-1FNE-RDisp-Trans
1 - MC J3-1FNE-Rot-Trans
2 - MC J9-2FNE-RDisp-Trans
3 - MC J9-2FNE-Rot-Trans
4 - MC J3-1FNE-RDisp-Long
5 - MC J3-1FNE-Rot-Long

```

6 - MC J9-2FNE-RDisp-Long
7 - MC J9-2FNE-Rot-Long
8 - MC J2-1FEN-RDisp-Long
9 - MC J2-1FEN-Rot-Long
10 - MC J8-2FEN-RDisp-Long
11 - MC J8-2FEN-Rot-Long
12 - MC J2-1FES-RDisp-Long
13 - MC J2-1FES-Rot-Long
14 - MC J8-2FES-RDisp-Long
15 - MC J8-2FES-Rot-Long

```

CHANNELS OUT UNITS

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rad
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rad
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rad
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rad
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rad

```

E.3.6 Center of Mass Relative Displacements

MATH CHANNELS REPORT
LNEC-SPA - Signal Processing and Analysis Program
File: 6-MC CMRDispRot.bin

USED CHANNELS

```

69 - MC DA1-1FNW-RDisp-Trans
70 - MC DA1-1FNW-RDisp-Long
71 - MC DA2-1FSE-RDisp-Trans
72 - MC DA2-1FSE-RDisp-Long
73 - MC DA3-2FNE-RDisp-Trans
74 - MC DA3-2FNE-RDisp-Long
75 - MC DA4-2FNW-RDisp-Trans
76 - MC DA4-2FNW-RDisp-Long
77 - MC DA5-2FSE-RDisp-Trans
78 - MC DA5-2FSE-RDisp-Long
79 - MC DA6-2FSW-RDisp-Trans
80 - MC DA6-2FSW-RDisp-Long
85 - MC DA7-1FNE-RDisp-Trans
86 - MC DA7-1FNE-RDisp-Long
87 - MC DA8-1FSW-RDisp-Trans
88 - MC DA8-1FSW-RDisp-Long

```

LEVEL 0 - Type: Operations between channels

OPERATIONS

0 - ID: Calculate Disp&Rot - Disp x1; Channels n°: 71,72,85,86;; Parameters:
0.000000,0.000000,4000.000000,0.000000,4000.000000,0.000000,0.000000,4000.000000,0.000000,0.000000,0.000000,4000.000000,0.000000,0.000000,1750.000000,0.000000,4000.000000,0.000000,0.000000,0.000000,1750.000000,2000.000000,

1 - ID: Calculate Disp&Rot - Disp x2; Channels n°: 71,72,85,86;; Parameters:
0.000000,0.000000,4000.000000,0.000000,4000.000000,0.000000,0.000000,4000.000000,0.000000,0.000000,0.000000,4000.000000,0.000000,0.000000,1750.000000,0.000000,4000.000000,0.000000,0.000000,0.000000,1750.000000,2000.000000,

2 - ID: Calculate Disp&Rot - Rot; Channels n°: 71,72,85,86;; Parameters:
0.000000,0.000000,4000.000000,0.000000,4000.000000,0.000000,0.000000,4000.000000,0.000000,0.000000,0.000000,4000.000000,0.000000,0.000000,1750.000000,0.000000,4000.000000,0.000000,0.000000,0.000000,1750.000000,2000.000000,

3 - ID: Calculate Disp&Rot - Disp x1; Channels n°: 85,86,87,88;; Parameters:
0.000000,0.000000,0.000000,0.000000,0.000000,3500.000000,0.000000,0.000000,3500.000000,4000.000000,0.000000,0.000000,3500.000000,4000.000000,1750.000000,0.000000,0.000000,3500.000000,4000.000000,1750.000000,2000.000000,

4 - ID: Calculate Disp&Rot - Disp x2; Channels n°: 85,86,87,88;; Parameters:
0.000000,0.000000,0.000000,0.000000,0.000000,3500.000000,0.000000,0.000000,3500.000000,4000.000000,0.000000,0.000000,3500.000000,4000.000000,1750.000000,0.000000,0.000000,3500.000000,4000.000000,1750.000000,2000.000000,


```

11 - DA2 DA8 Calculate RDisp&Rot - Rot
12 - DA1 DA2 Calculate RDisp&Rot - Disp x1
13 - DA1 DA2 Calculate RDisp&Rot - Disp x2
14 - DA1 DA2 Calculate RDisp&Rot - Rot
15 - DA1 DA8 Calculate RDisp&Rot - Disp x1
16 - DA1 DA8 Calculate RDisp&Rot - Disp x2
17 - DA1 DA8 Calculate RDisp&Rot - Rot
18 - DA3 DA4 Calculate RDisp&Rot - Disp x1
19 - DA3 DA4 Calculate RDisp&Rot - Disp x2
20 - DA3 DA4 Calculate RDisp&Rot - Rot
21 - DA3 DA5 Calculate RDisp&Rot - Disp x1
22 - DA3 DA5 Calculate RDisp&Rot - Disp x2
23 - DA3 DA5 Calculate RDisp&Rot - Rot
24 - DA3 DA6 Calculate RDisp&Rot - Disp x1
25 - DA3 DA6 Calculate RDisp&Rot - Disp x2
26 - DA3 DA6 Calculate RDisp&Rot - Rot
27 - DA4 DA5 Calculate RDisp&Rot - Disp x1
28 - DA4 DA5 Calculate RDisp&Rot - Disp x2
29 - DA4 DA5 Calculate RDisp&Rot - Rot
30 - DA4 DA6 Calculate RDisp&Rot - Disp x1
31 - DA4 DA6 Calculate RDisp&Rot - Disp x2
32 - DA4 DA6 Calculate RDisp&Rot - Rot
33 - DA5 DA6 Calculate RDisp&Rot - Disp x1
34 - DA5 DA6 Calculate RDisp&Rot - Disp x2
35 - DA5 DA6 Calculate RDisp&Rot - Rot

```

LEVEL 1 - Type: Operations between channels

OPERATIONS

```

0 - ID: Unchanged; Channels n°: 12,; Parameters:
1 - ID: Unchanged; Channels n°: 13,; Parameters:
2 - ID: Unchanged; Channels n°: 14,; Parameters:
3 - ID: Average Channels; Channels n°: 18,21,24,27,30,33,; Parameters:
4 - ID: Average Channels; Channels n°: 19,22,25,28,31,34,; Parameters:
5 - ID: Average Channels; Channels n°: 20,23,26,29,32,35,; Parameters:

```

CHANNELS OUT NAMES

```

0 - MC CM1-CM1F-RDisp-Trans
1 - MC CM2-CM1F-RDisp-Long
2 - MC CM3-CM1F-RDisp-Rot
3 - MC CM4-CM2F-RDisp-Trans
4 - MC CM5-CM2F-RDisp-Long
5 - MC CM6-CM2F-RDisp-Rot

```

CHANNELS OUT UNITS

```

mm
mm
rad
mm
mm
rad

```

E.3.7 Center of Mass Absolute Displacements

MATH CHANNELS REPORT

LNEC-SPA - Signal Processing and Analysis Program

File: 7-MC CMADispRot.bin

USED CHANNELS

```

6 - DA1-1FNW-ADisp-Trans
7 - DA1-1FNW-ADisp-Long
8 - DA2-1FSE-ADisp-Trans
9 - DA2-1FSE-ADisp-Long
10 - DA3-2FNE-ADisp-Trans
11 - DA3-2FNE-ADisp-Long
12 - DA4-2FNW-ADisp-Trans
13 - DA4-2FNW-ADisp-Long
14 - DA5-2FSE-ADisp-Trans
15 - DA5-2FSE-ADisp-Long
16 - DA6-2FSW-ADisp-Trans
17 - DA6-2FSW-ADisp-Long
81 - MC DA7-1FNE-ADisp-Trans
82 - MC DA7-1FNE-ADisp-Long
83 - MC DA8-1FSW-ADisp-Trans
84 - MC DA8-1FSW-ADisp-Long
85 - MC DA7-1FNE-RDisp-Trans

```

LEVEL 0 - Type: Operations between channels

OPERATIONS

0000,4000.000000,0.000000,4000.000000,3500.000000,4000.000000,1750.000000,0.000000,4000.000
000,3500.000000,4000.000000,1750.000000,2000.000000,

CHANNELS OUT NAMES

0 - DA2 DA7 Calculate ADisp&Rot - Disp x1
1 - DA2 DA7 Calculate ADisp&Rot - Disp x2
2 - DA2 DA7 Calculate ADisp&Rot - Rot
3 - DA7 DA8 Calculate ADisp&Rot - Disp x1
4 - DA7 DA8 Calculate ADisp&Rot - Disp x2
5 - DA7 DA8 Calculate ADisp&Rot - Rot
6 - DA1 DA7 Calculate ADisp&Rot - Disp x1
7 - DA1 DA7 Calculate ADisp&Rot - Disp x2
8 - DA1 DA7 Calculate ADisp&Rot - Rot
9 - DA2 DA8 Calculate ADisp&Rot - Disp x1
10 - DA2 DA8 Calculate ADisp&Rot - Disp x2
11 - DA2 DA8 Calculate ADisp&Rot - Rot
12 - DA1 DA2 Calculate ADisp&Rot - Disp x1
13 - DA1 DA2 Calculate ADisp&Rot - Disp x2
14 - DA1 DA2 Calculate ADisp&Rot - Rot
15 - DA1 DA8 Calculate ADisp&Rot - Disp x1
16 - DA1 DA8 Calculate ADisp&Rot - Disp x2
17 - DA1 DA8 Calculate ADisp&Rot - Rot
18 - DA3 DA4 Calculate ADisp&Rot - Disp x1
19 - DA3 DA4 Calculate ADisp&Rot - Disp x2
20 - DA3 DA4 Calculate ADisp&Rot - Rot
21 - DA3 DA5 Calculate ADisp&Rot - Disp x1
22 - DA3 DA5 Calculate ADisp&Rot - Disp x2
23 - DA3 DA5 Calculate ADisp&Rot - Rot
24 - DA3 DA6 Calculate ADisp&Rot - Disp x1
25 - DA3 DA6 Calculate ADisp&Rot - Disp x2
26 - DA3 DA6 Calculate ADisp&Rot - Rot
27 - DA4 DA5 Calculate ADisp&Rot - Disp x1
28 - DA4 DA5 Calculate ADisp&Rot - Disp x2
29 - DA4 DA5 Calculate ADisp&Rot - Rot
30 - DA4 DA6 Calculate ADisp&Rot - Disp x1
31 - DA4 DA6 Calculate ADisp&Rot - Disp x2
32 - DA4 DA6 Calculate ADisp&Rot - Rot
33 - DA5 DA6 Calculate ADisp&Rot - Disp x1
34 - DA5 DA6 Calculate ADisp&Rot - Disp x2
35 - DA5 DA6 Calculate ADisp&Rot - Rot

LEVEL 1 - Type: Operations between channels

OPERATIONS

0 - ID: Unchanged; Channels n°: 12,; Parameters:
1 - ID: Unchanged; Channels n°: 13,; Parameters:
2 - ID: Unchanged; Channels n°: 14,; Parameters:
3 - ID: Average Channels; Channels n°: 18,21,24,27,30,33,; Parameters:
4 - ID: Average Channels; Channels n°: 19,22,25,28,31,34,; Parameters:
5 - ID: Average Channels; Channels n°: 20,23,26,29,32,35,; Parameters:

CHANNELS OUT NAMES

0 - MC CM7-CM1F-ADisp-Trans
1 - MC CM8-CM1F-ADisp-Long
2 - MC CM9-CM1F-ADisp-Rot
3 - MC CM10-CM2F-ADisp-Trans
4 - MC CM11-CM2F-ADisp-Long
5 - MC CM12-CM2F-ADisp-Rot

CHANNELS OUT UNITS

g
g
rad/s^2
g
g
rad/s^2

E.3.8 Averaged Storey Accelerations

MATH CHANNELS REPORT

LNEC-SPA - Signal Processing and Analysis Program

File: 8-MC AccAvg Pisos.bin

USED CHANNELS

26 - A1-2FNE-Acc-Trans
27 - A2-2FNE-Acc-Long
28 - A3-2FE-Acc-Trans
29 - A4-2FE-Acc-Long
30 - A5-2FSE-Acc-Trans
31 - A6-2FSE-Acc-Long

- 32 - A7-2FSW-Acc-Trans
- 33 - A8-2FSW-Acc-Long
- 34 - A9-2FW-Acc-Trans
- 35 - A10-2FW-Acc-Long
- 36 - A11-2FNW-Acc-Trans
- 37 - A12-2FNW-Acc-Long
- 38 - A13-1FNE-Acc-Trans
- 39 - A14-1FNE-Acc-Long
- 40 - A15-1FE-Acc-Trans
- 41 - A16-1FE-Acc-Long
- 42 - A17-1FSE-Acc-Trans
- 43 - A18-1FSE-Acc-Long
- 44 - A19-1FSW-Acc-Trans
- 45 - A20-1FSW-Acc-Long
- 46 - A21-1FW-Acc-Trans
- 47 - A22-1FW-Acc-Long
- 48 - A23-1FNW-Acc-Trans
- 49 - A24-1FNW-Acc-Long

LEVEL 0 - Type: Operations between channels

OPERATIONS

- 0 - ID: Average Channels; Channels n°: 38,40,42,44,46,48,; Parameters:
- 1 - ID: Average Channels; Channels n°: 39,41,43,45,47,49,; Parameters:
- 2 - ID: Average Channels; Channels n°: 26,28,30,32,34,36,; Parameters:
- 3 - ID: Average Channels; Channels n°: 27,29,31,33,35,37,; Parameters:
- 4 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 38,42,;

Parameters:

- 5 - ID: Subtract channels (1st ch - 2nd ch); Channels n°: 44,48,;

Parameters:

- 6 - ID: Subtract channels (1st ch - 2nd ch); Channels n°: 39,49,;

Parameters:

- 7 - ID: Subtract channels (1st ch - 2nd ch); Channels n°: 43,45,;

Parameters:

- 8 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 26,30,;

Parameters:

- 9 - ID: Subtract channels (1st ch - 2nd ch); Channels n°: 32,36,;

Parameters:

- 10 - ID: Subtract channels (1st ch - 2nd ch); Channels n°: 27,37,;

Parameters:

- 11 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 33,37,;

Parameters:

CHANNELS OUT NAMES

- 0 - MC AA1-1F-AccAvg-Trans
- 1 - MC AA2-1F-AccAvg-Long
- 2 - MC AA3-2F-AccAvg-Trans
- 3 - MC AA4-2F-AccAvg-Long
- 4 - A13-1FNE-Acc-Trans (Subtract channels (2nd ch - 1st ch))A17-1FSE-Acc-Trans
- 5 - A19-1FSW-Acc-Trans (Subtract channels (1st ch - 2nd ch))A23-1FNW-Acc-Trans
- 6 - A14-1FNE-Acc-Long (Subtract channels (1st ch - 2nd ch))A24-1FNW-Acc-Long
- 7 - A18-1FSE-Acc-Long (Subtract channels (1st ch - 2nd ch))A20-1FSW-Acc-Long
- 8 - A1-2FNE-Acc-Trans (Subtract channels (2nd ch - 1st ch))A5-2FSE-Acc-Trans
- 9 - A7-2FSW-Acc-Trans (Subtract channels (1st ch - 2nd ch))A11-2FNW-Acc-Trans
- 10 - A2-2FNE-Acc-Long (Subtract channels (1st ch - 2nd ch))A12-2FNW-Acc-Long
- 11 - A8-2FSW-Acc-Long (Subtract channels (2nd ch - 1st ch))A12-2FNW-Acc-Long

Trans

Trans

LEVEL 1 - Type: Operations to individual channels

OPERATIONS

- 0 - ID: Unchanged; Channels n°: 0,; Parameters:
- 1 - ID: Unchanged; Channels n°: 1,; Parameters:
- 2 - ID: Unchanged; Channels n°: 2,; Parameters:
- 3 - ID: Unchanged; Channels n°: 3,; Parameters:
- 4 - ID: Divide by scalar (chan/scalar); Channels n°: 4,; Parameters:
- 5 - ID: Divide by scalar (chan/scalar); Channels n°: 5,; Parameters:
- 6 - ID: Divide by scalar (chan/scalar); Channels n°: 6,; Parameters:
- 7 - ID: Divide by scalar (chan/scalar); Channels n°: 7,; Parameters:
- 8 - ID: Divide by scalar (chan/scalar); Channels n°: 8,; Parameters:
- 9 - ID: Divide by scalar (chan/scalar); Channels n°: 9,; Parameters:
- 10 - ID: Divide by scalar (chan/scalar); Channels n°: 10,; Parameters:

4.000000,

4.000000,

4.000000,

4.000000,

3.500000,

3.500000,

3.500000,

11 - ID: Divide by scalar (chan/scalar); Channels n°: 11;; Parameters:
 3.500000,
 CHANNELS OUT NAMES
 0 - MC AA1-1F-AccAvg-Trans (Unchanged)
 1 - MC AA2-1F-AccAvg-Long (Unchanged)
 2 - MC AA3-2F-AccAvg-Trans (Unchanged)
 3 - MC AA4-2F-AccAvg-Long (Unchanged)
 4 - A13-1FNE-Acc-Trans (Subtract channels (2nd ch - 1st ch))A17-1FSE-Acc-
 Trans(Divide by scalar (chan/scalar))
 5 - A19-1FSW-Acc-Trans (Subtract channels (1st ch - 2nd ch))A23-1FNW-Acc-
 Trans(Divide by scalar (chan/scalar))
 6 - A14-1FNE-Acc-Long (Subtract channels (1st ch - 2nd ch))A24-1FNW-Acc-
 Long(Divide by scalar (chan/scalar))
 7 - A18-1FSE-Acc-Long (Subtract channels (1st ch - 2nd ch))A20-1FSW-Acc-
 Long(Divide by scalar (chan/scalar))
 8 - A1-2FNE-Acc-Trans (Subtract channels (2nd ch - 1st ch))A5-2FSE-Acc-
 Trans(Divide by scalar (chan/scalar))
 9 - A7-2FSW-Acc-Trans (Subtract channels (1st ch - 2nd ch))A11-2FNW-Acc-
 Trans(Divide by scalar (chan/scalar))
 10 - A2-2FNE-Acc-Long (Subtract channels (1st ch - 2nd ch))A12-2FNW-Acc-
 Long(Divide by scalar (chan/scalar))
 11 - A8-2FSW-Acc-Long (Subtract channels (2nd ch - 1st ch))A12-2FNW-Acc-
 Long(Divide by scalar (chan/scalar))

LEVEL 2 - Type: Operations between channels

OPERATIONS

- 0 - ID: Unchanged; Channels n°: 0;; Parameters:
- 1 - ID: Unchanged; Channels n°: 1;; Parameters:
- 2 - ID: Unchanged; Channels n°: 2;; Parameters:
- 3 - ID: Unchanged; Channels n°: 3;; Parameters:
- 4 - ID: Average Channels; Channels n°: 4,5;; Parameters:
- 5 - ID: Average Channels; Channels n°: 6,7;; Parameters:
- 6 - ID: Average Channels; Channels n°: 8,9;; Parameters:
- 7 - ID: Average Channels; Channels n°: 10,11;; Parameters:

CHANNELS OUT NAMES

0 - MC AA1-1F-AccAvg-Trans (Unchanged) (Unchanged)
 1 - MC AA2-1F-AccAvg-Long (Unchanged) (Unchanged)
 2 - MC AA3-2F-AccAvg-Trans (Unchanged) (Unchanged)
 3 - MC AA4-2F-AccAvg-Long (Unchanged) (Unchanged)
 4 - A13-1FNE-Acc-Trans (Subtract channels (2nd ch - 1st ch))A17-1FSE-Acc-
 Trans(Divide by scalar (chan/scalar)) (Average Channels)A19-1FSW-Acc-Trans (Subtract channels
 (1st ch - 2nd ch))A23-1FNW-Acc-Trans (Divide by scalar (chan/scalar))
 5 - A14-1FNE-Acc-Long (Subtract channels (1st ch - 2nd ch))A24-1FNW-Acc-
 Long(Divide by scalar (chan/scalar)) (Average Channels)A18-1FSE-Acc-Long (Subtract channels
 (1st ch - 2nd ch))A20-1FSW-Acc-Long (Divide by scalar (chan/scalar))
 6 - A1-2FNE-Acc-Trans (Subtract channels (2nd ch - 1st ch))A5-2FSE-Acc-
 Trans(Divide by scalar (chan/scalar)) (Average Channels)A7-2FSW-Acc-Trans (Subtract channels
 (1st ch - 2nd ch))A11-2FNW-Acc-Trans (Divide by scalar (chan/scalar))
 7 - A2-2FNE-Acc-Long (Subtract channels (1st ch - 2nd ch))A12-2FNW-Acc-
 Long(Divide by scalar (chan/scalar)) (Average Channels)A8-2FSW-Acc-Long (Subtract channels
 (2nd ch - 1st ch))A12-2FNW-Acc-Long (Divide by scalar (chan/scalar))

LEVEL 3 - Type: Operations between channels

OPERATIONS

- 0 - ID: Unchanged; Channels n°: 0;; Parameters:
- 1 - ID: Unchanged; Channels n°: 1;; Parameters:
- 2 - ID: Unchanged; Channels n°: 4;; Parameters:
- 3 - ID: Unchanged; Channels n°: 5;; Parameters:
- 4 - ID: Unchanged; Channels n°: 2;; Parameters:
- 5 - ID: Unchanged; Channels n°: 3;; Parameters:
- 6 - ID: Unchanged; Channels n°: 6;; Parameters:
- 7 - ID: Unchanged; Channels n°: 7;; Parameters:

CHANNELS OUT NAMES

- 0 - MC AA1-1F-AccAvg-Trans
- 1 - MC AA2-1F-AccAvg-Long
- 2 - MC AA3-1F-RotAccAvg-Trans
- 3 - MC AA4-1F-RotAccAvg-Long
- 4 - MC AA5-2F-AccAvg-Trans
- 5 - MC AA6-2F-AccAvg-Long
- 6 - MC AA7-2F-RotAccAvg-Trans
- 7 - MC AA8-2F-RotAccAvg-Long

LEVEL 4 - Type: Operations between channels

OPERATIONS

- 0 - ID: Unchanged; Channels n°: 0;; Parameters:
- 1 - ID: Unchanged; Channels n°: 1;; Parameters:
- 2 - ID: Unchanged; Channels n°: 2;; Parameters:

3 - ID: Unchanged; Channels n°: 3,; Parameters:
 4 - ID: Average Channels; Channels n°: 2,3,; Parameters:
 5 - ID: Unchanged; Channels n°: 4,; Parameters:
 6 - ID: Unchanged; Channels n°: 5,; Parameters:
 7 - ID: Unchanged; Channels n°: 6,; Parameters:
 8 - ID: Unchanged; Channels n°: 7,; Parameters:
 9 - ID: Average Channels; Channels n°: 6,7,; Parameters:

CHANNELS OUT NAMES

0 - MC AA1-1F-AccAvg-Trans
 1 - MC AA2-1F-AccAvg-Long
 2 - MC AA3-1F-RotAccAvg-Trans
 3 - MC AA3-1F-RotAccAvg-Long
 4 - MC AA3-1F-RotAccAvg-DirAvg
 5 - MC AA4-2F-AccAvg-Trans
 6 - MC AA5-2F-AccAvg-Long
 7 - MC AA6-2F-RotAccAvg-Trans
 8 - MC AA6-2F-RotAccAvg-Long
 9 - MC AA6-2F-RotAccAvg-DirAvg

LEVEL 5 - Type: Operations to individual channels

OPERATIONS

0 - ID: Unchanged; Channels n°: 0,; Parameters:
 1 - ID: Unchanged; Channels n°: 1,; Parameters:
 2 - ID: Divide by scalar (chan/scalar); Channels n°: 2,; Parameters:
 9.806700,
 3 - ID: Divide by scalar (chan/scalar); Channels n°: 3,; Parameters:
 9.806700,
 4 - ID: Divide by scalar (chan/scalar); Channels n°: 4,; Parameters:
 9.806700,
 5 - ID: Unchanged; Channels n°: 5,; Parameters:
 6 - ID: Unchanged; Channels n°: 6,; Parameters:
 7 - ID: Divide by scalar (chan/scalar); Channels n°: 7,; Parameters:
 9.806700,
 8 - ID: Divide by scalar (chan/scalar); Channels n°: 8,; Parameters:
 9.806700,
 9 - ID: Divide by scalar (chan/scalar); Channels n°: 9,; Parameters:
 9.806700,

CHANNELS OUT NAMES

0 - MC AAV1-CM1F-AccAvg-Trans
 1 - MCAAV2-CM1F-AccAvg-Long
 2 - MC AAV3-CM1F-RotAccAvg-Trans
 3 - MC AAV3-CM1F-RotAccAvg-Long
 4 - MC AAV3-CM1F-RotAccAvg-DirAvg
 5 - MC AAV4-CM2F-AccAvg-Trans
 6 - MC AAV5-CM2F-AccAvg-Long
 7 - MC AAV6-CM2F-RotAccAvg-Trans
 8 - MC AAV6-CM2F-RotAccAvg-Long
 9 - MC AAV6-CM2F-RotAccAvg-DirAvg

CHANNELS OUT UNITS

g
 g
 rad/s^2
 rad/s^2
 rad/s^2
 g
 g
 rad/s^2
 rad/s^2
 rad/s^2

E.3.9 Global Forces

MDOF GLOBAL BEHAVIOUR REPORT

LNEC-SPA - Signal Processing and Analysis Program

File: 9-MDOF-01.bin

DOF 0

Height: 1.500000 Units: m
 Mass X1: 12.960000 Units: ton
 Mass X2: 12.960000 Units: ton
 Inertia Rot. X3: 33.000000 Units: ton.m^2
 Channel Acceleration X1: 137-MC AAV1-CM1F-AccAvg-Trans Units: g
 Channel Acceleration X2: 138-MCAAV2-CM1F-AccAvg-Long Units: g
 Channel Acceleration X3: 139-MC AAV3-CM1F-RotAccAvg-Trans Units: rad/s^2
 Channel Displacement X1: 125-MC CM1-CM1F-RDisp-Trans Units: mm
 Channel Displacement X2: 126-MC CM2-CM1F-RDisp-Long Units: mm

Channel Displacement X3: 127-MC CM2-CM1F-RDisp-Long Units: rad
Output Units Forces: kN
Output Units Moments: kN.m

DOF 1

Height: 3.000000 Units: m
Mass X1: 12.960000 Units: ton
Mass X2: 12.960000 Units: ton
Inertia Rot. X3: 33.000000 Units: ton.m^2
Channel Acceleration X1: 142-MC AAV4-CM2F-AccAvg-Trans Units: g
Channel Acceleration X2: 143-MC AAV5-CM2F-AccAvg-Long Units: g
Channel Acceleration X3: 144-MC AAV6-CM2F-RotAccAvg-Trans Units: rad/s^2
Channel Displacement X1: 128-MC CM4-CM2F-RDisp-Trans Units: mm
Channel Displacement X2: 129-MC CM5-CM2F-RDisp-Long Units: mm
Channel Displacement X3: 130-MC CM5-CM2F-RDisp-Long Units: rad
Output Units Forces: kN
Output Units Moments: kN.m

Output Channels:

MC IF1-DOF1-X1 Units: kN
MC IF2-DOF1-X2 Units: kN
MC IF3-DOF1-Tor Units: kN.m
MC IF4-DOF2-X1 Units: kN
MC IF5-DOF2-X2 Units: kN
MC IF6-DOF2-Tor Units: kN.m
MC SF1-DOF1-X1 Units: kN
MC SF2-DOF1-X2 Units: kN
MC SF3-DOF1-Tor Units: kN.m
MC SF4-DOF2-X1 Units: kN
MC SF5-DOF2-X2 Units: kN
MC SF6-DOF2-Tor Units: kN.m
MC BS1-ST-X1 Units: kN
MC BS2-ST-X2 Units: kN
MC BT-ST-Tor Units: kN.m
MC BOM1-ST-X1 Units: kN.m
MC BOM2-ST-X2 Units: kN.m

E.3.10 Center of Mass Interstorey Drifts

MATH CHANNELS REPORT
LNEC-SPA - Signal Processing and Analysis Program
File: 10-MC CM Interstorey Drifts.bin

USED CHANNELS

125 - MC CM1-CM1F-RDisp-Trans
126 - MC CM2-CM1F-RDisp-Long
128 - MC CM4-CM2F-RDisp-Trans
129 - MC CM5-CM2F-RDisp-Long

LEVEL 0 - Type: Operations between channels

OPERATIONS

0 - ID: Unchanged; Channels n°: 125,; Parameters:
1 - ID: Unchanged; Channels n°: 126,; Parameters:
2 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 125,128,;

Parameters:

3 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 126,129,;

Parameters:

CHANNELS OUT NAMES

0 - MC CM1-CM1F-RDisp-Trans (Unchanged)
1 - MC CM2-CM1F-RDisp-Long (Unchanged)
2 - MC CM1-CM1F-RDisp-Trans (Subtract channels (2nd ch - 1st ch))MC CM4-CM2F-
RDisp-Trans
3 - MC CM2-CM1F-RDisp-Long (Subtract channels (2nd ch - 1st ch))MC CM5-CM2F-
RDisp-Long

LEVEL 1 - Type: Operations to individual channels

OPERATIONS

0 - ID: Divide by scalar (chan/scalar); Channels n°: 0,; Parameters:
1500.000000,
1 - ID: Divide by scalar (chan/scalar); Channels n°: 1,; Parameters:
1500.000000,
2 - ID: Divide by scalar (chan/scalar); Channels n°: 2,; Parameters:
1500.000000,
3 - ID: Divide by scalar (chan/scalar); Channels n°: 3,; Parameters:
1500.000000,

CHANNELS OUT NAMES

```

    0 - MC CM1-CM1F-RDisp-Trans(Unchanged)(Divide by scalar (chan/scalar))
    1 - MC CM2-CM1F-RDisp-Long(Unchanged)(Divide by scalar (chan/scalar))
    2 - MC CM1-CM1F-RDisp-Trans(Subtract channels (2nd ch - 1st ch))MC CM4-CM2F-
RDisp-Trans(Divide by scalar (chan/scalar))
    3 - MC CM2-CM1F-RDisp-Long(Subtract channels (2nd ch - 1st ch))MC CM5-CM2F-
RDisp-Long(Divide by scalar (chan/scalar))

```

LEVEL 2 - Type: Operations to individual channels

```

OPERATIONS
    0 - ID: Multiply by scalar; Channels n°: 0,; Parameters: 100.000000,
    1 - ID: Multiply by scalar; Channels n°: 1,; Parameters: 100.000000,
    2 - ID: Multiply by scalar; Channels n°: 2,; Parameters: 100.000000,
    3 - ID: Multiply by scalar; Channels n°: 3,; Parameters: 100.000000,
CHANNELS OUT NAMES
    0 - MC ID9-1FCM-ID-Trans
    1 - MC ID9-1FCM-ID-Long
    2 - MC ID10-2FCM-ID-Trans
    3 - MC ID10-2FCM-ID-Long
CHANNELS OUT UNITS
    Undefined
    Undefined
    Undefined
    Undefined

```

E.3.11 Center of Mass – Second Floor – Relative Displacements

MATH CHANNELS REPORT

LNEC-SPA - Signal Processing and Analysis Program
File: 11-MC CM2FRDisp.bin

USED CHANNELS

```

125 - MC CM1-CM1F-RDisp-Trans
126 - MC CM2-CM1F-RDisp-Long
128 - MC CM4-CM2F-RDisp-Trans
129 - MC CM5-CM2F-RDisp-Long

```

LEVEL 0 - Type: Operations between channels

```

OPERATIONS
    0 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 125,128,;
Parameters:
    1 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 126,129,;
Parameters:
CHANNELS OUT NAMES
    0 - MC CM13-CM2F-CM1F-RDisp-Trans
    1 - MC CM14-CM2F-CM1F-RDisp-Long
CHANNELS OUT UNITS
    mm
    mm

```

E.3.12 Joints Displacements – All Stages

MATH CHANNELS REPORT

LNEC-SPA - Signal Processing and Analysis Program
File: 12-MC JointsDisp&RotAllStages.bin

USED CHANNELS

```

0 - DR5-1FNE-RDisp-Btrans
1 - DR6-1FNE-RDisp-TTrans
2 - DR7-2FNE-RDisp-BTrans
3 - DR8-2FNE-RDisp-TTrans
4 - DR9-1FNE-RDisp-BLong
5 - DR10-1FNE-RDisp-TLong
6 - DR11-2FNE-RDisp-BLong
7 - DR12-2FNE-RDisp-TLong
8 - DR13-1FE-RDisp-BLong
9 - DR14-1FE-RDisp-TLong
10 - DR15-2FE-RDisp-BLong
11 - DR16-2FE-RDisp-TLong
12 - DR17-1FE-RDisp-BLong
13 - DR18-1FE-RDisp-TLong
14 - DR19-2FE-RDisp-BLong
15 - DR20-2FE-RDisp-TLong

```

LEVEL 0 - Type: Operations between channels

OPERATIONS

0 - ID: Add channels; Channels n°: 0,1,; Parameters:
 1 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 0,1,; Parameters:
 2 - ID: Add channels; Channels n°: 2,3,; Parameters:
 3 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 2,3,; Parameters:
 4 - ID: Add channels; Channels n°: 4,5,; Parameters:
 5 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 4,5,; Parameters:
 6 - ID: Add channels; Channels n°: 6,7,; Parameters:
 7 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 6,7,; Parameters:
 8 - ID: Add channels; Channels n°: 8,9,; Parameters:
 9 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 8,9,; Parameters:
 10 - ID: Add channels; Channels n°: 10,11,; Parameters:
 11 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 10,11,;
 Parameters:
 12 - ID: Add channels; Channels n°: 12,13,; Parameters:
 13 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 12,13,;
 Parameters:
 14 - ID: Add channels; Channels n°: 14,15,; Parameters:
 15 - ID: Subtract channels (2nd ch - 1st ch); Channels n°: 14,15,;

Parameters:

CHANNELS OUT NAMES

0 - DR5-1FNE-RDisp-BTrans(Add channels)DR6-1FNE-RDisp-TTrans
 1 - DR5-1FNE-RDisp-BTrans(Subtract channels (2nd ch - 1st ch))DR6-1FNE-
 RDisp-TTrans
 2 - DR7-2FNE-RDisp-BTrans(Add channels)DR8-2FNE-RDisp-TTrans
 3 - DR7-2FNE-RDisp-BTrans(Subtract channels (2nd ch - 1st ch))DR8-2FNE-
 RDisp-TTrans
 4 - DR9-1FNE-RDisp-BLong(Add channels)DR10-1FNE-RDisp-TLong
 5 - DR9-1FNE-RDisp-BLong(Subtract channels (2nd ch - 1st ch))DR10-1FNE-
 RDisp-TLong
 6 - DR11-2FNE-RDisp-BLong(Add channels)DR12-2FNE-RDisp-TLong
 7 - DR11-2FNE-RDisp-BLong(Subtract channels (2nd ch - 1st ch))DR12-2FNE-
 RDisp-TLong
 8 - DR13-1FE-RDisp-BLong(Add channels)DR14-1FE-RDisp-TLong
 9 - DR13-1FE-RDisp-BLong(Subtract channels (2nd ch - 1st ch))DR14-1FE-RDisp-
 TLong
 10 - DR15-2FE-RDisp-BLong(Add channels)DR16-2FE-RDisp-TLong
 11 - DR15-2FE-RDisp-BLong(Subtract channels (2nd ch - 1st ch))DR16-2FE-
 RDisp-TLong
 12 - DR17-1FE-RDisp-BLong(Add channels)DR18-1FE-RDisp-TLong
 13 - DR17-1FE-RDisp-BLong(Subtract channels (2nd ch - 1st ch))DR18-1FE-
 RDisp-TLong
 14 - DR19-2FE-RDisp-BLong(Add channels)DR20-2FE-RDisp-TLong
 15 - DR19-2FE-RDisp-BLong(Subtract channels (2nd ch - 1st ch))DR20-2FE-
 RDisp-TLong

LEVEL 1 - Type: Operations to individual channels

OPERATIONS

2.000000, 0 - ID: Divide by scalar (chan/scalar); Channels n°: 0,; Parameters:
 190.000000, 1 - ID: Divide by scalar (chan/scalar); Channels n°: 1,; Parameters: -
 2.000000, 2 - ID: Divide by scalar (chan/scalar); Channels n°: 2,; Parameters:
 195.000000, 3 - ID: Divide by scalar (chan/scalar); Channels n°: 3,; Parameters: -
 2.000000, 4 - ID: Divide by scalar (chan/scalar); Channels n°: 4,; Parameters:
 158.000000, 5 - ID: Divide by scalar (chan/scalar); Channels n°: 5,; Parameters: -
 2.000000, 6 - ID: Divide by scalar (chan/scalar); Channels n°: 6,; Parameters:
 173.000000, 7 - ID: Divide by scalar (chan/scalar); Channels n°: 7,; Parameters: -
 2.000000, 8 - ID: Divide by scalar (chan/scalar); Channels n°: 8,; Parameters:
 206.000000, 9 - ID: Divide by scalar (chan/scalar); Channels n°: 9,; Parameters: -
 2.000000, 10 - ID: Divide by scalar (chan/scalar); Channels n°: 10,; Parameters:
 189.000000, 11 - ID: Divide by scalar (chan/scalar); Channels n°: 11,; Parameters: -
 2.000000, 12 - ID: Divide by scalar (chan/scalar); Channels n°: 12,; Parameters:
 217.000000, 13 - ID: Divide by scalar (chan/scalar); Channels n°: 13,; Parameters: -

2.000000, 14 - ID: Divide by scalar (chan/scalar); Channels n°: 14,; Parameters:
187.000000, 15 - ID: Divide by scalar (chan/scalar); Channels n°: 15,; Parameters: -

CHANNELS OUT NAMES

0 - MC J3-1FNE-RDisp-Trans
1 - MC J3-1FNE-Rot-Trans
2 - MC J9-2FNE-RDisp-Trans
3 - MC J9-2FNE-Rot-Trans
4 - MC J3-1FNE-RDisp-Long
5 - MC J3-1FNE-Rot-Long
6 - MC J9-2FNE-RDisp-Long
7 - MC J9-2FNE-Rot-Long
8 - MC J2-1FEN-RDisp-Long
9 - MC J2-1FEN-Rot-Long
10 - MC J8-2FEN-RDisp-Long
11 - MC J8-2FEN-Rot-Long
12 - MC J2-1FES-RDisp-Long
13 - MC J2-1FES-Rot-Long
14 - MC J8-2FES-RDisp-Long
15 - MC J8-2FES-Rot-Long

LEVEL 2 - Type: Operations to individual channels

OPERATIONS

0 - ID: Unchanged; Channels n°: 0,; Parameters:
1 - ID: Apply atan (rad); Channels n°: 1,; Parameters:
2 - ID: Unchanged; Channels n°: 2,; Parameters:
3 - ID: Apply atan (rad); Channels n°: 3,; Parameters:
4 - ID: Unchanged; Channels n°: 4,; Parameters:
5 - ID: Apply atan (rad); Channels n°: 5,; Parameters:
6 - ID: Unchanged; Channels n°: 6,; Parameters:
7 - ID: Apply atan (rad); Channels n°: 7,; Parameters:
8 - ID: Unchanged; Channels n°: 8,; Parameters:
9 - ID: Apply atan (rad); Channels n°: 9,; Parameters:
10 - ID: Unchanged; Channels n°: 10,; Parameters:
11 - ID: Apply atan (rad); Channels n°: 11,; Parameters:
12 - ID: Unchanged; Channels n°: 12,; Parameters:
13 - ID: Apply atan (rad); Channels n°: 13,; Parameters:
14 - ID: Unchanged; Channels n°: 14,; Parameters:
15 - ID: Apply atan (rad); Channels n°: 15,; Parameters:

CHANNELS OUT NAMES

0 - MC J3-1FNE-RDisp-Trans
1 - MC J3-1FNE-Rot-Trans
2 - MC J9-2FNE-RDisp-Trans
3 - MC J9-2FNE-Rot-Trans
4 - MC J3-1FNE-RDisp-Long
5 - MC J3-1FNE-Rot-Long
6 - MC J9-2FNE-RDisp-Long
7 - MC J9-2FNE-Rot-Long
8 - MC J2-1FEN-RDisp-Long
9 - MC J2-1FEN-Rot-Long
10 - MC J8-2FEN-RDisp-Long
11 - MC J8-2FEN-Rot-Long
12 - MC J2-1FES-RDisp-Long
13 - MC J2-1FES-Rot-Long
14 - MC J8-2FES-RDisp-Long
15 - MC J8-2FES-Rot-Long

CHANNELS OUT UNITS

mm
rad
mm
rad
mm
rad
mm
rad
mm
rad
mm
rad
mm
rad
mm
rad

E.4 Drawings

List of Drawings:

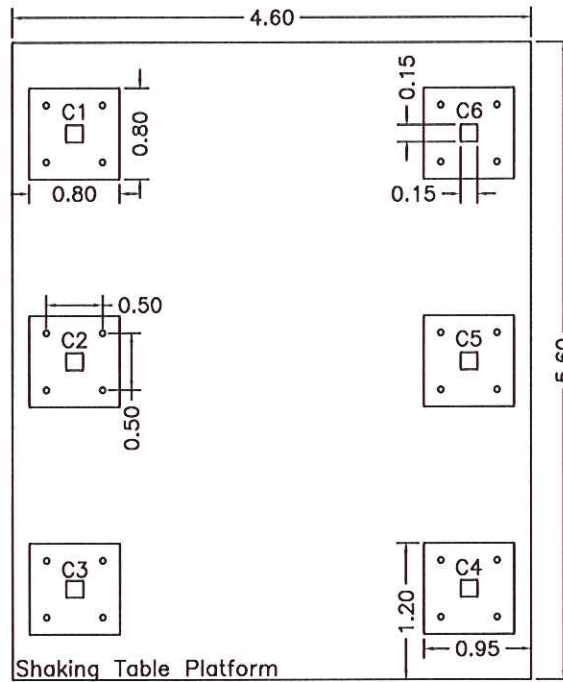
Drawing nº 1 – Model – Test Setup – Plans

Drawing nº 2 – Model – Test Setup – Views

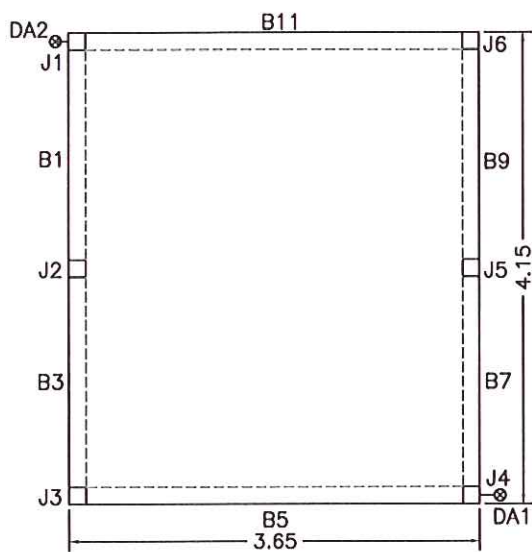
Test Setup

Plans

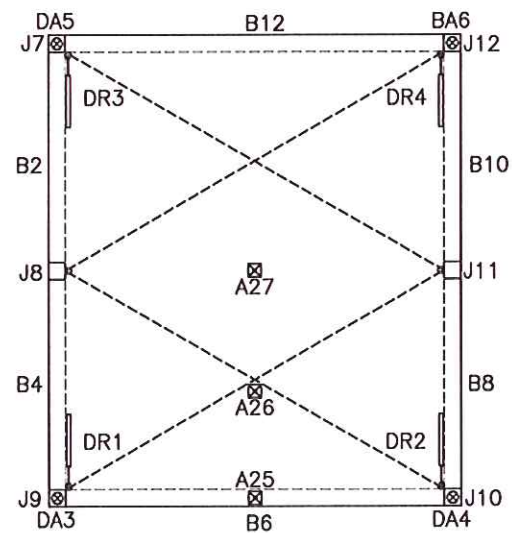
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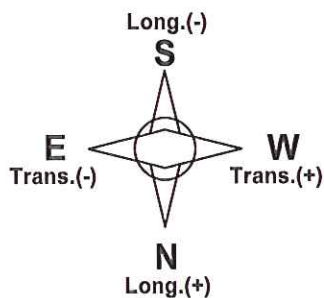
BASE LEVEL



FIRST FLOOR

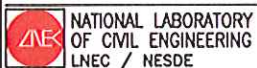


SECOND FLOOR



LEGEND	
	Optical displacement transducers (lens/target led)
	Accelerometers (Arrows represents directions measured)
	LVDT displacement transducers

Dimensions in meters



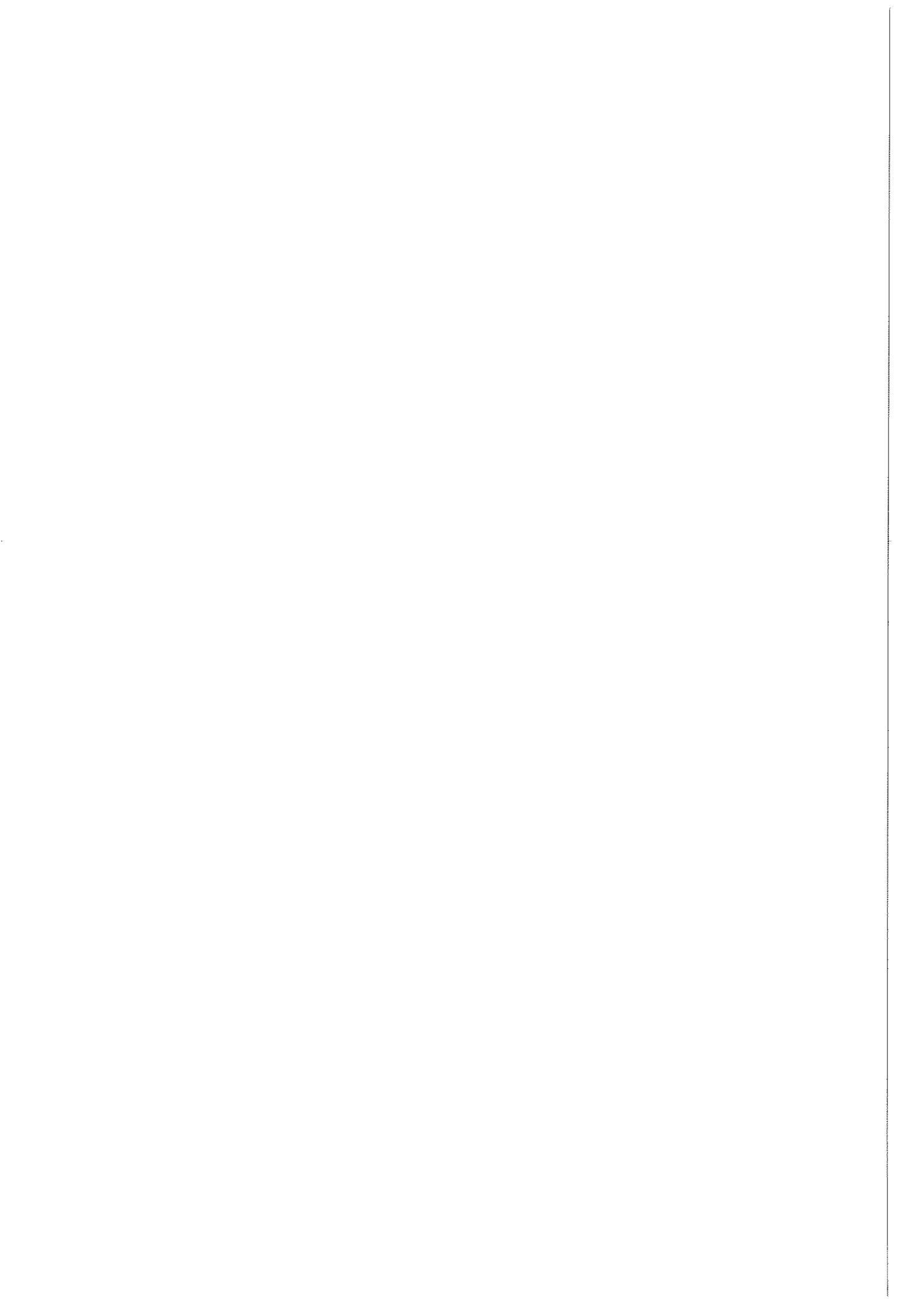
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(except if indicated)

PRECAST STRUCTURES EC8

Test Setup – Plans

Dec 2005

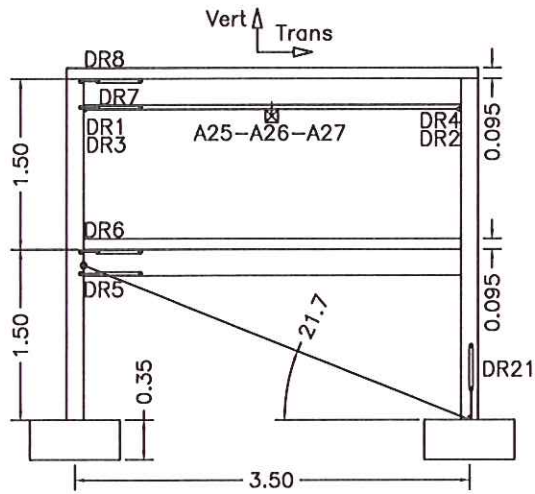
Drawing N°
1



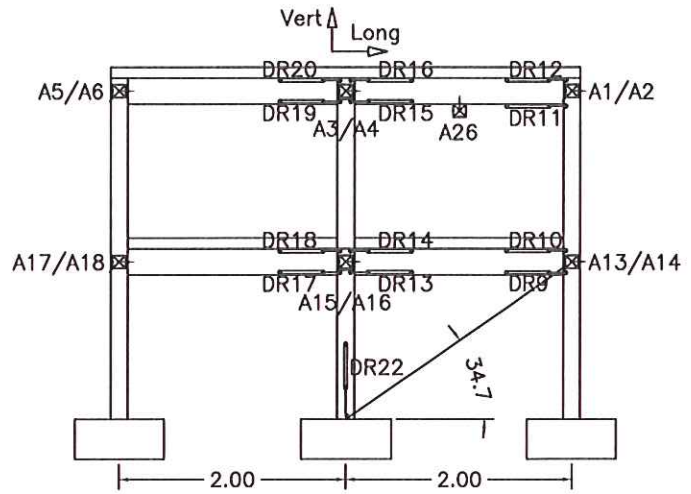
Test Setup

Views

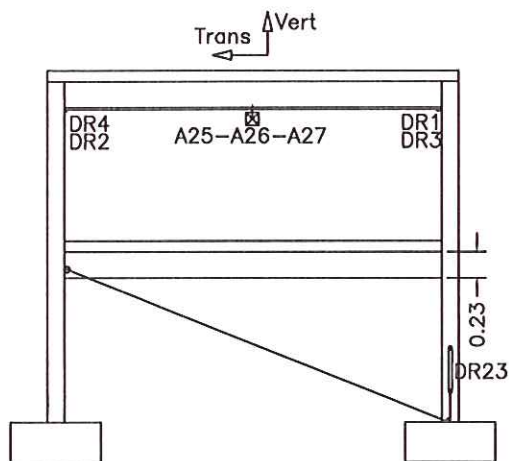
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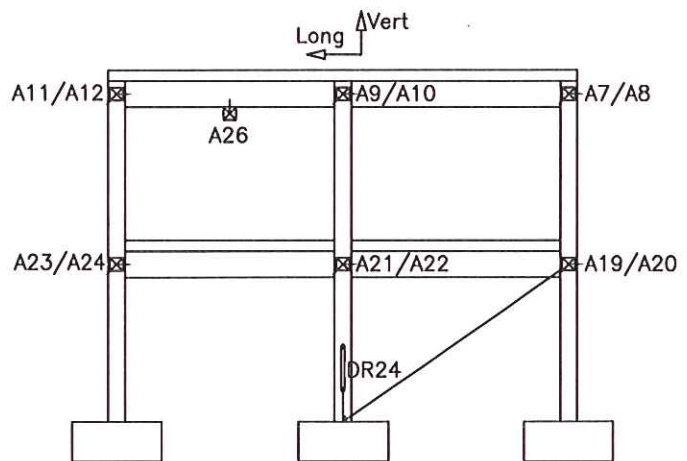
NORTH VIEW



EAST VIEW



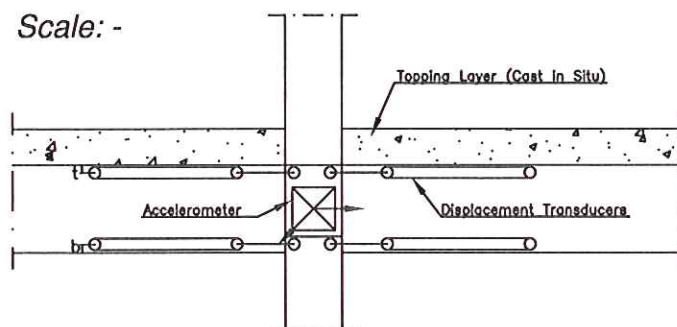
SOUTH VIEW



WEST VIEW

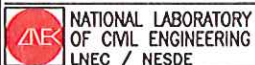
Detail

Scale: -



LEGEND	
	Optical displacement transducers (lens/target led)
	Accelerometers (Arrows represents directions measured)
	LVDT displacement transducers

Dimensions in meters



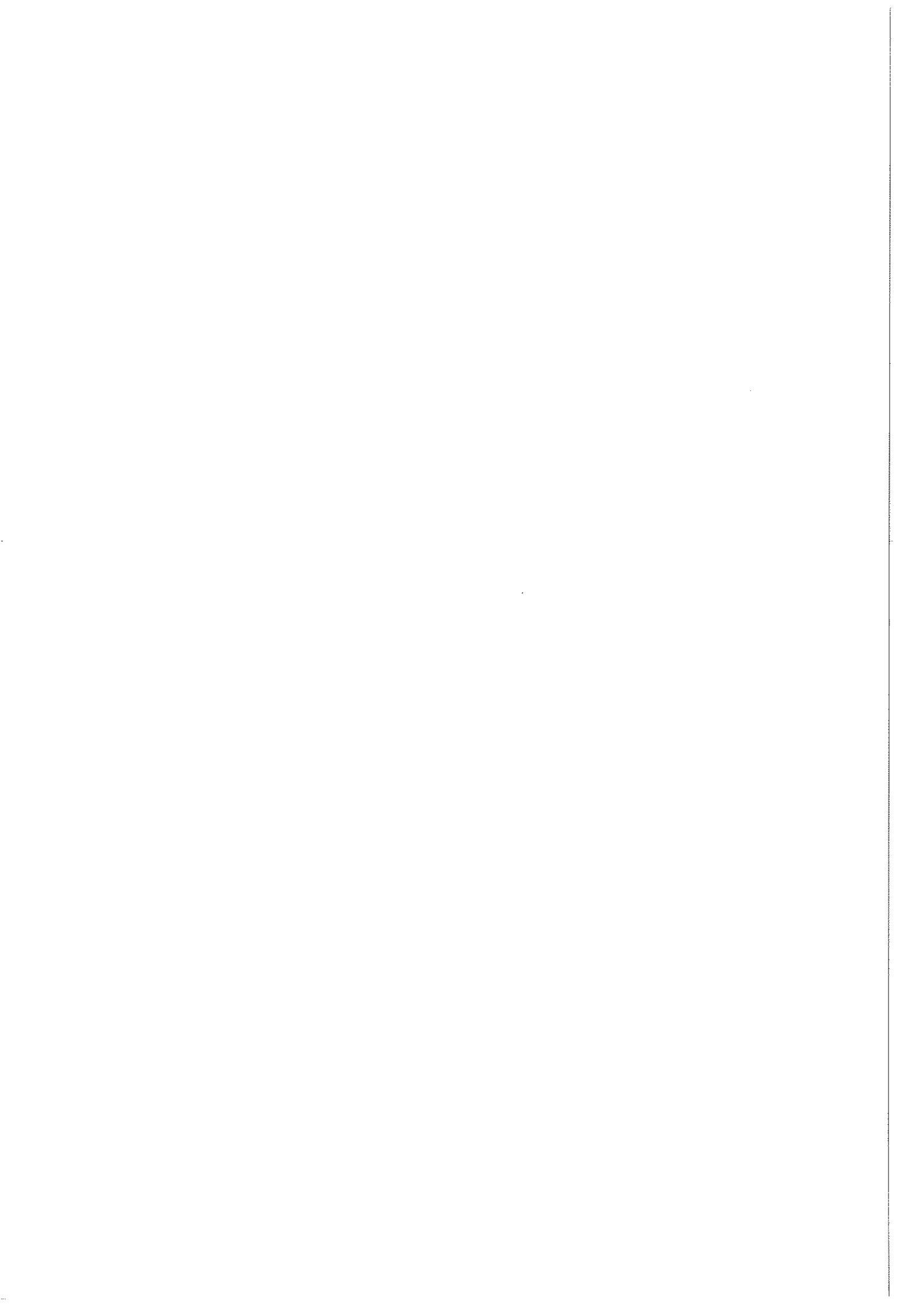
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PRECAST STRUCTURES EC8

Test Setup – Views

Dec 2005

Drawing N°
2



ANNEX F PHOTOGRAPHIC REPORT

In this annex is presented a selection of photographs taken before, during and after the test. On the figure's captions there is an identifier label to be matched with the positions indicated on the test setup drawings.

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Figure F-35: Detail of cracking on the topping layer (J3).....	F-18
Figure F-36: Detail of cracking on the topping layer (J1).....	F-18

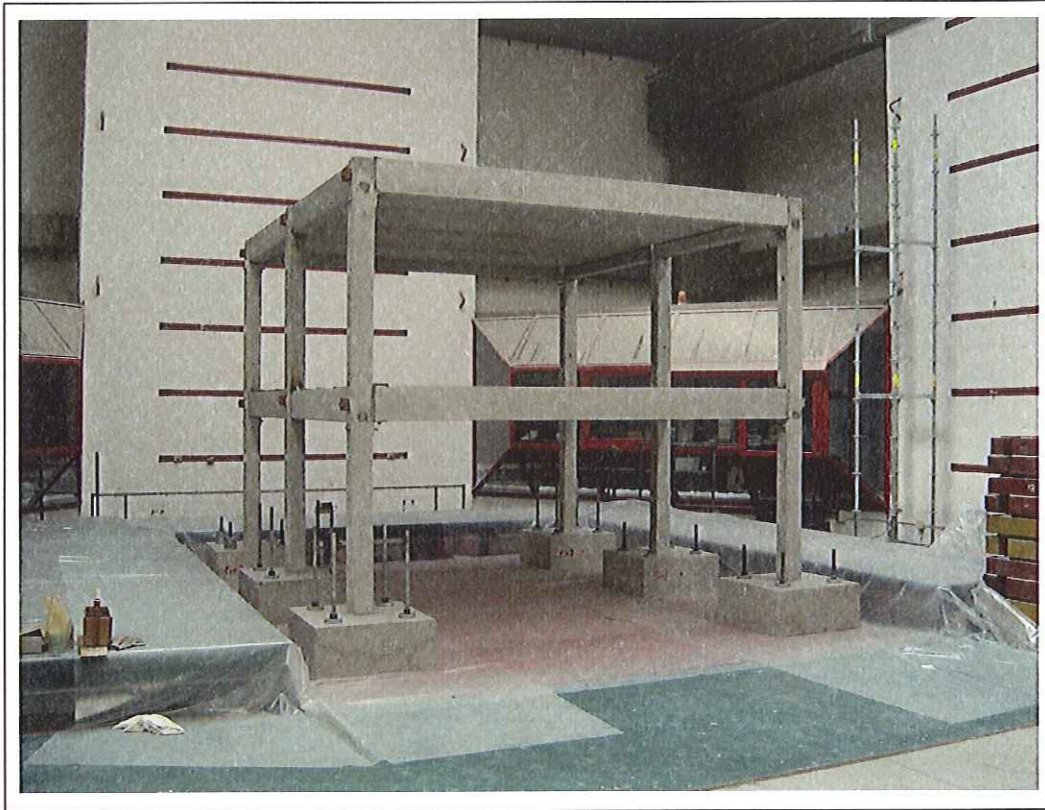


Figure F-1: General view of the model after erection.

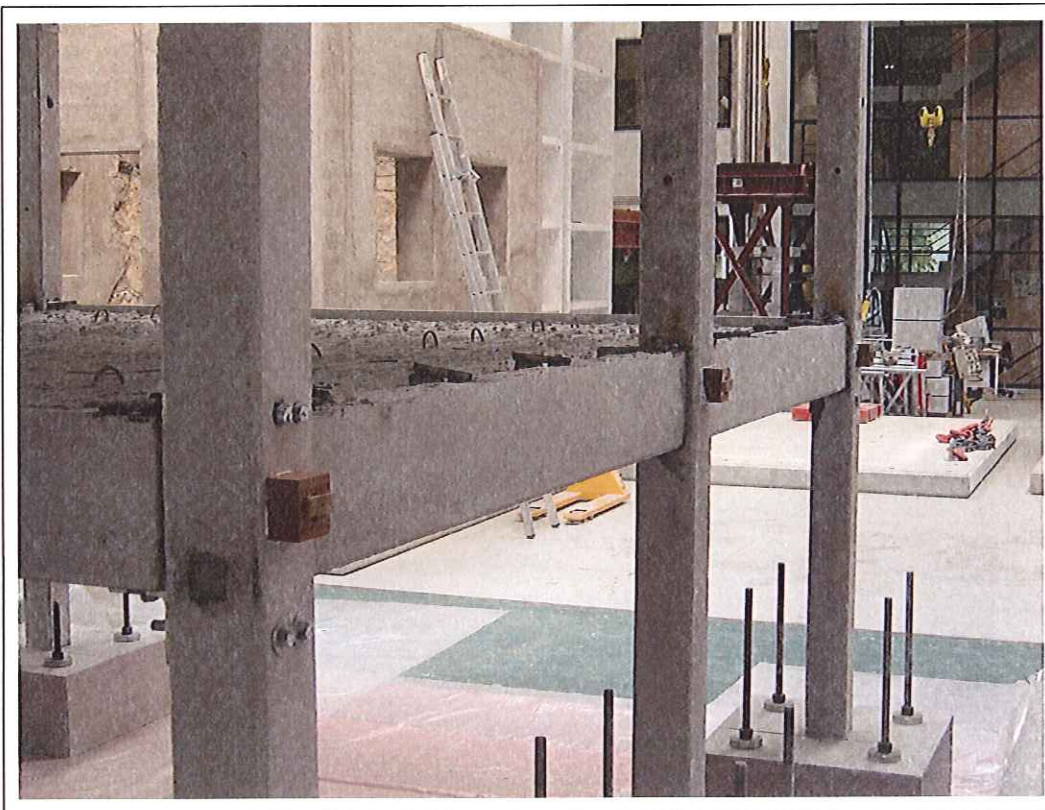


Figure F-2: General view – Longitudinal direction



Figure F-3: Detail of the beam-column connections (J3-Long.)



Figure F-4: Detail of the beam-column connections (J2-Long.)



Figure F-5: Detail of the beam-column connections (J6-Trans.)



Figure F-6: Detail of the contact points between the slabs and the longitudinal beams



Figure F-7: Detail of the joint sealing operation (J7-Long.)



Figure F-8: General view of the first floor slab after concreting the topping layer

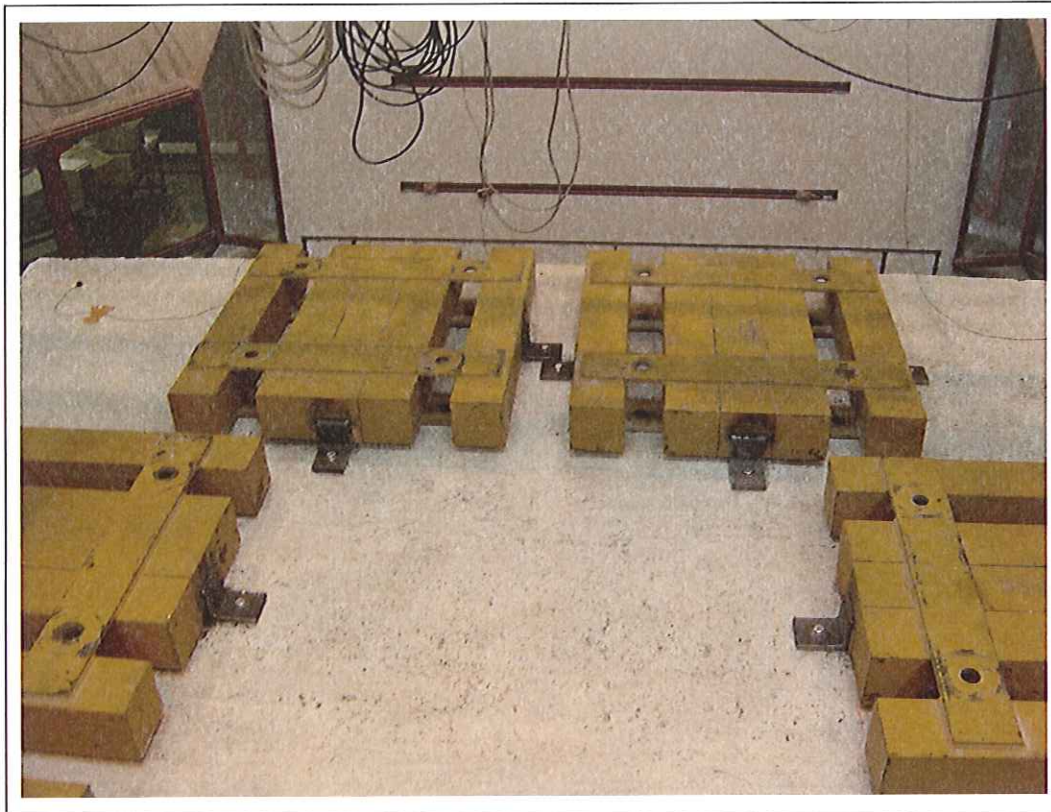


Figure F-9: Detail of the additional masses and the fixing system



Figure F-10: General view of the model before the test



Figure F-11: General view of the test setup (NE View)



Figure F-12: General view of the test setup (SW View)

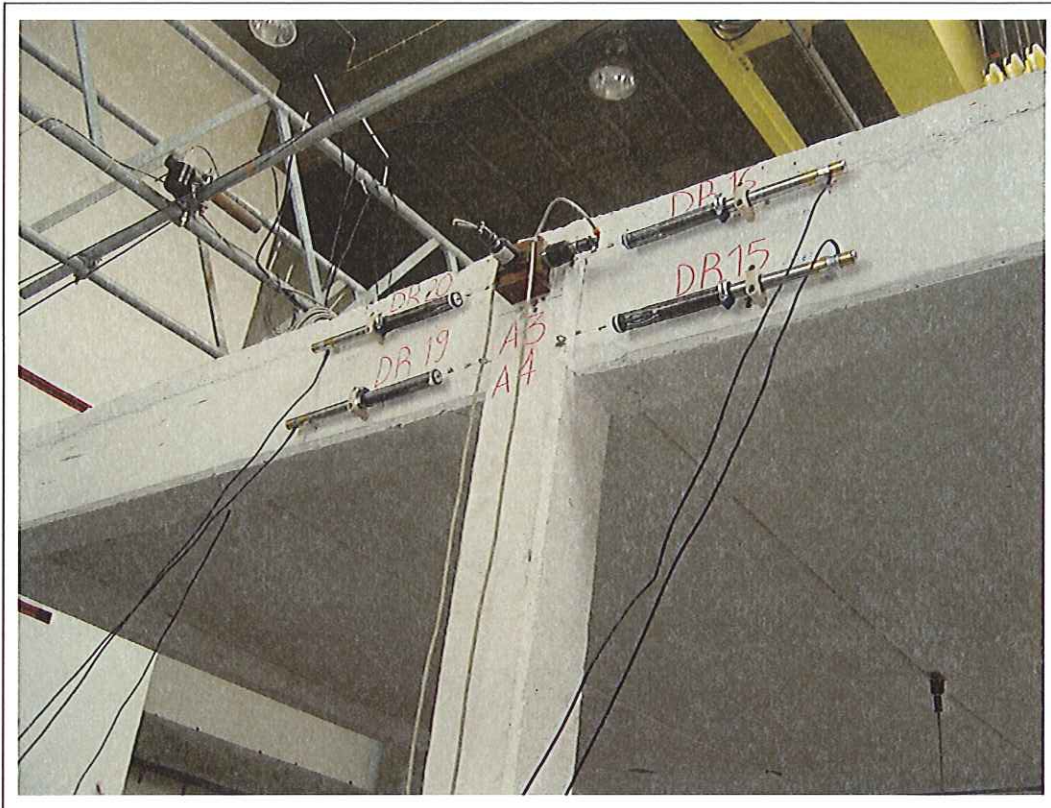


Figure F-13: Detail of the accelerometers and displacement transducers at the joints (J8-Long.)

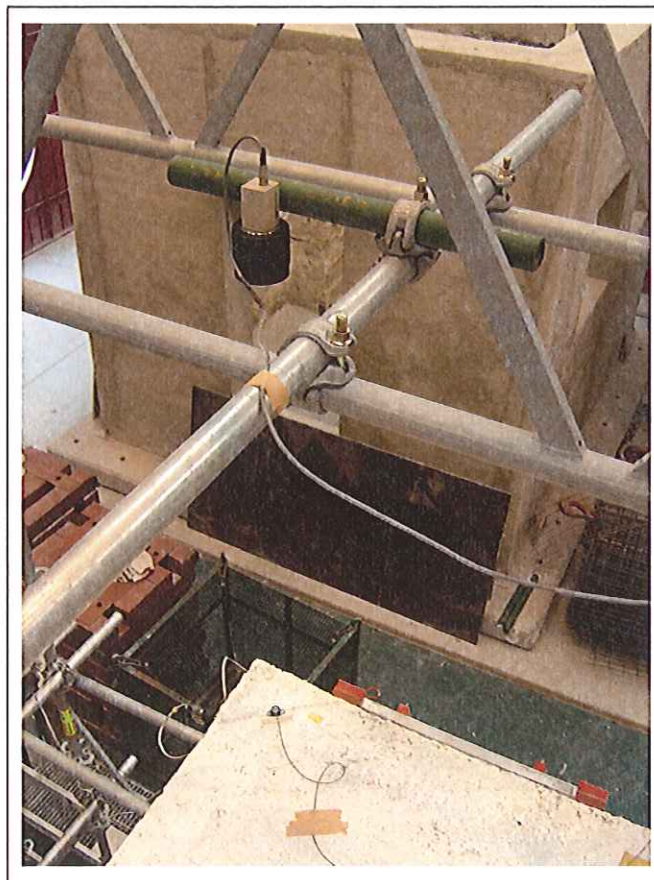


Figure F-14: Detail of the optical displacement transducers (J10)

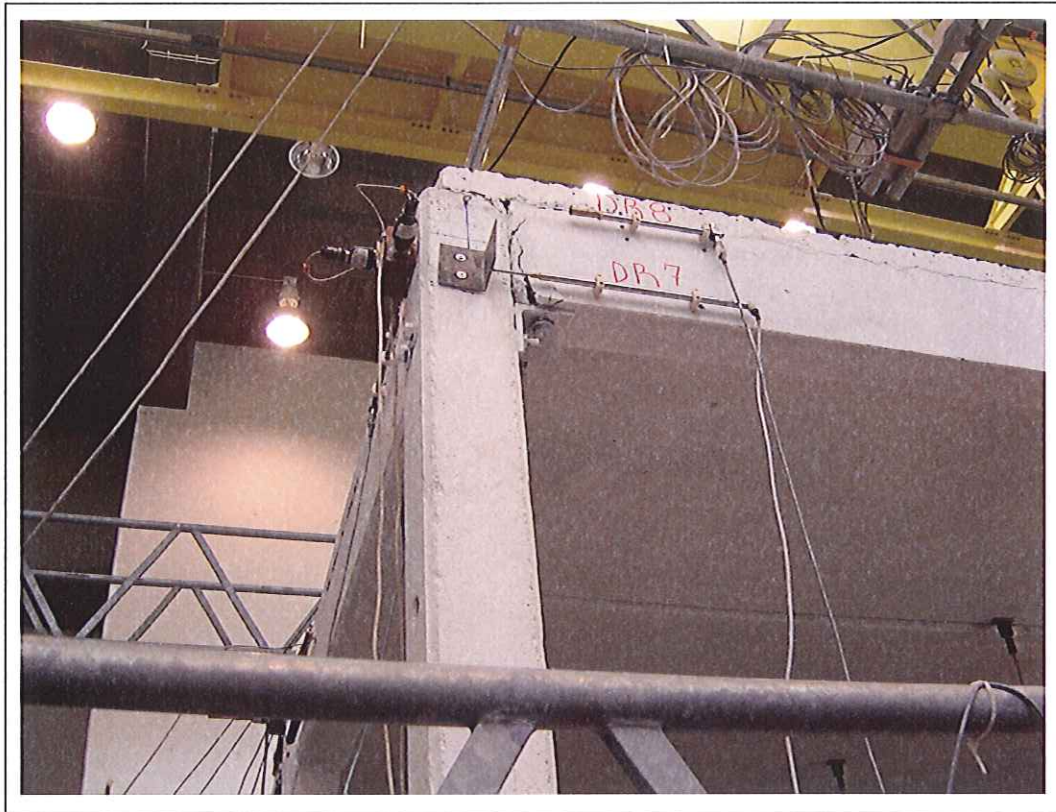


Figure F-15: Detail of the test setup (J9 - Trans.)



Figure F-16: Detail of the inductive displacement transducers (C4)



Figure F-17: Detail of the beam-column connection after stage 02 (J6-Trans.)



Figure F-18: Detail of the beam-column connection after stage 02 (J11-Long.)

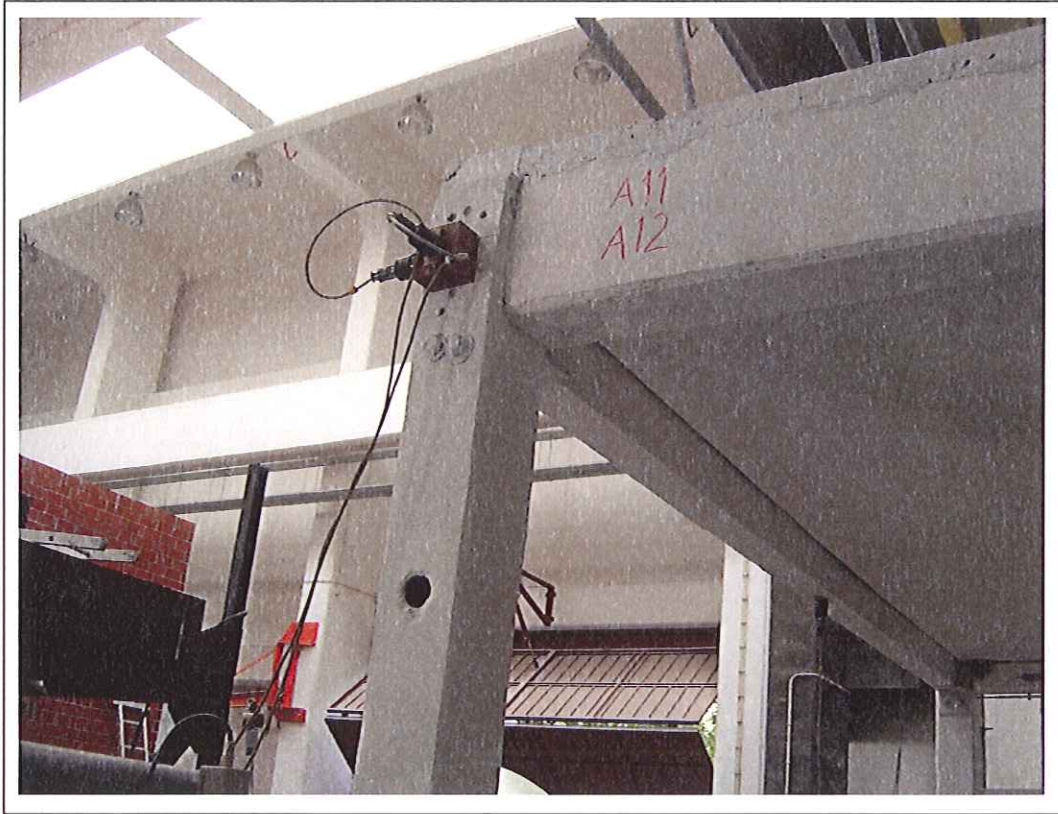


Figure F-19: Detail of the beam-column connection after stage 02 (J10-Long.)

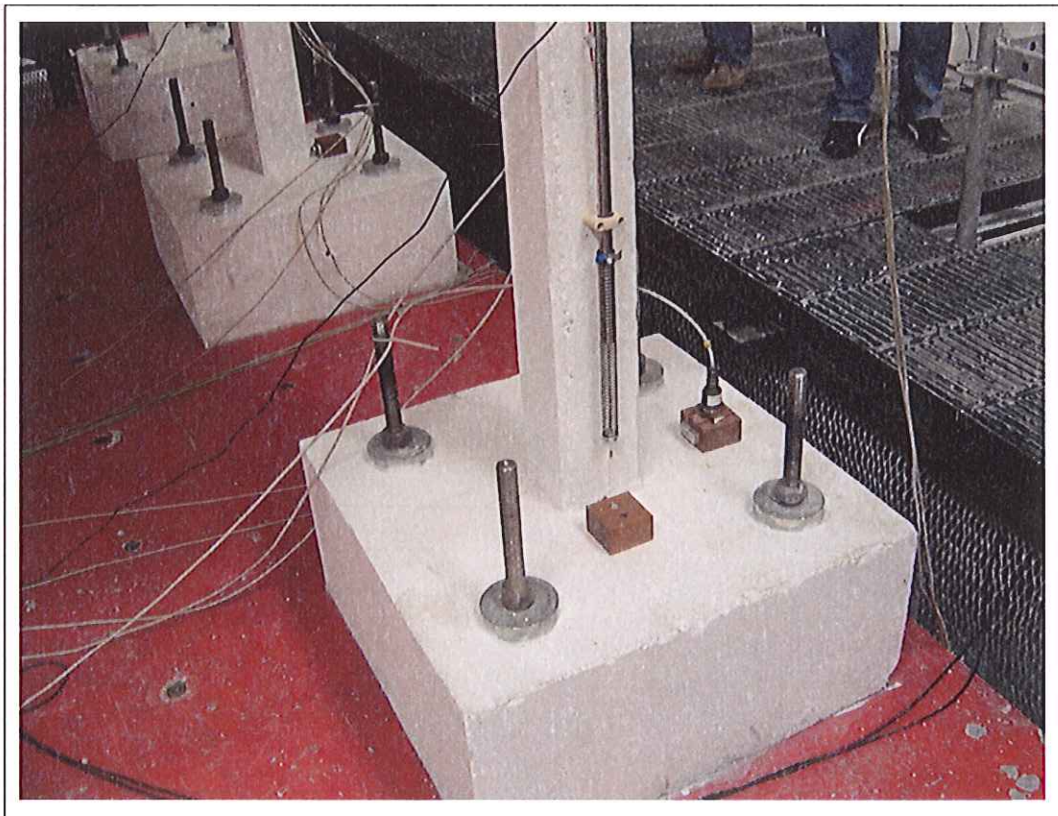


Figure F-20: Detail of the base of the column C4 after stage 02



Figure F-21: Detail of the beam-column connection after the test (J9-Trans.)



Figure F-22: Detail of the beam-column connection's inner part after the test (J9-Trans.)

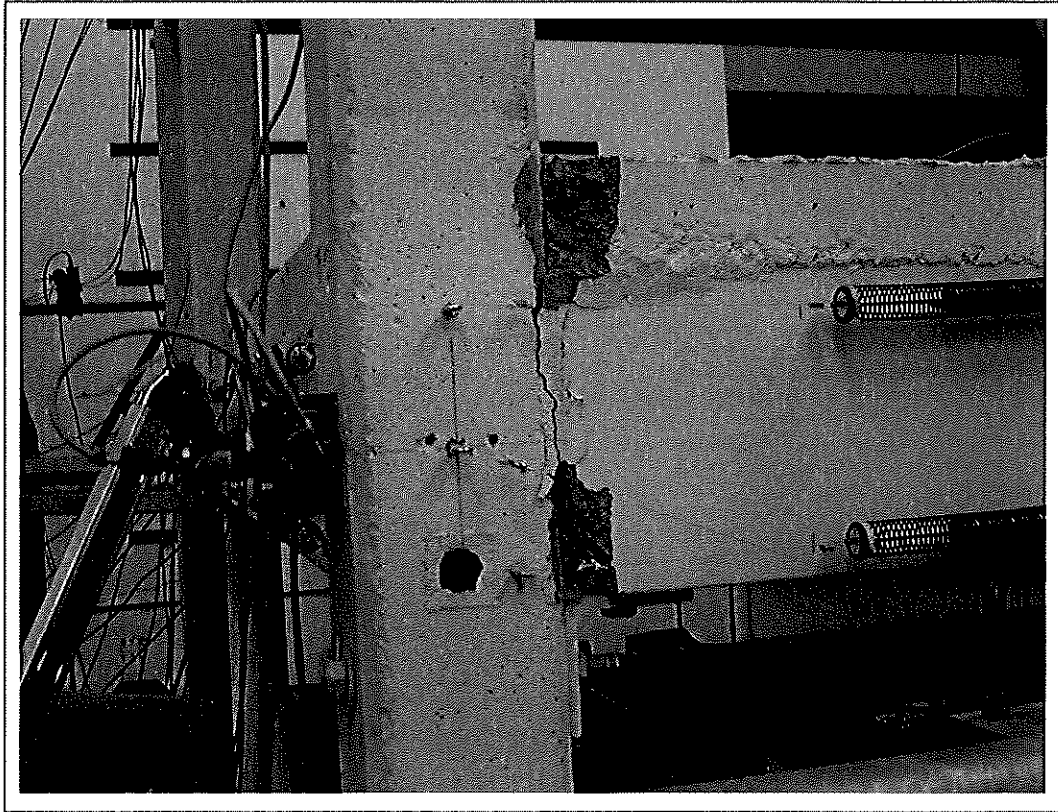


Figure F-23: Detail of the beam-column connection after the test (J3-Trans.)

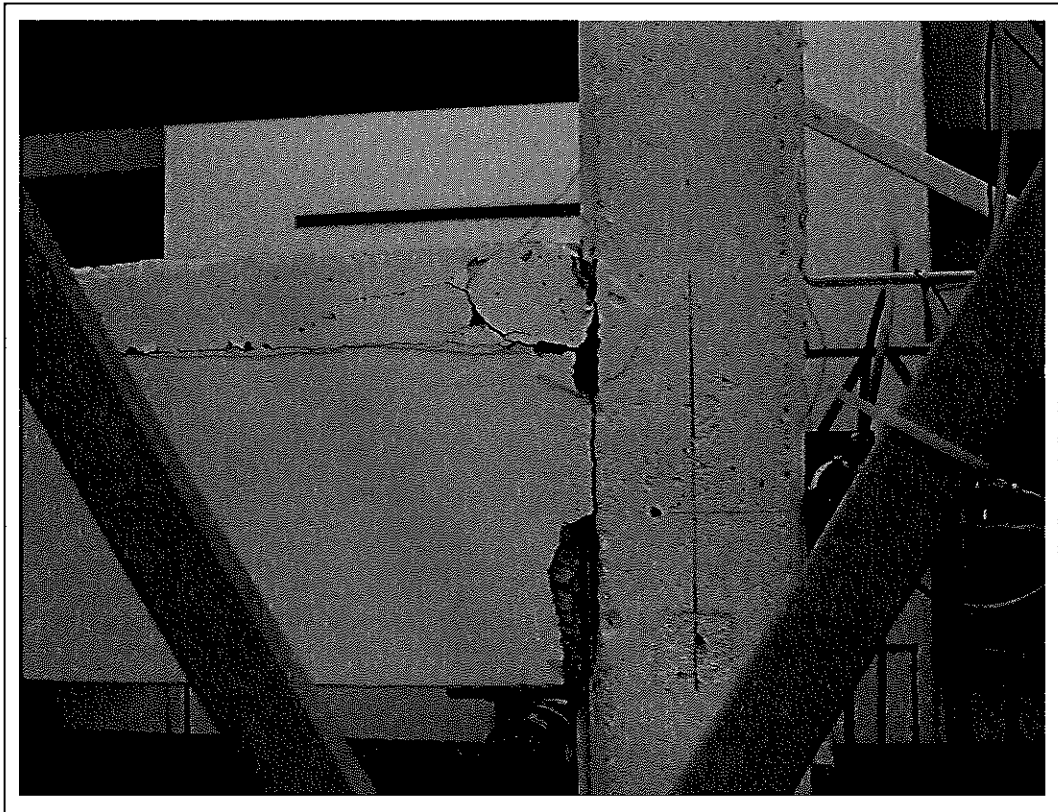


Figure F-24: Detail of the beam-column connection after the test (J4-Trans.)

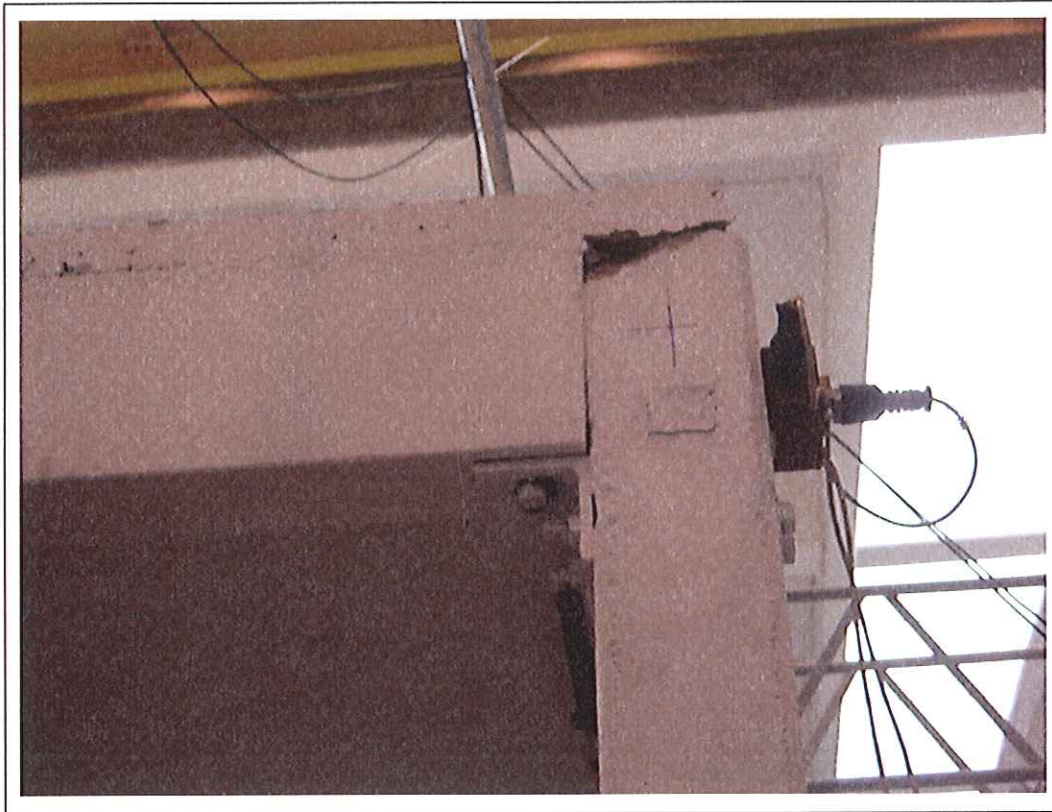


Figure F-25: Detail of the beam-column connection after the test (J7-Trans.)

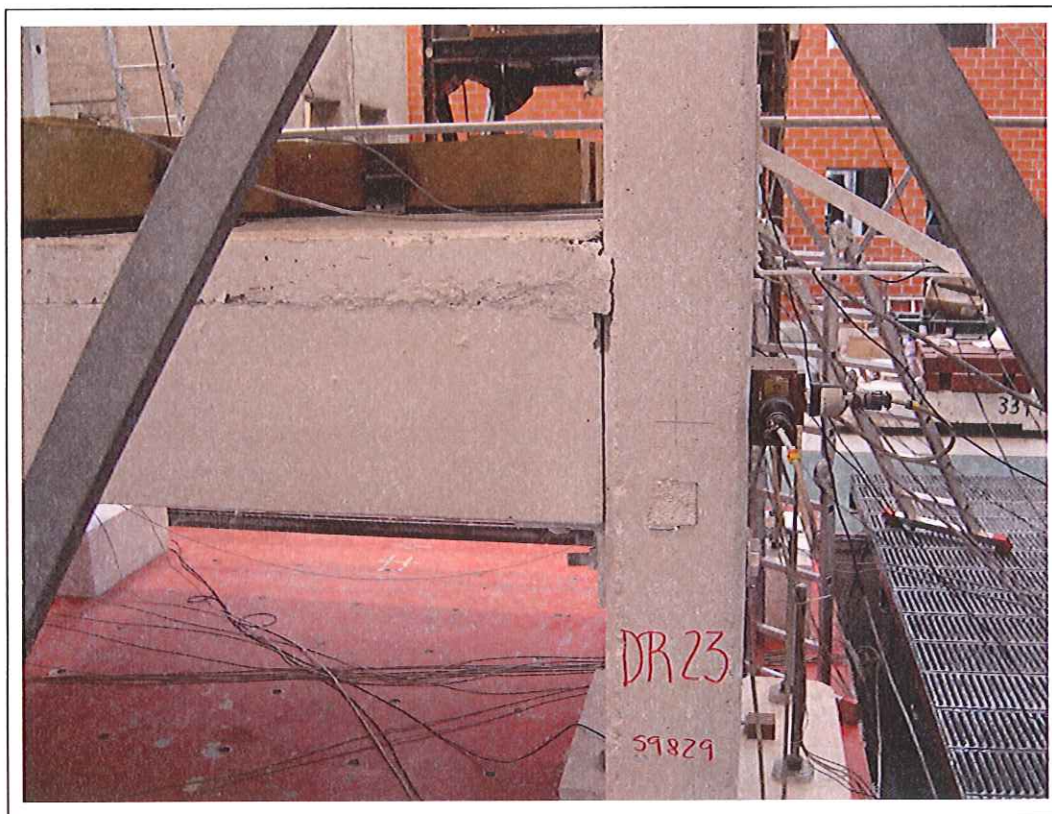


Figure F-26: Detail of the beam-column connection after the test (J1-Trans.)



Figure F-27: Detail of the beam-column connection after the test (J6-Long.)

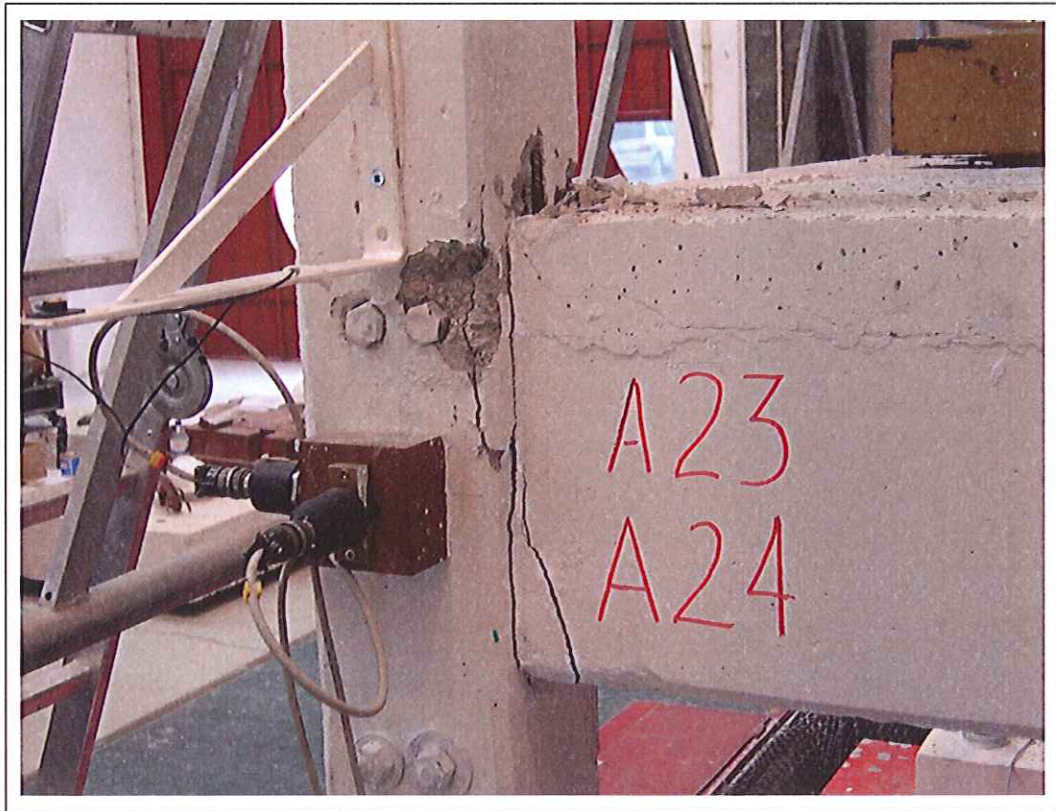


Figure F-28: Detail of the beam-column connection after the test (J10-Long.)



Figure F-29: Detail of the beam-column connection after the test (J1-Long.)

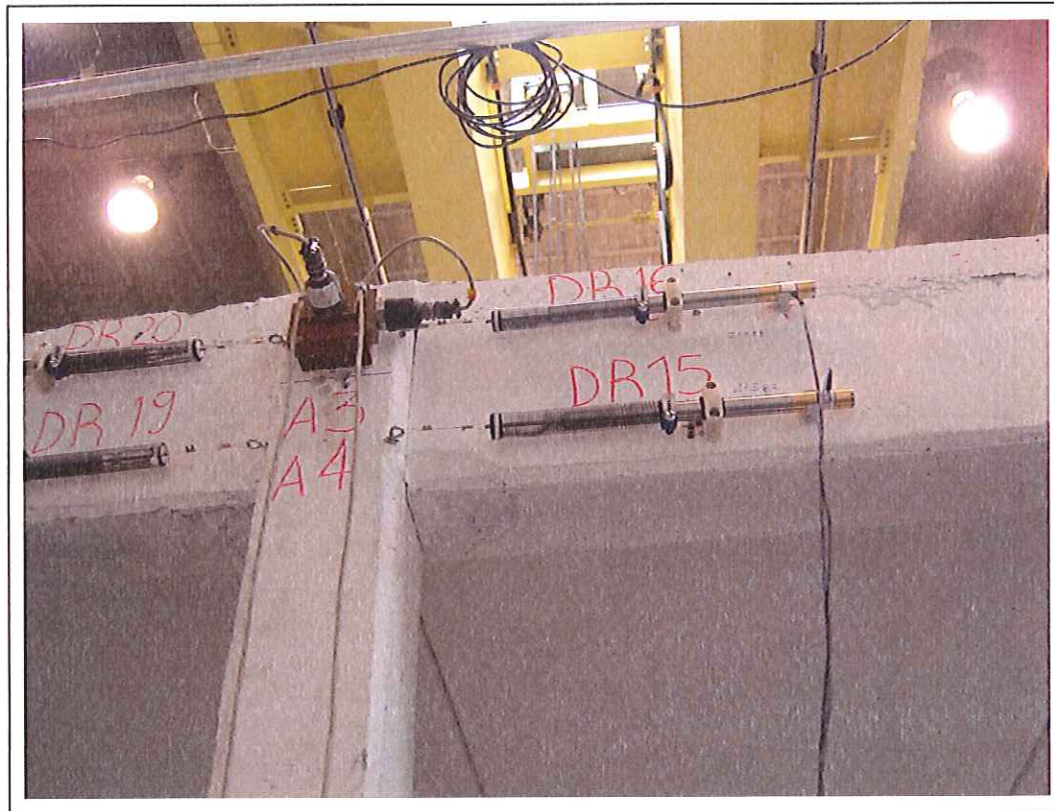


Figure F-30: Detail of the beam-column connection after the test (J8-Long.)

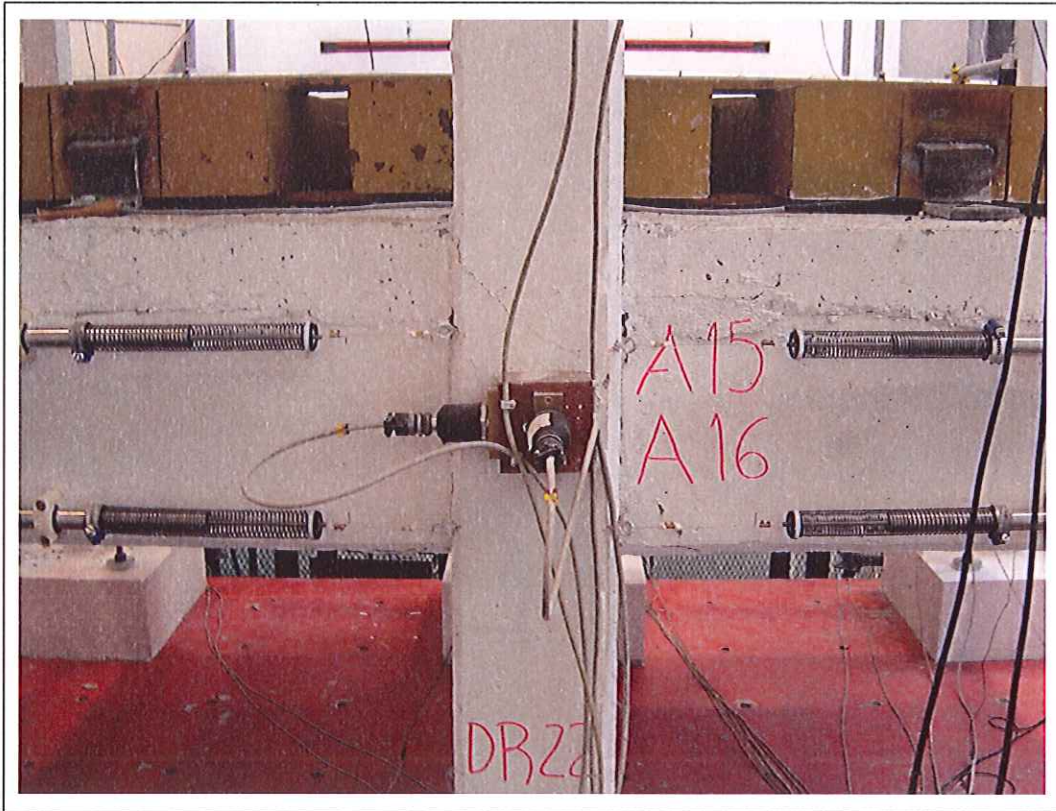


Figure F-31: Detail of the beam-column connection after the test (J2-Long.)

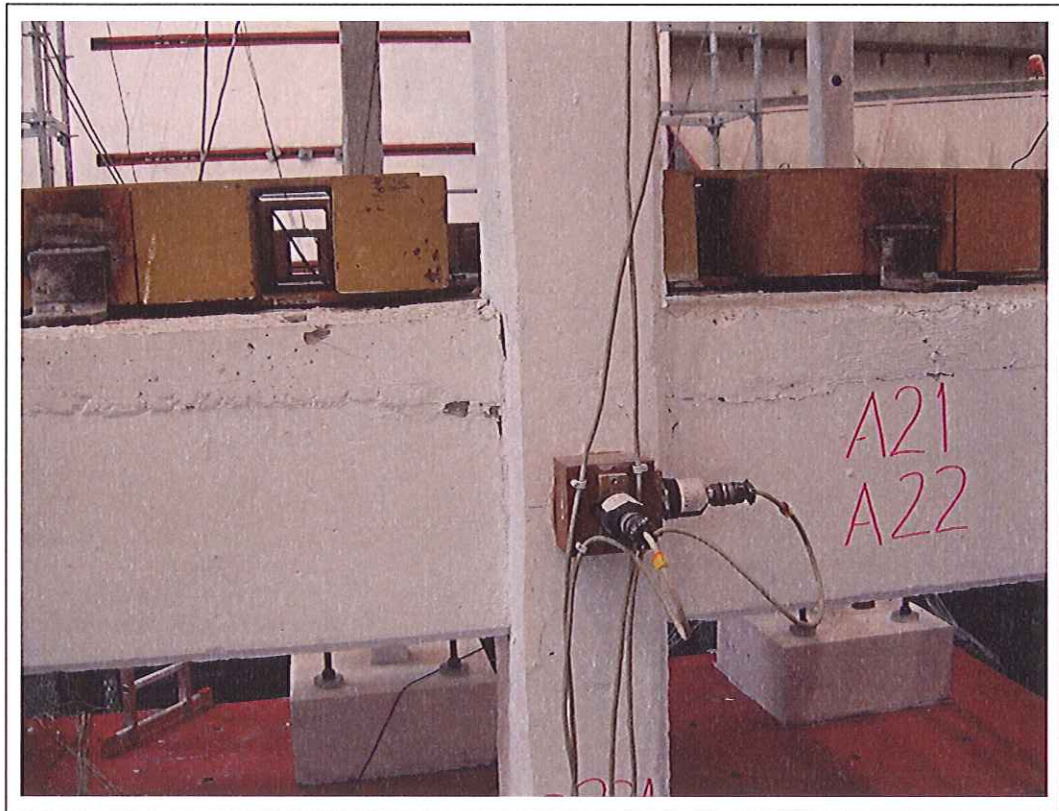


Figure F-32: Detail of the beam-column connection after the test (J5-Long.)



Figure F-33: Detail of the base of the column C3 after the test

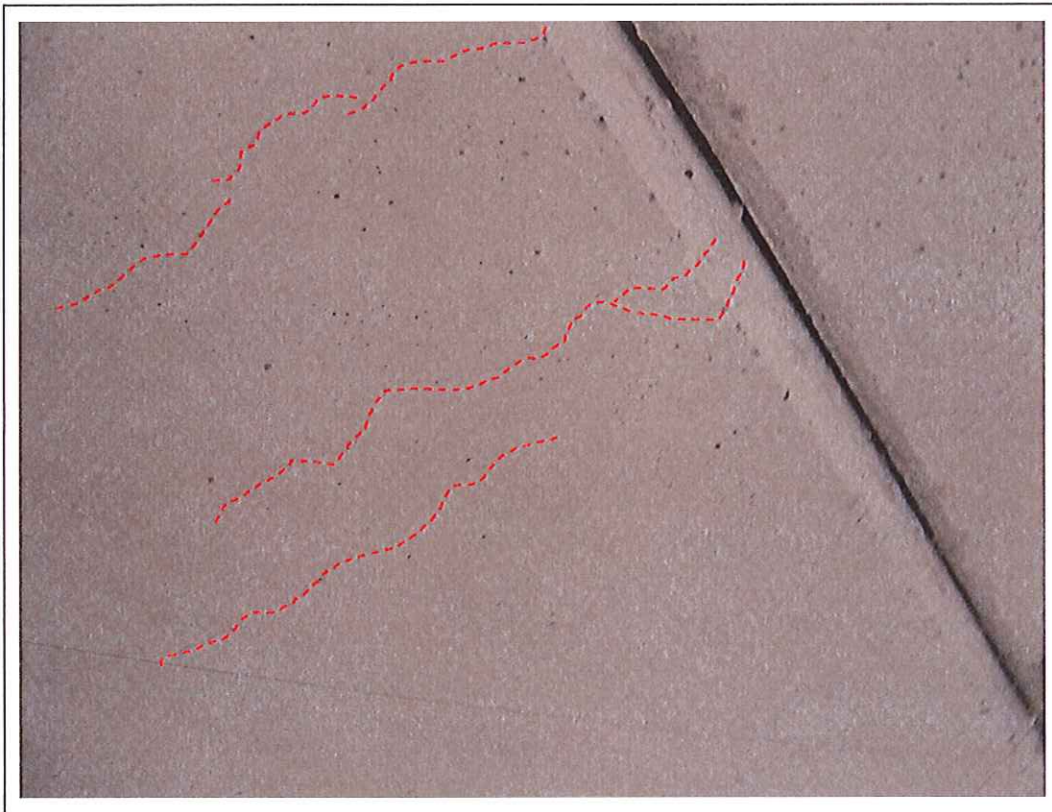


Figure F-34: Detail of cracking (bottom of the 2nd floor slab)

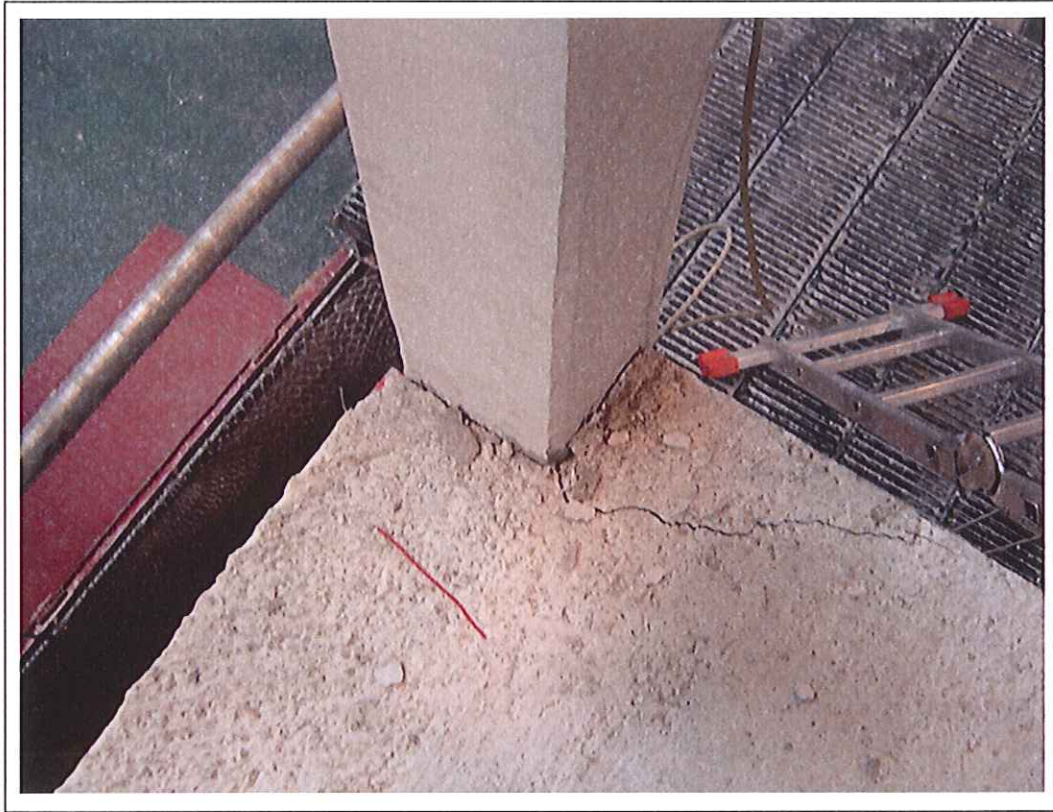


Figure F-35: Detail of cracking on the topping layer (J3)



Figure F-36: Detail of cracking on the topping layer (J1)

ANNEX G EXPERIMENTAL MODAL FREQUENCIES

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Figure G-4: FRF peak picking – Stage 00 – Longitudinal (1st Floor) G-4

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Figure G-8: FRF peak picking – Stage 01 – Longitudinal (1st Floor) G-6

Figure G-9: FRF peak picking – Stage 02 – Transversal (2nd Floor) G-7

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Figure G-12: FRF peak picking – Stage 02 – Longitudinal (1st Floor) G-8

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Figure G-14: FRF peak picking – Stage 03 – Transversal (1st Floor) G-9

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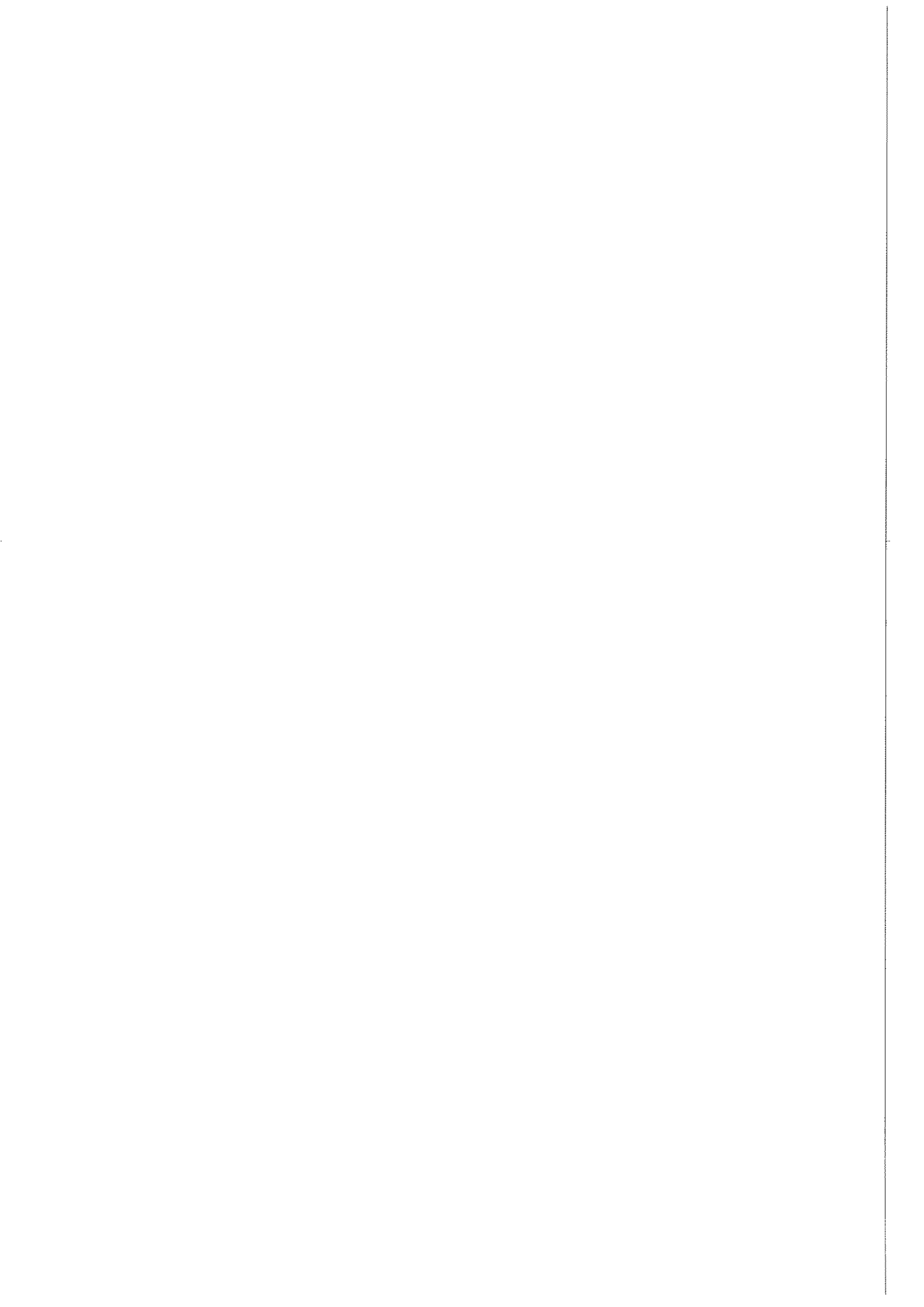
Figure G-16: FRF peak picking – Stage 03 – Longitudinal (1st Floor) G-10

Figure G-17: FRF peak picking – Stage 04 – Transversal (2nd Floor) G-11

Figure G-18: FRF peak picking – Stage 04 – Transversal (1st Floor) G-11

Figure G-19: FRF peak picking – Stage 04 – Longitudinal (2nd Floor) G-12

Figure G-20: FRF peak picking – Stage 04 – Longitudinal (1st Floor) G-12



G.1 Cat 00

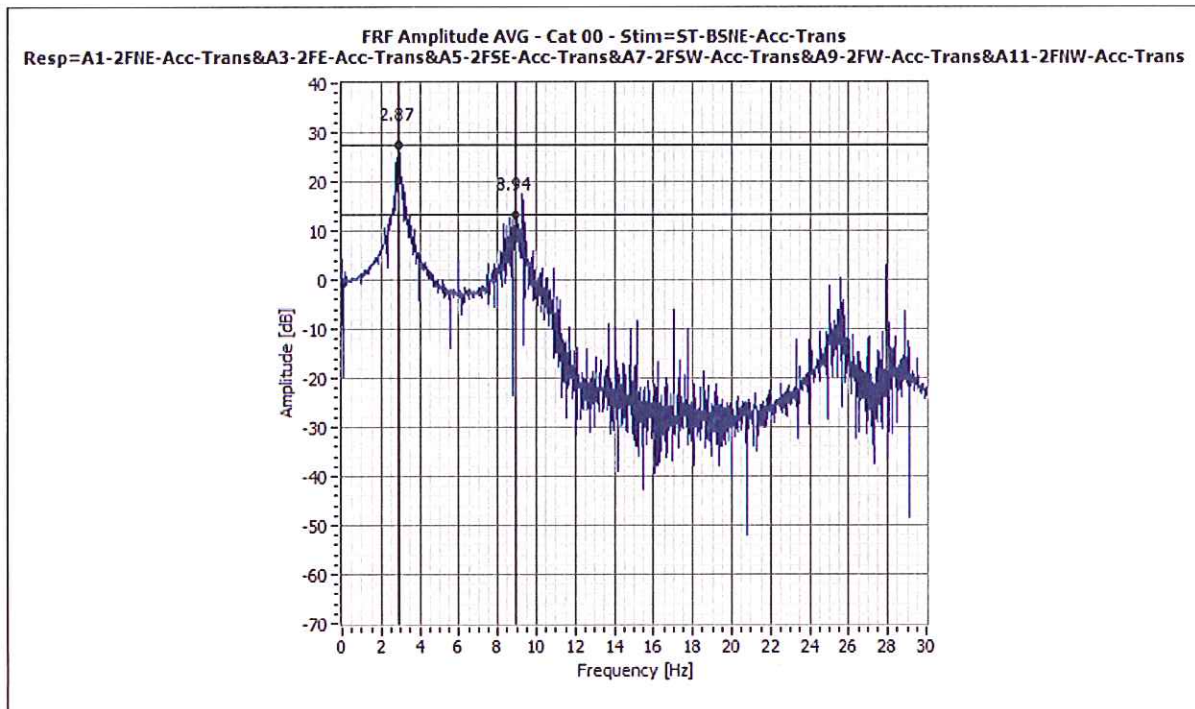


Figure G-1: FRF peak picking – Stage 00 – Transversal (2nd Floor)

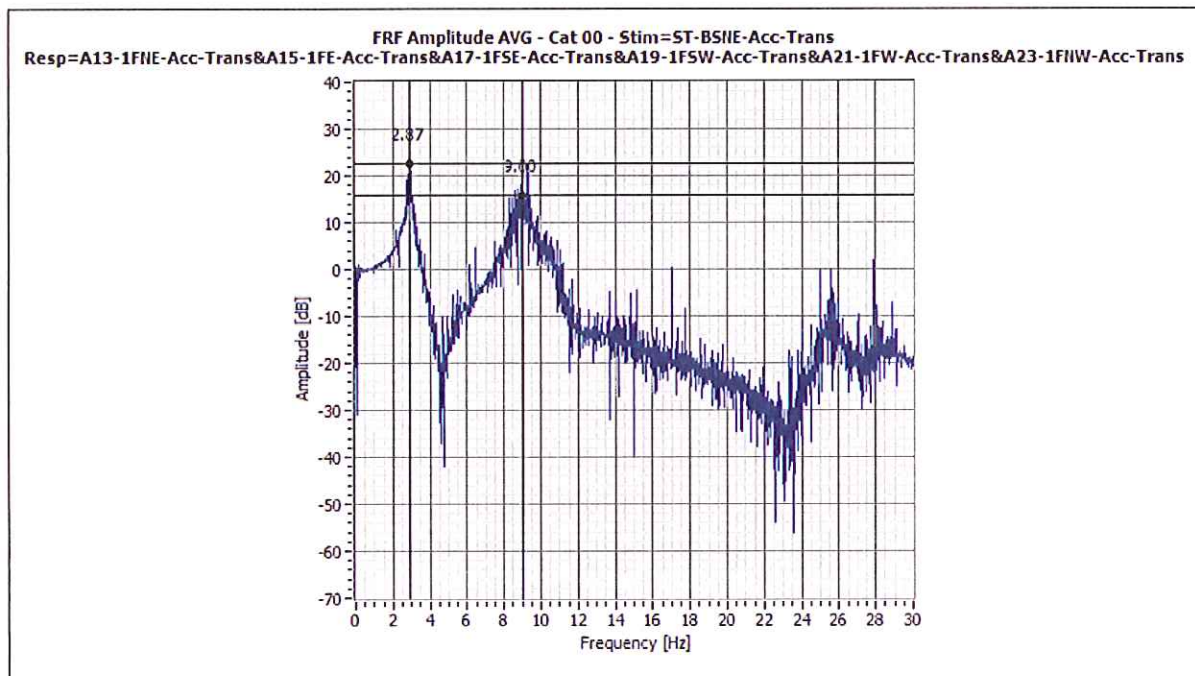


Figure G-2: FRF peak picking – Stage 00 – Transversal (1st Floor)

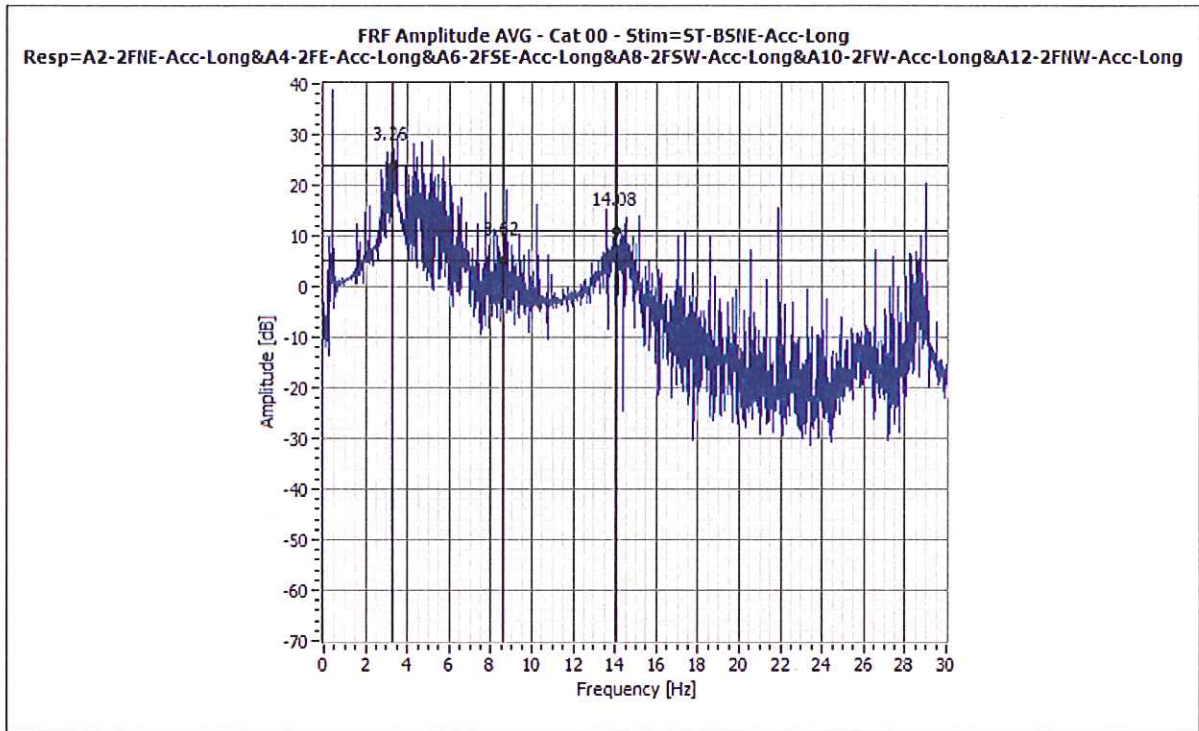


Figure G-3: FRF peak picking – Stage 00 – Longitudinal (2nd Floor)

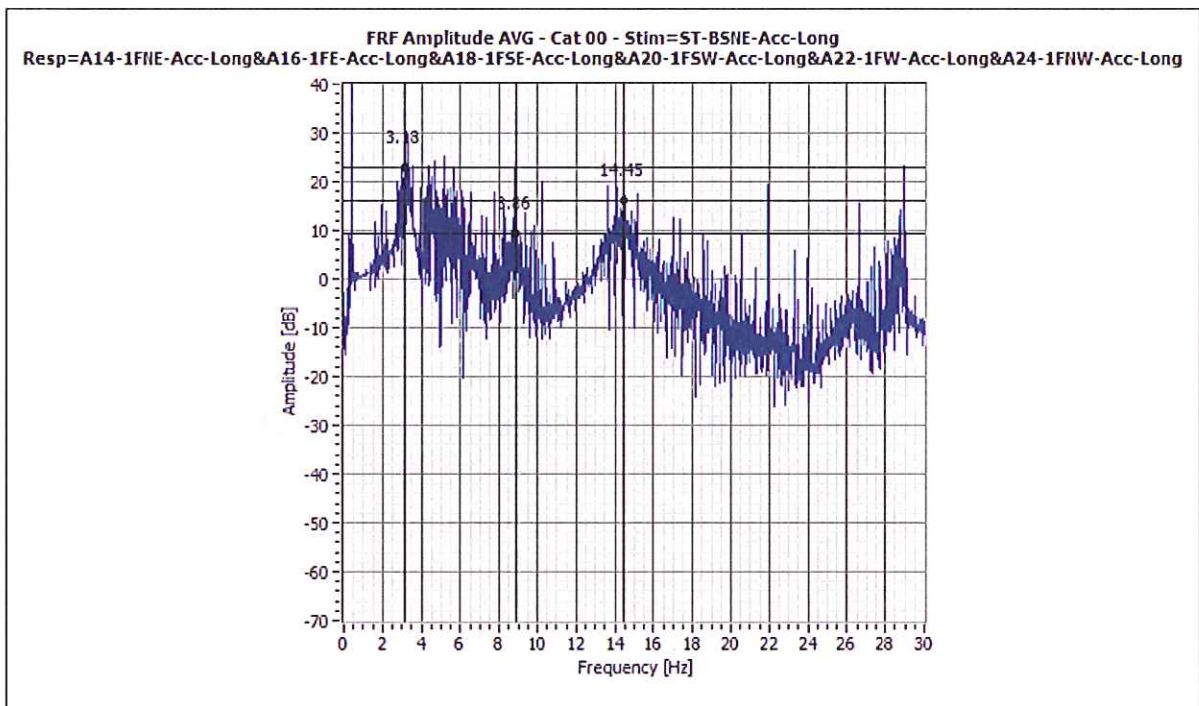


Figure G-4: FRF peak picking – Stage 00 – Longitudinal (1st Floor)

G.2 Cat 01

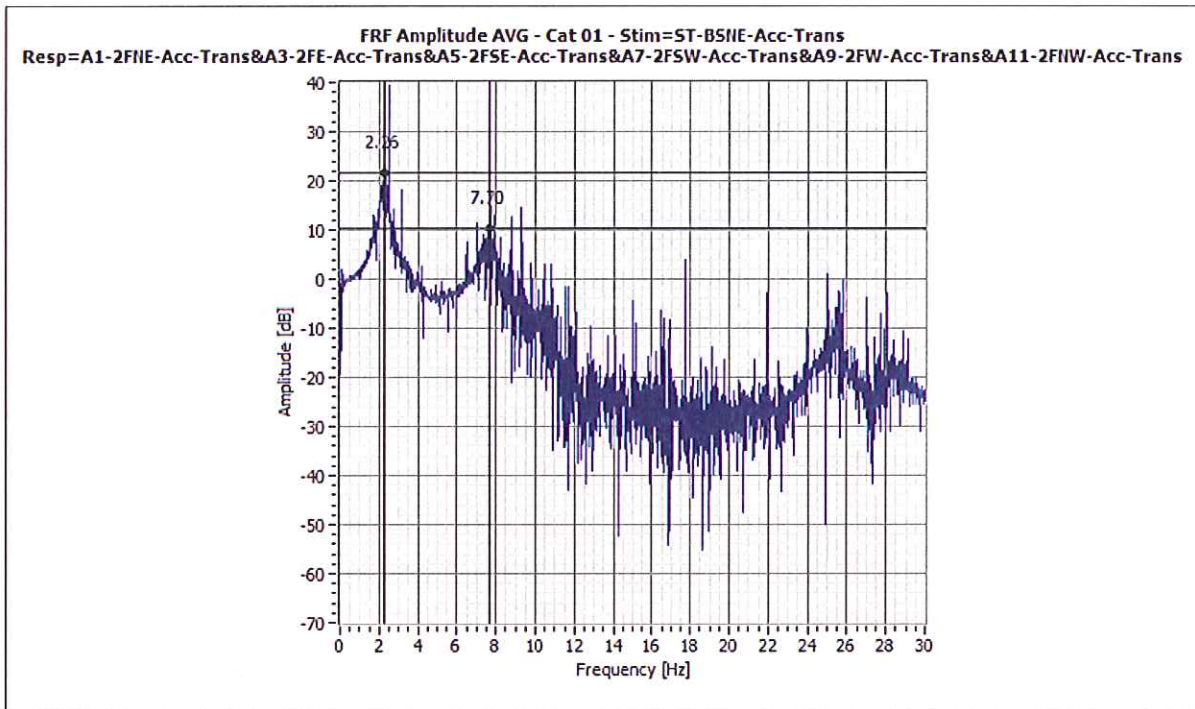


Figure G-5: FRF peak picking – Stage 01 – Transversal (2nd Floor)

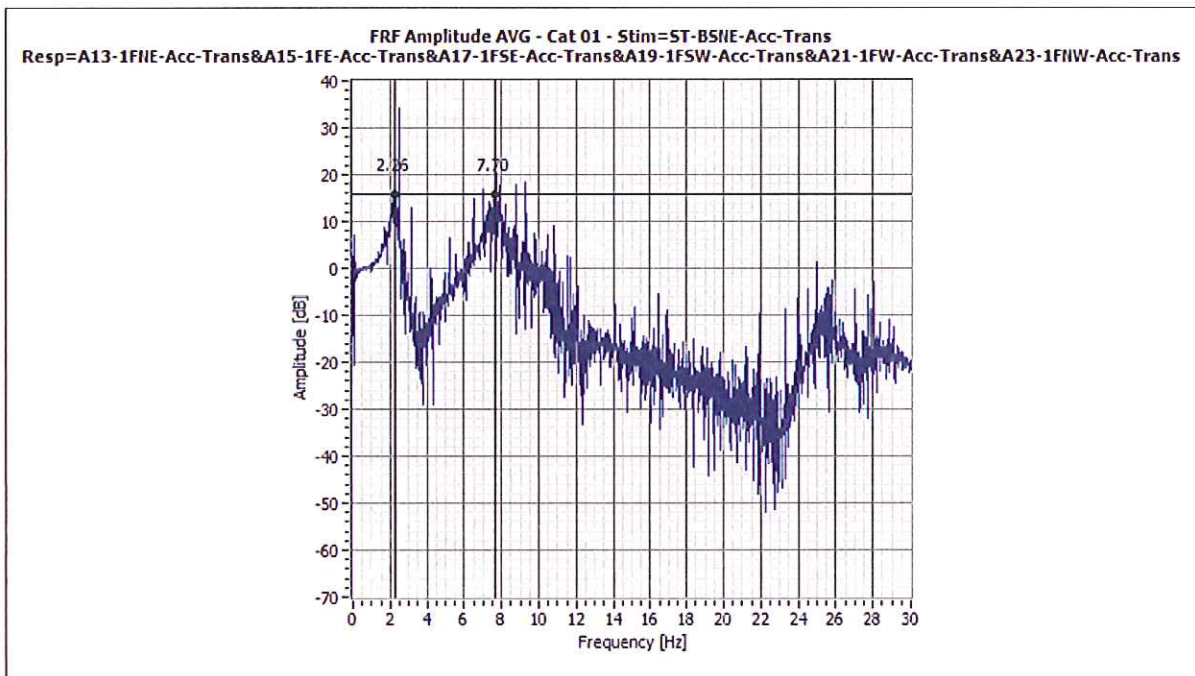


Figure G-6: FRF peak picking – Stage 01 – Transversal (1st Floor)

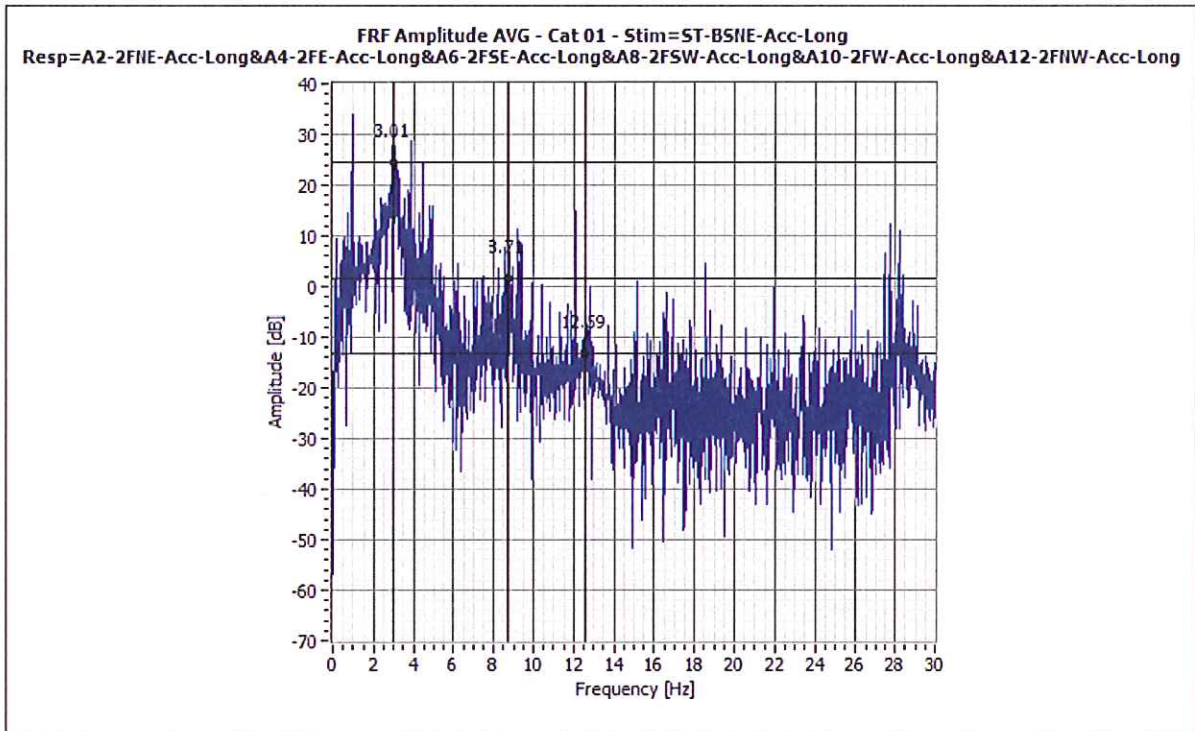


Figure G-7: FRF peak picking – Stage 01 – Longitudinal (2nd Floor)

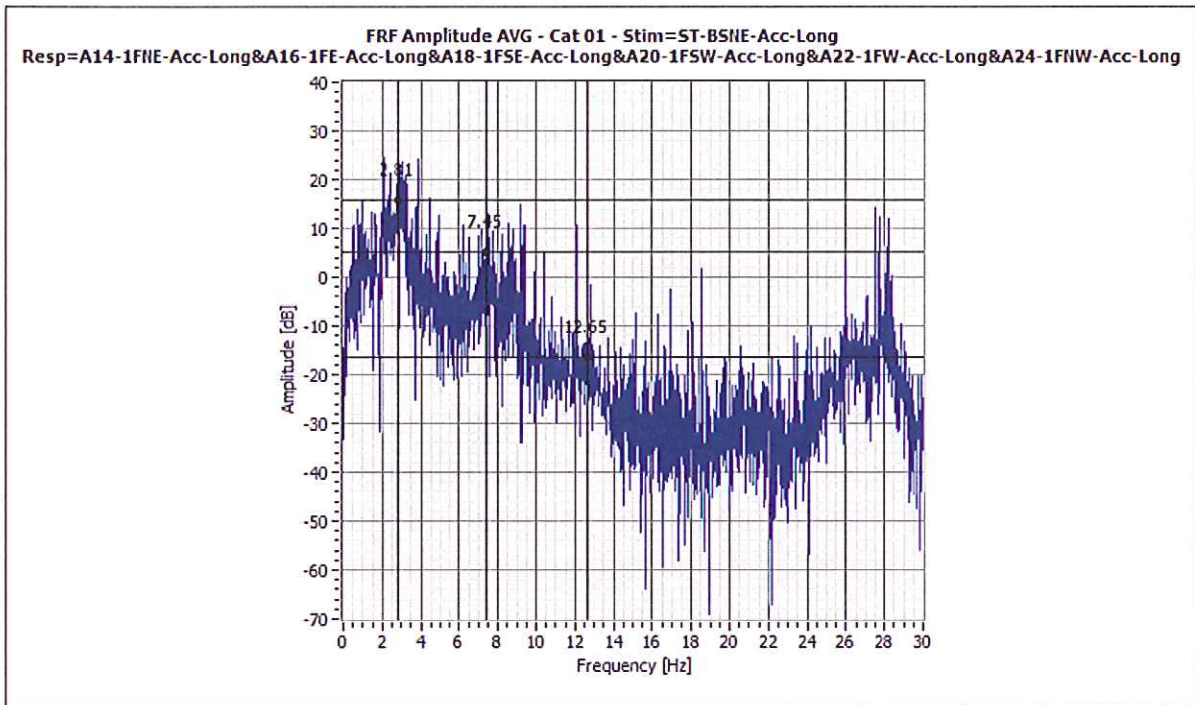


Figure G-8: FRF peak picking – Stage 01 – Longitudinal (1st Floor)

G.3 Cat 02

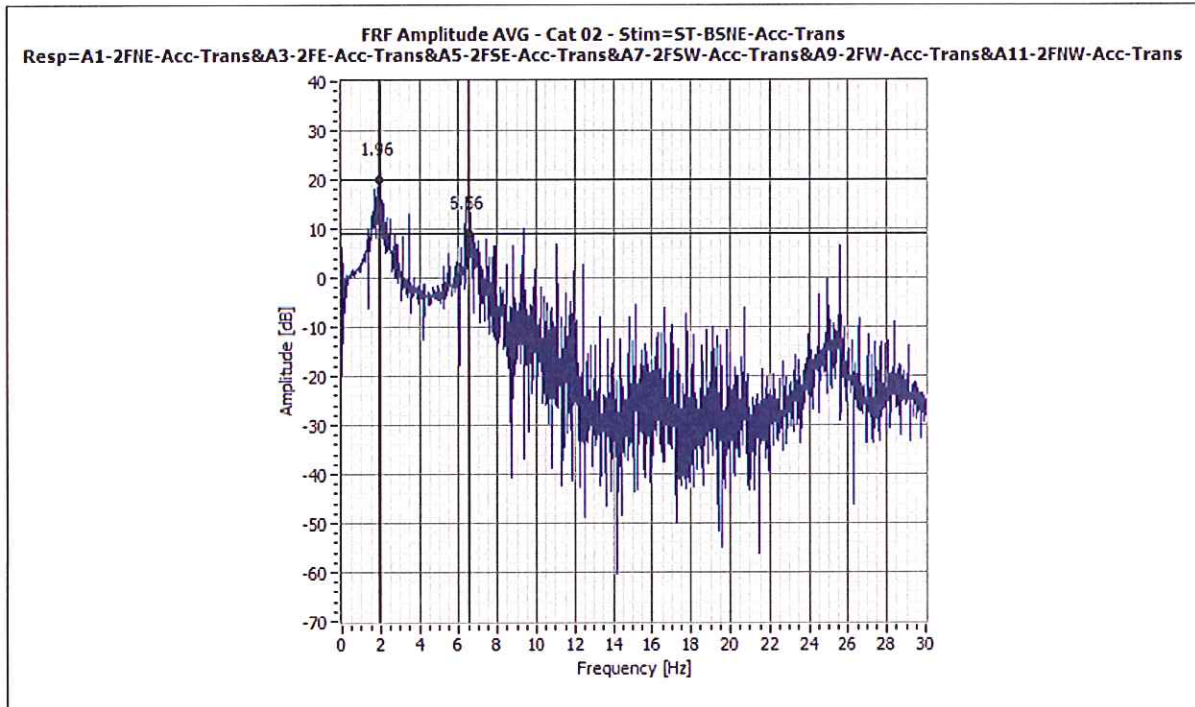


Figure G-9: FRF peak picking – Stage 02 – Transversal (2nd Floor)

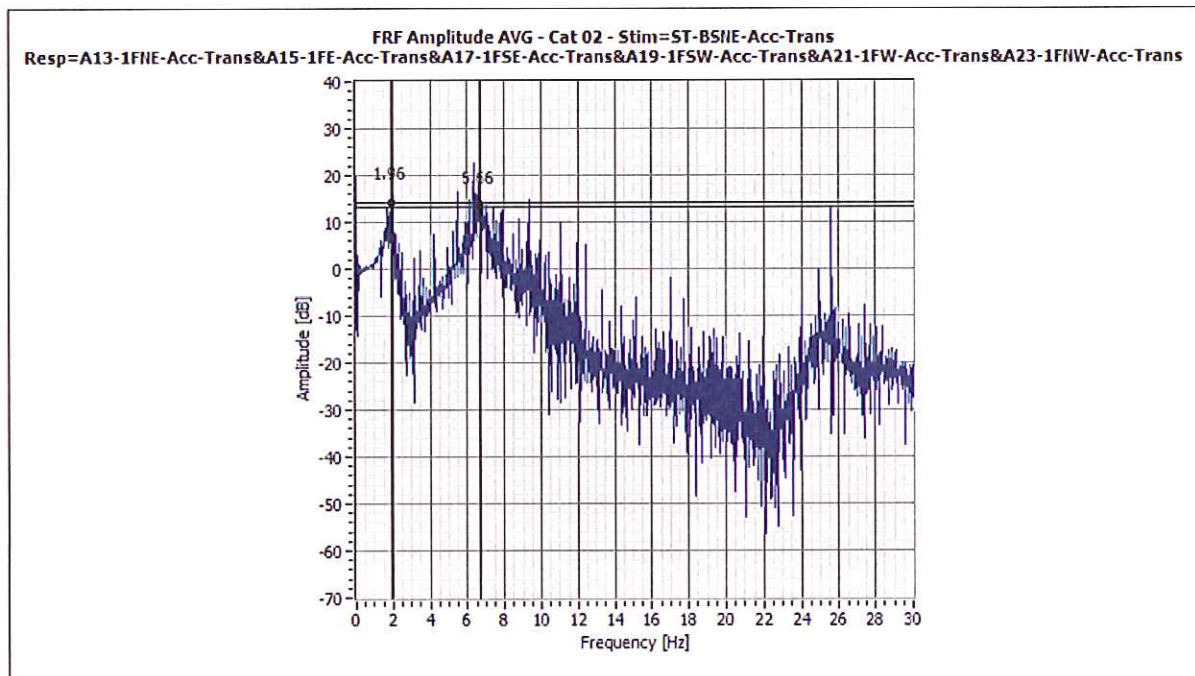


Figure G-10: FRF peak picking – Stage 02 – Transversal (1st Floor)

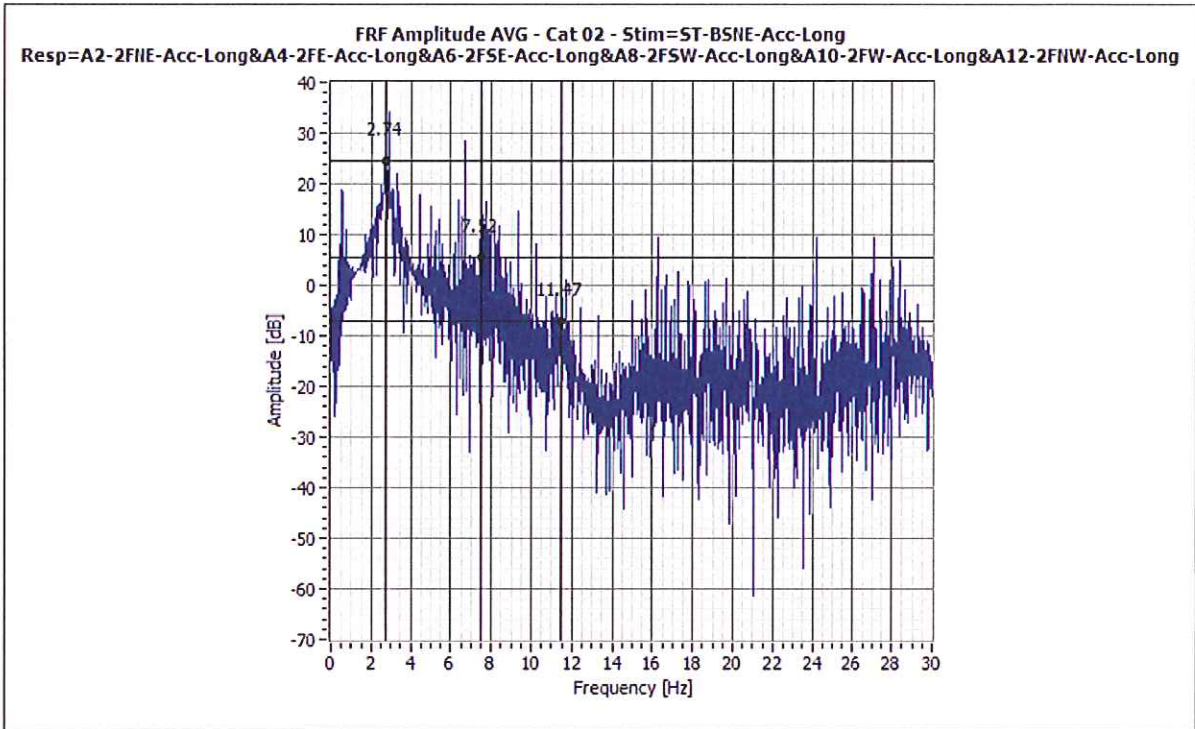


Figure G-11: FRF peak picking – Stage 02 – Longitudinal (2nd Floor)

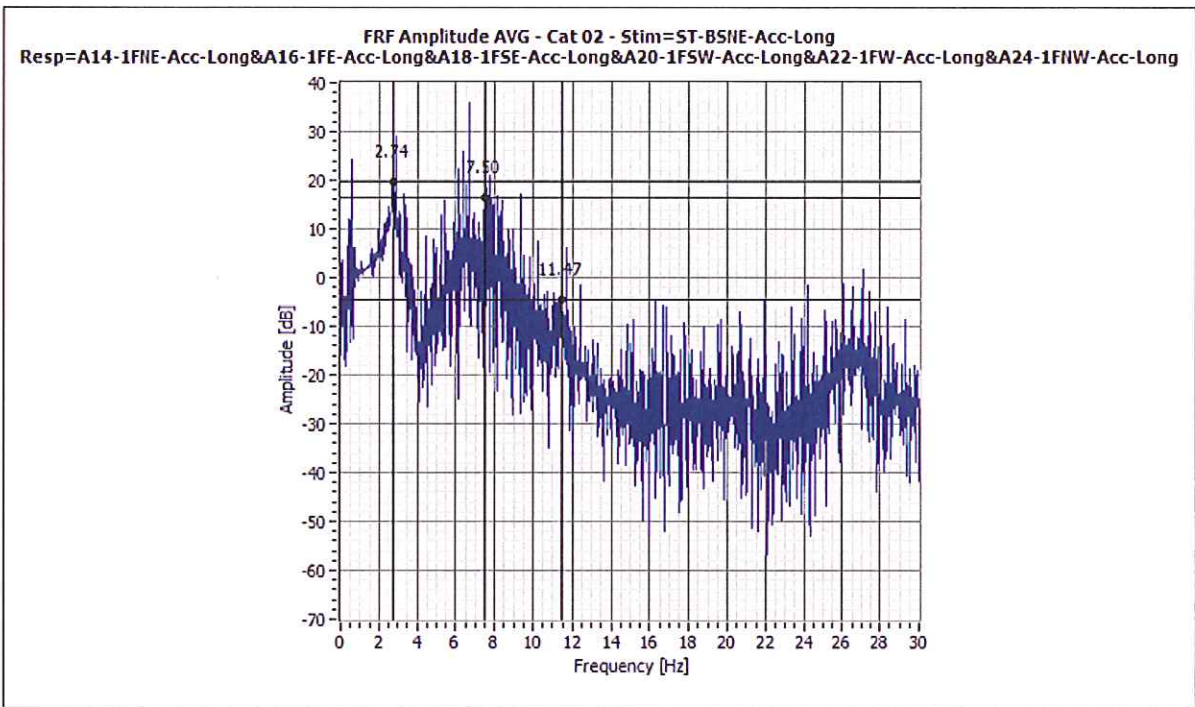


Figure G-12: FRF peak picking – Stage 02 – Longitudinal (1st Floor)

G.4 Cat 03

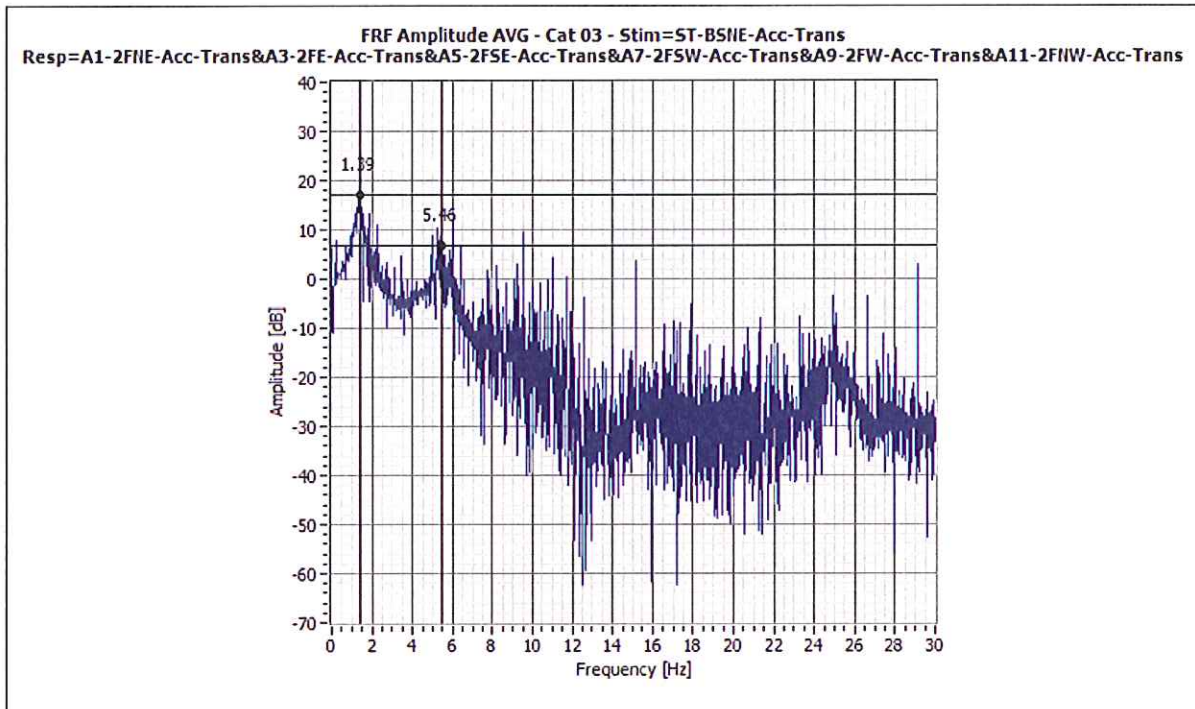


Figure G-13: FRF peak picking – Stage 03 – Transversal (2nd Floor)

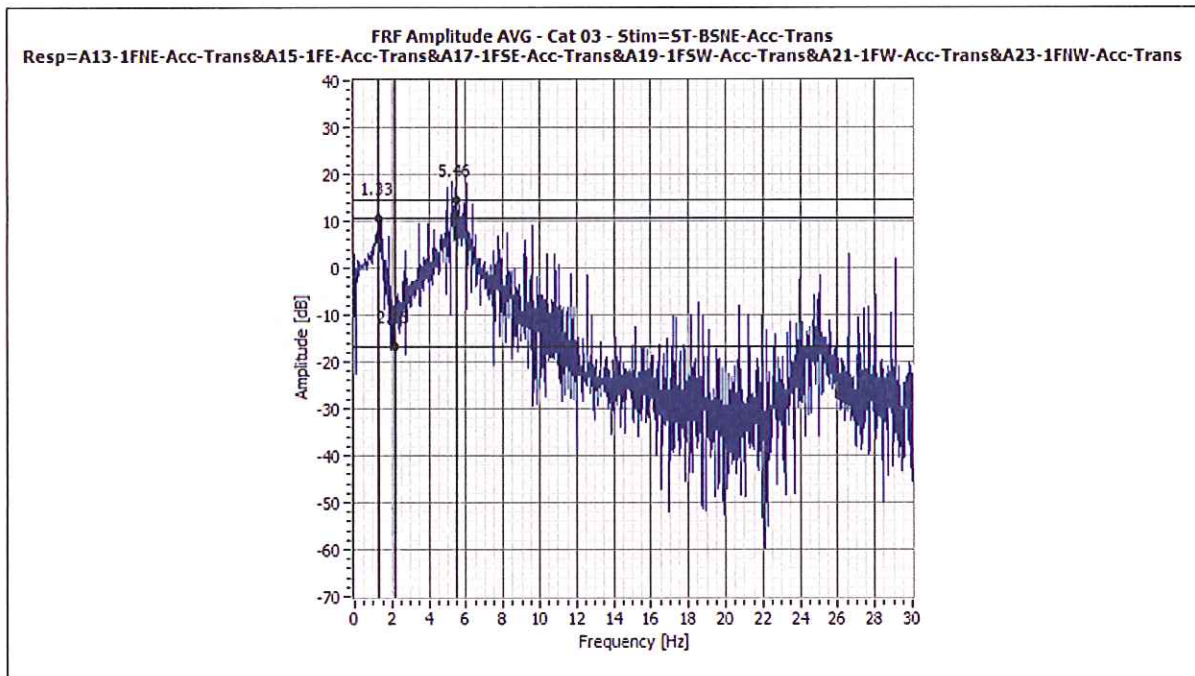


Figure G-14: FRF peak picking – Stage 03 – Transversal (1st Floor)

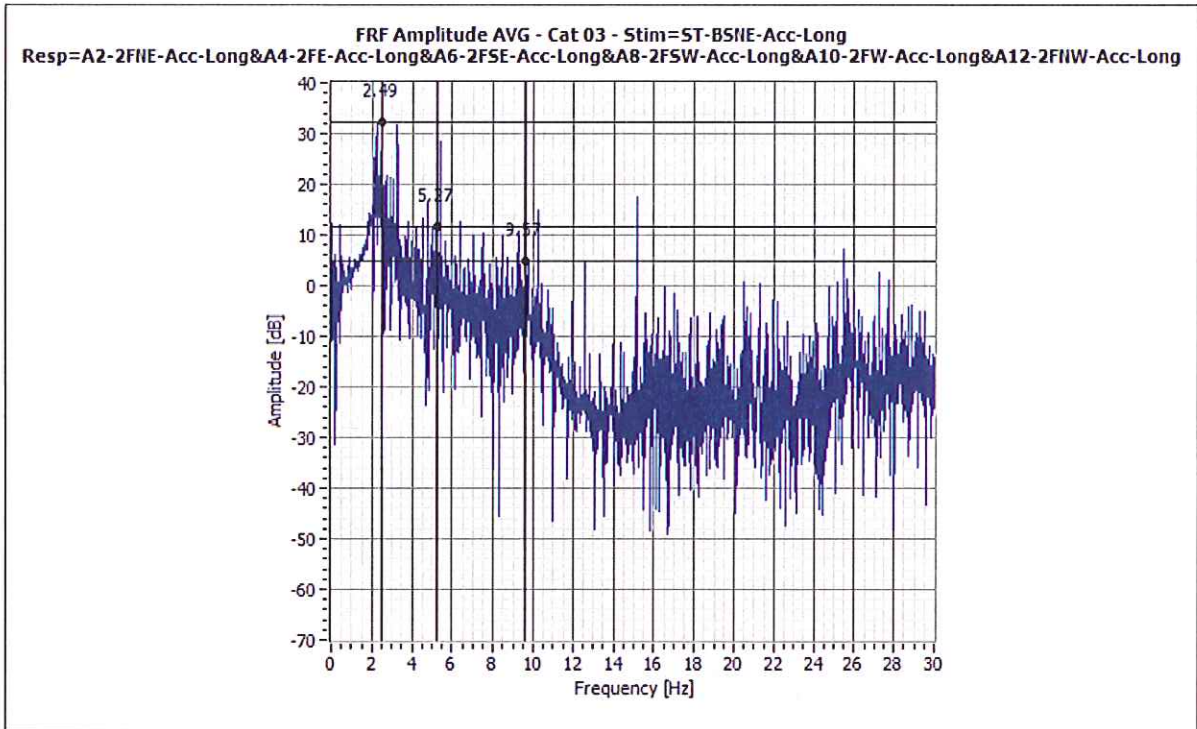


Figure G-15: FRF peak picking – Stage 03 – Longitudinal (2nd Floor)

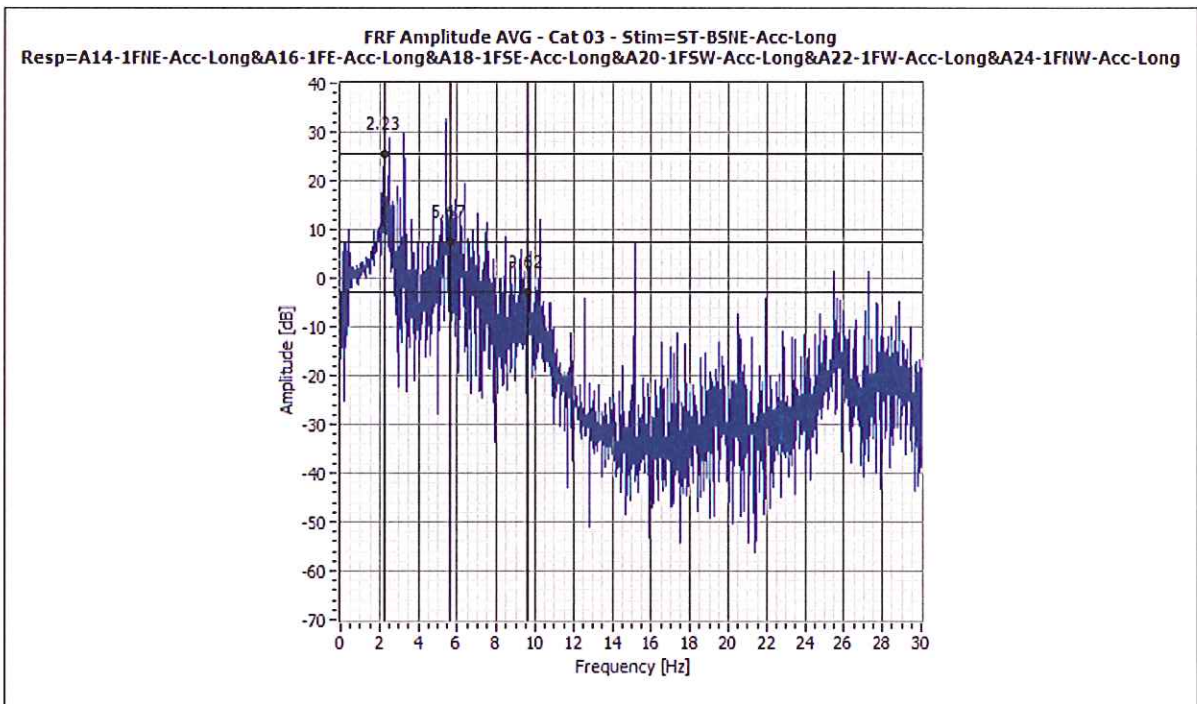


Figure G-16: FRF peak picking – Stage 03 – Longitudinal (1st Floor)

G.5 Cat 04

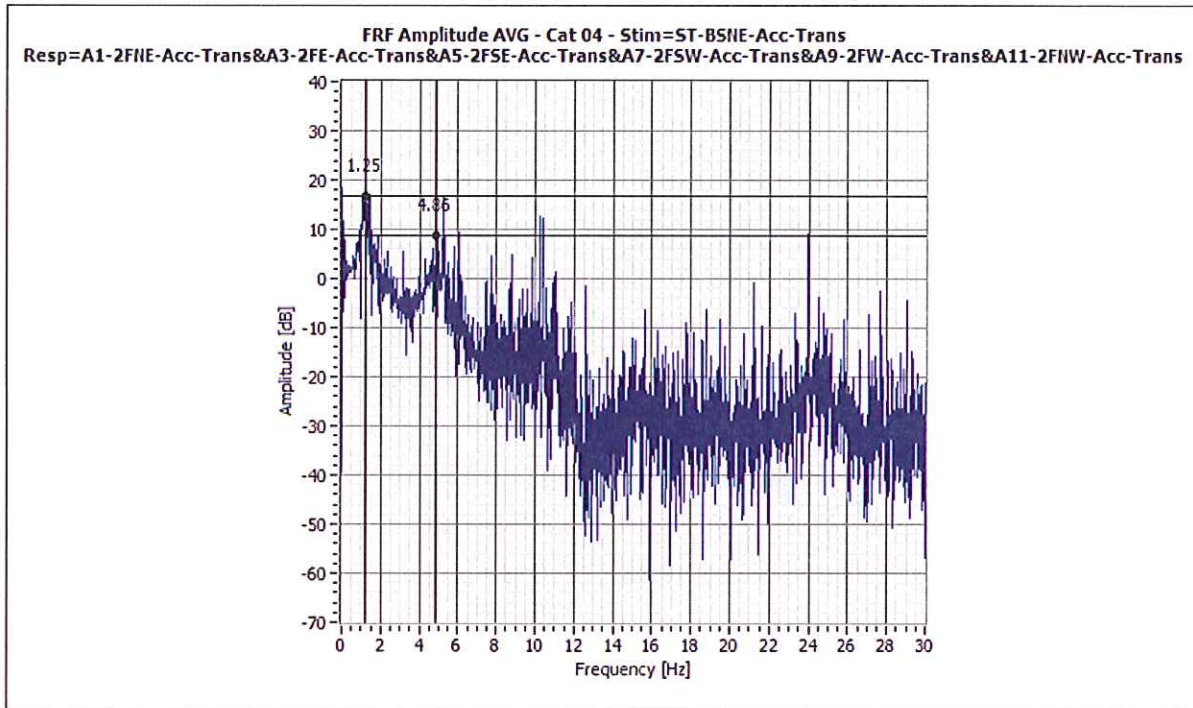


Figure G-17: FRF peak picking – Stage 04 – Transversal (2nd Floor)

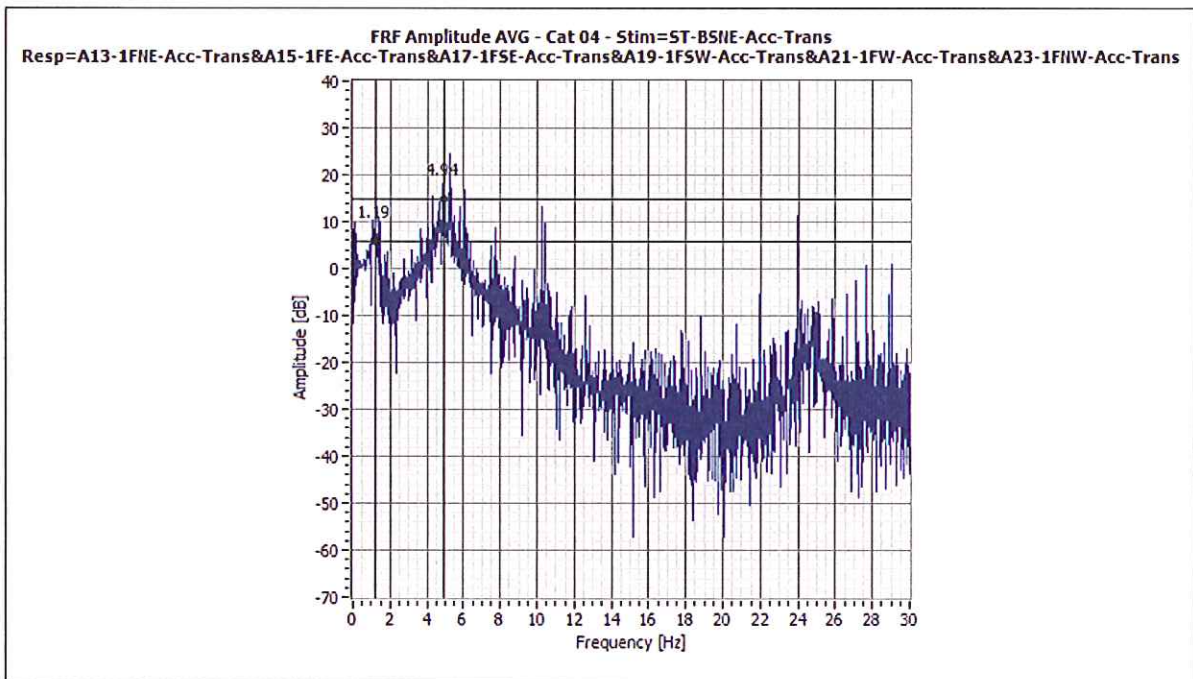


Figure G-18: FRF peak picking – Stage 04 – Transversal (1st Floor)

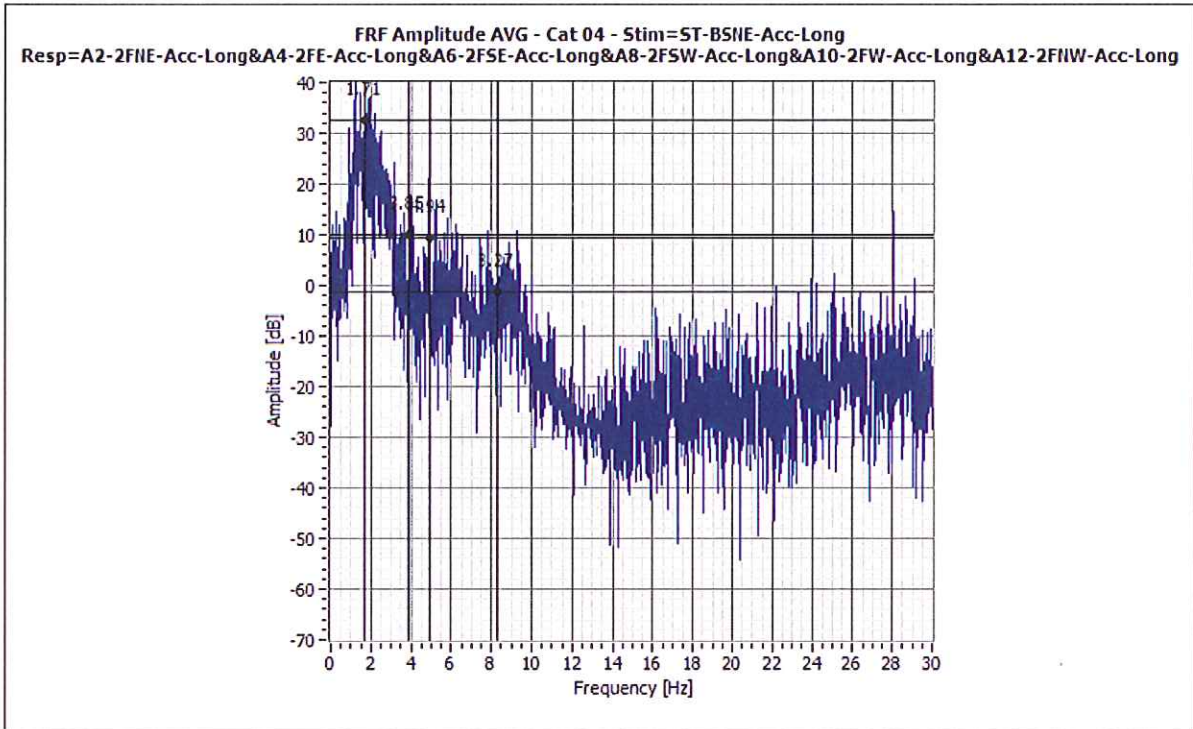


Figure G-19: FRF peak picking – Stage 04 – Longitudinal (2nd Floor)

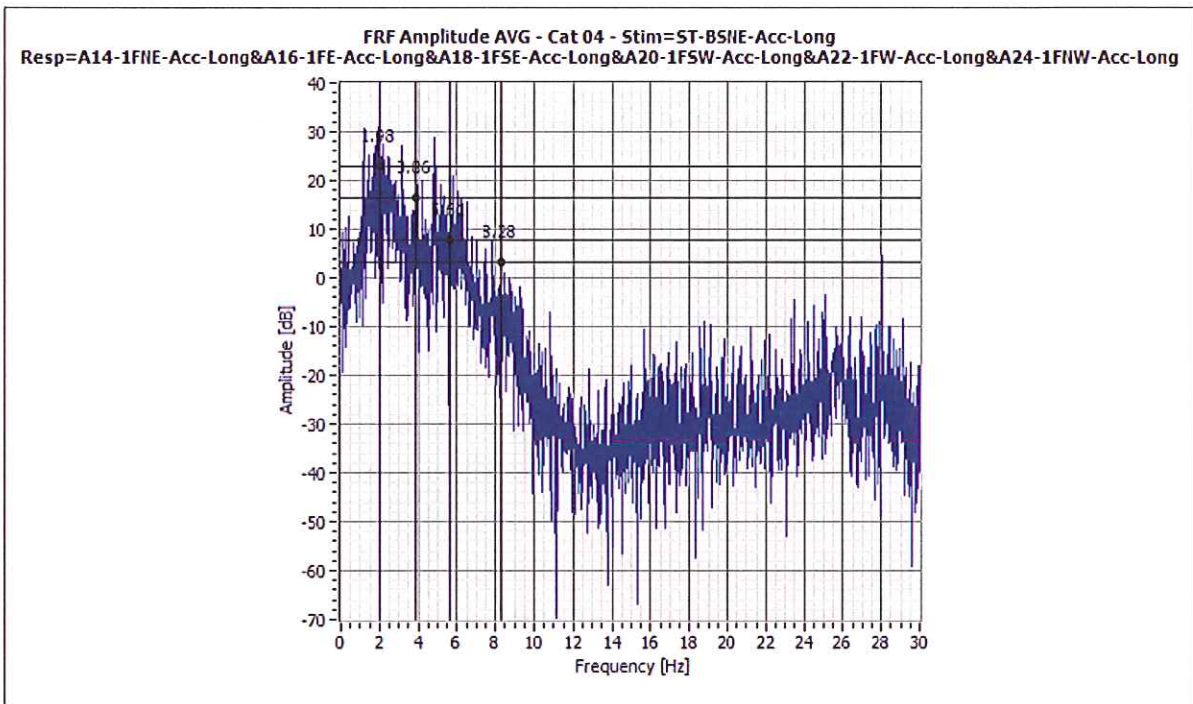
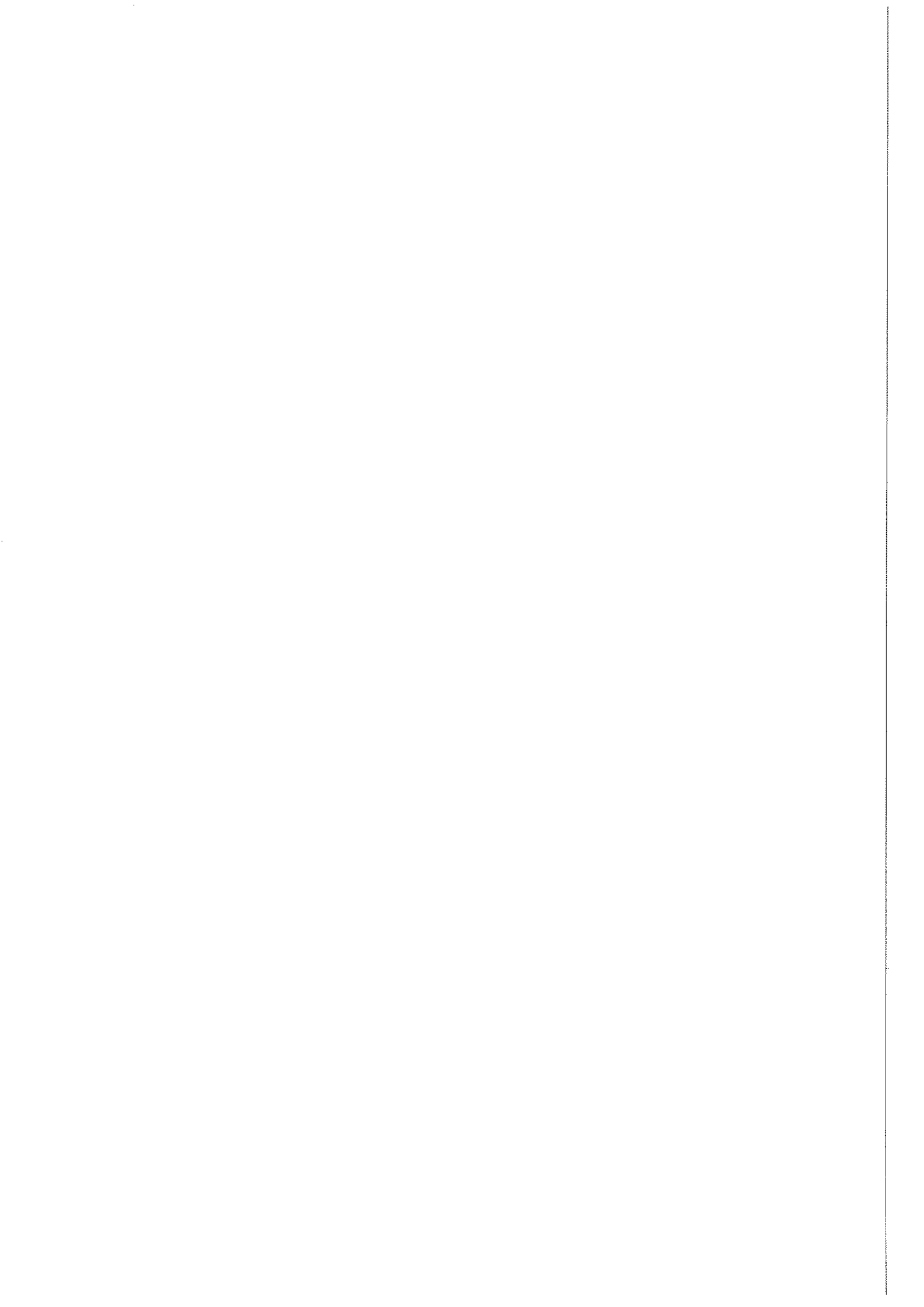


Figure G-20: FRF peak picking – Stage 04 – Longitudinal (1st Floor)

ANNEX H EARTHQUAKE SCENARIO

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H.1 Time Histories

H.1.1 Stage 00

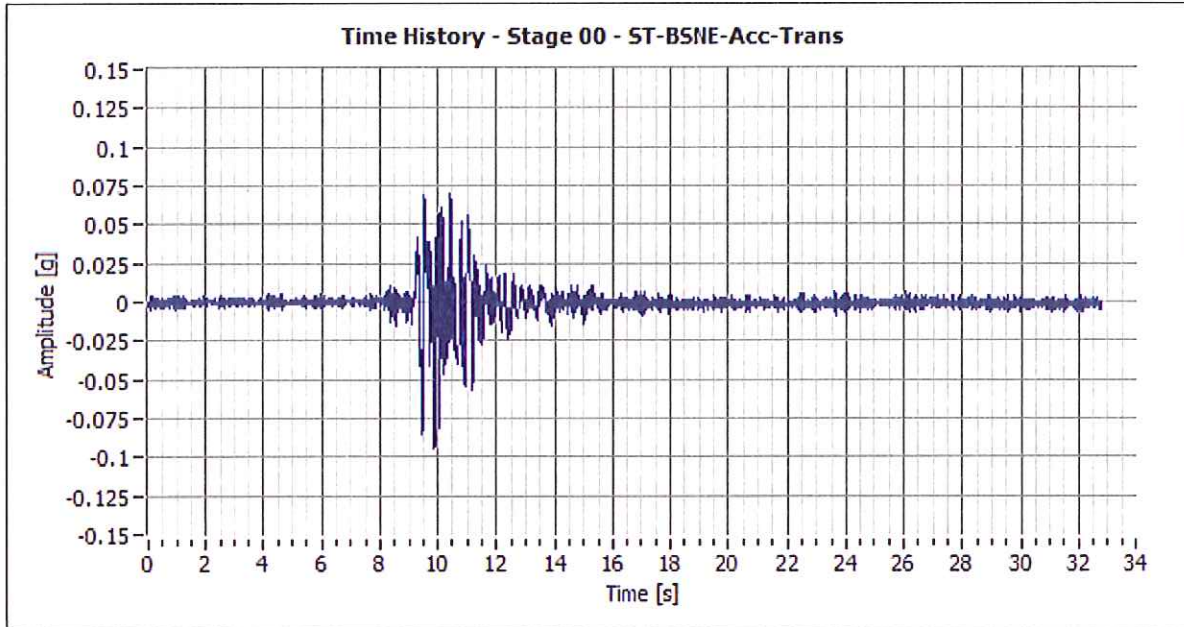


Figure H-1: Time History – Base Accelerations – Stage 00 – Transversal direction

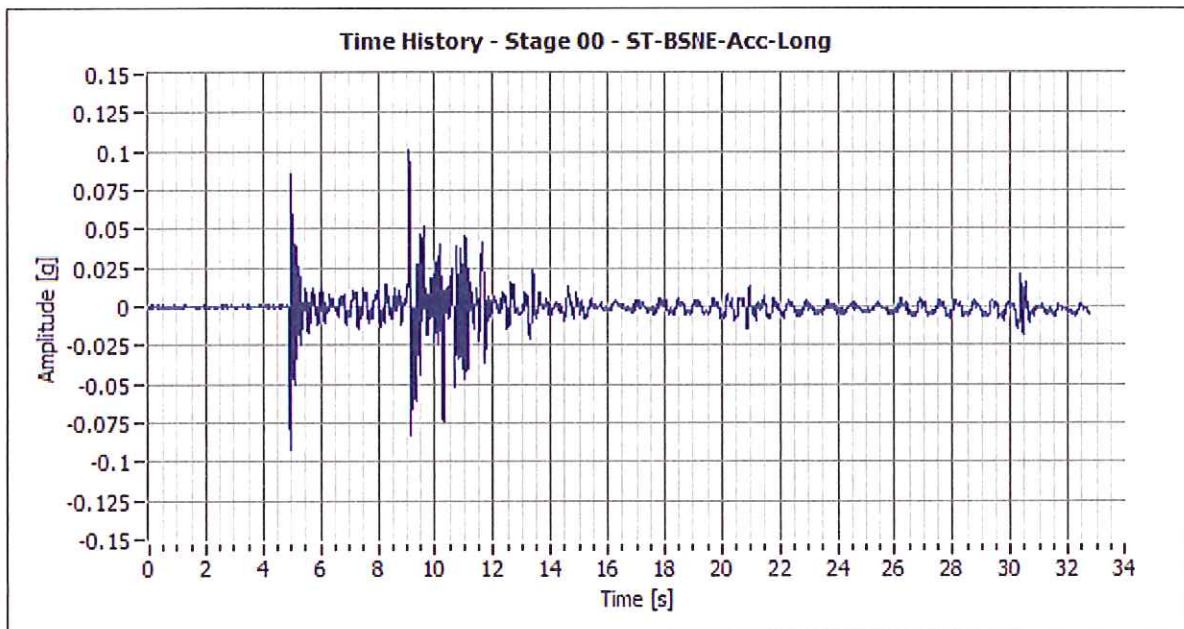


Figure H-2: Time History – Base Accelerations – Stage 00 – Longitudinal direction

H.1.2 Stage 01

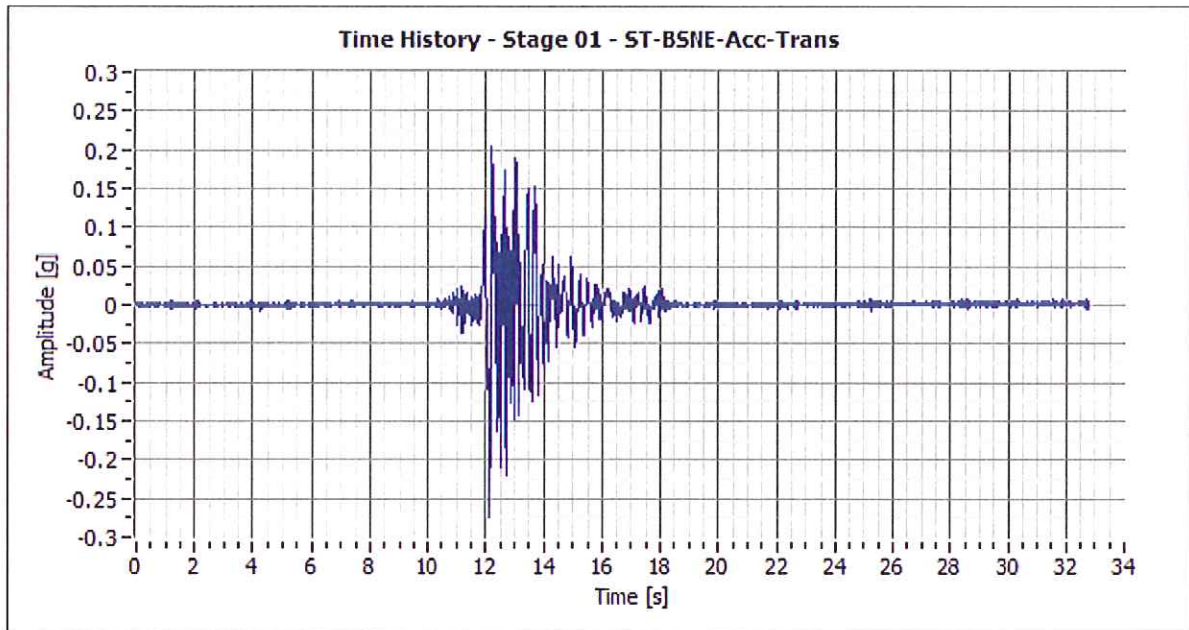


Figure H-3: Time History – Base Accelerations – Stage 01 – Transversal direction

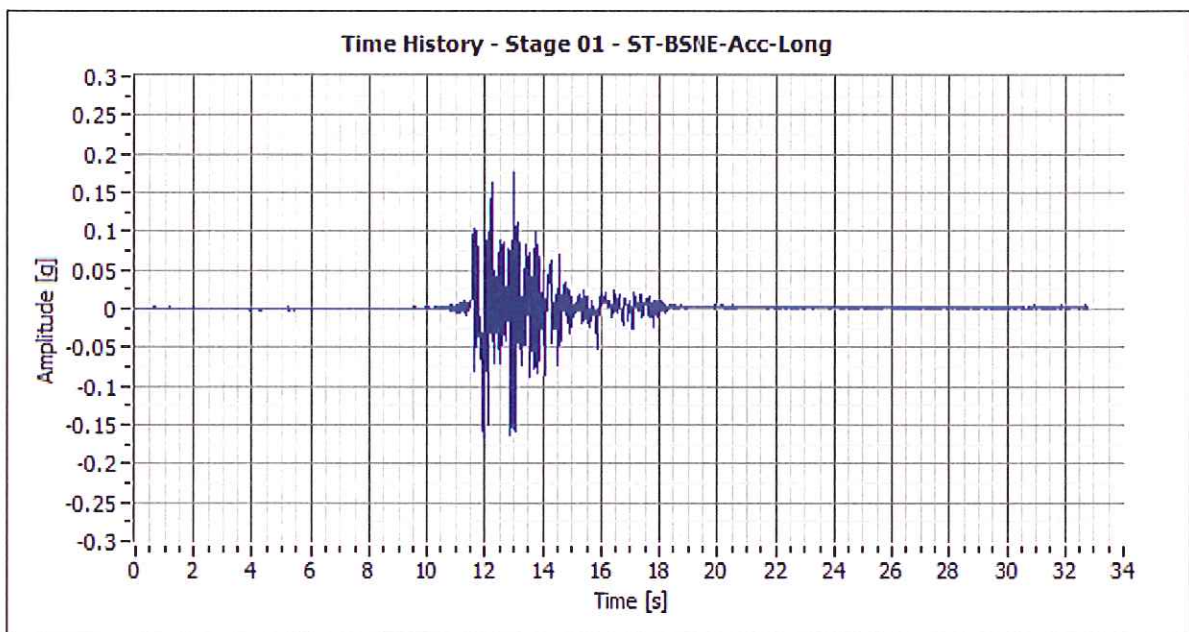


Figure H-4: Time History – Base Accelerations – Stage 01 – Longitudinal direction

H.1.3 Stage 02

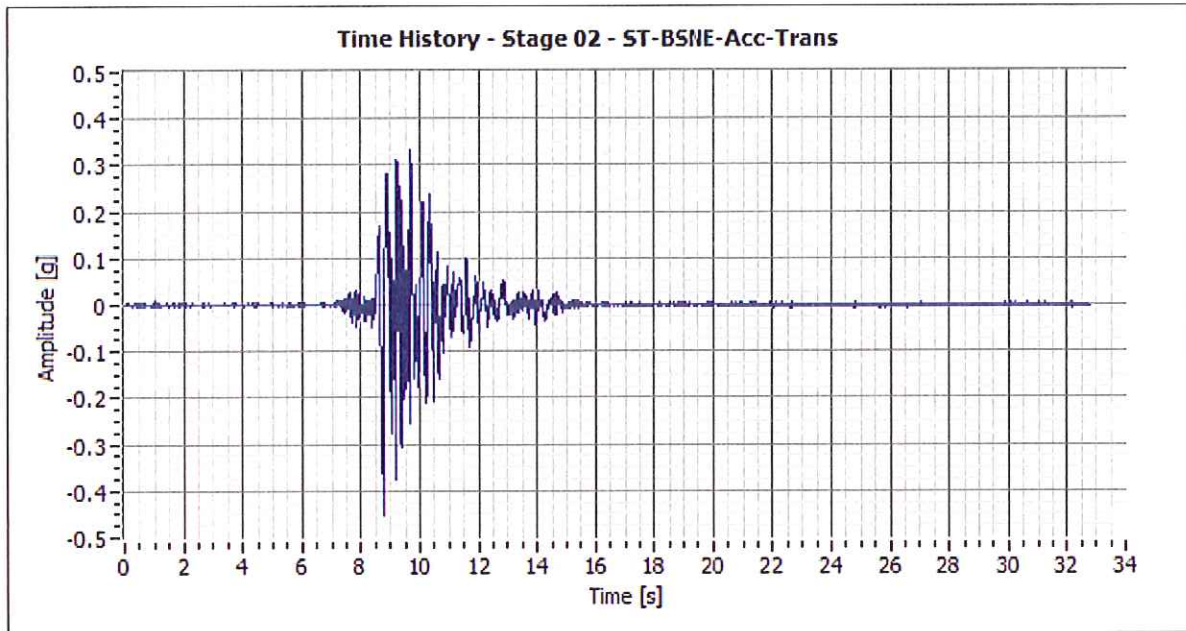


Figure H-5: Time History – Base Accelerations – Stage 02 – Transversal direction

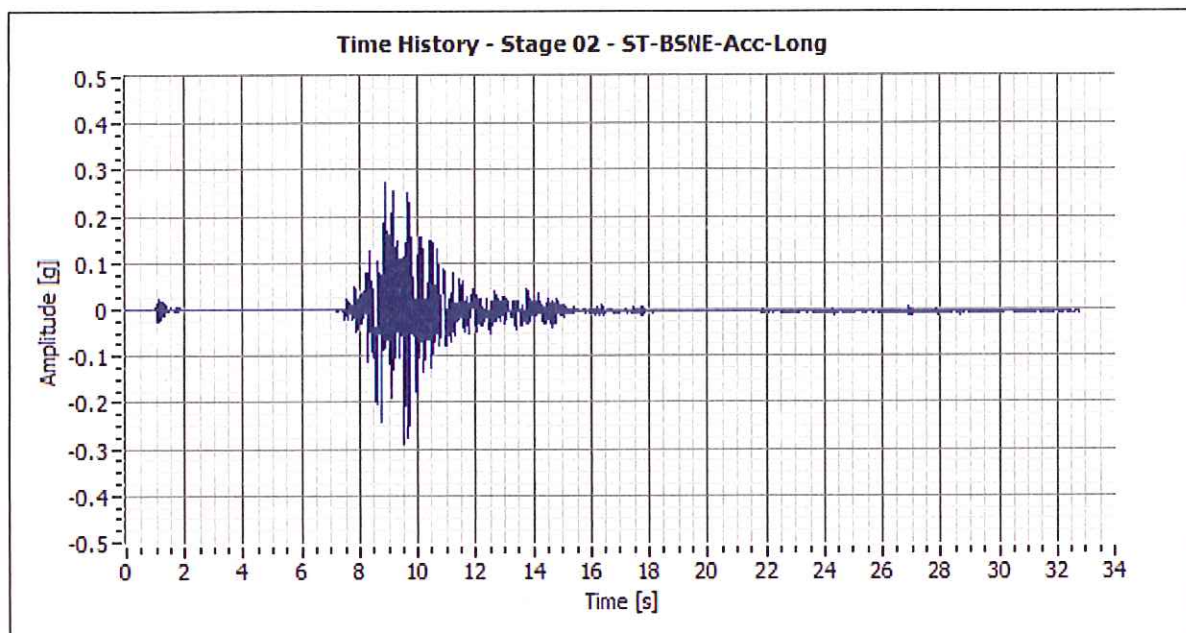


Figure H-6: Time History – Base Accelerations – Stage 02 – Longitudinal direction

H.1.4 Stage 03

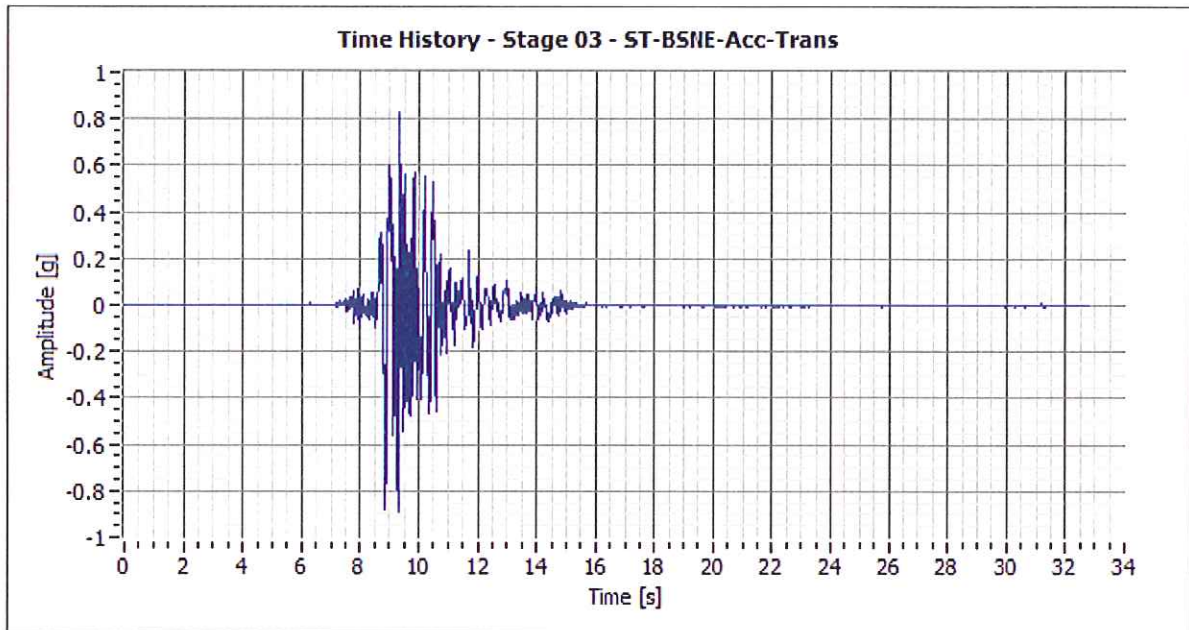


Figure H-7: Time History – Base Accelerations – Stage 03 – Transversal direction

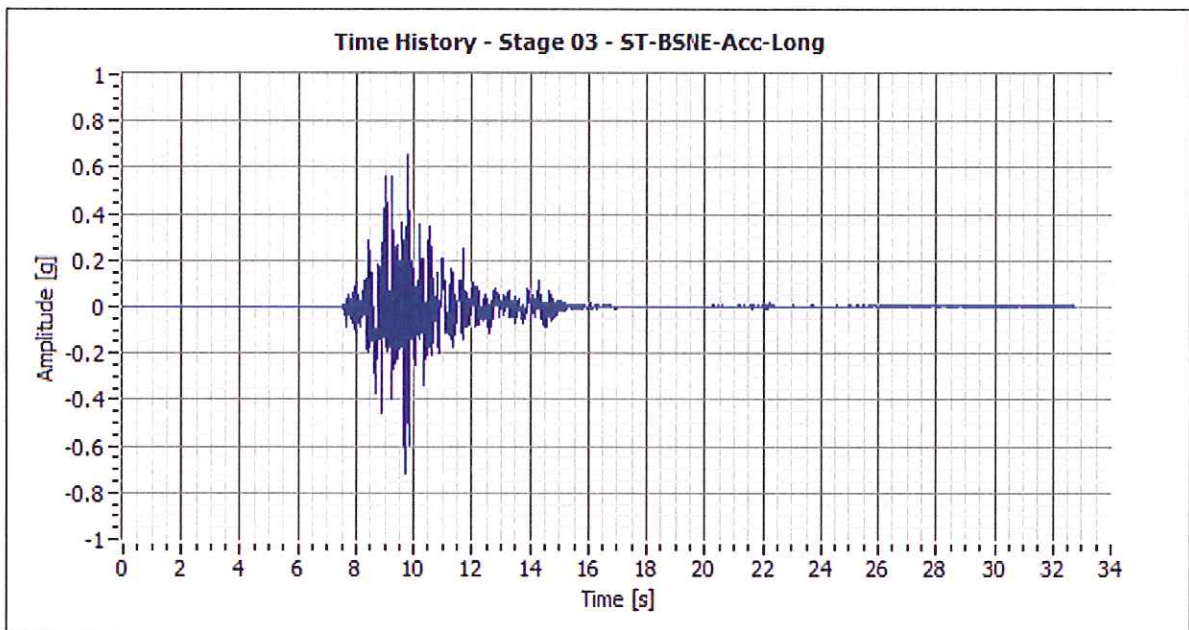


Figure H-8: Time History – Base Accelerations – Stage 03 – Longitudinal direction

H.1.5 Stage 04

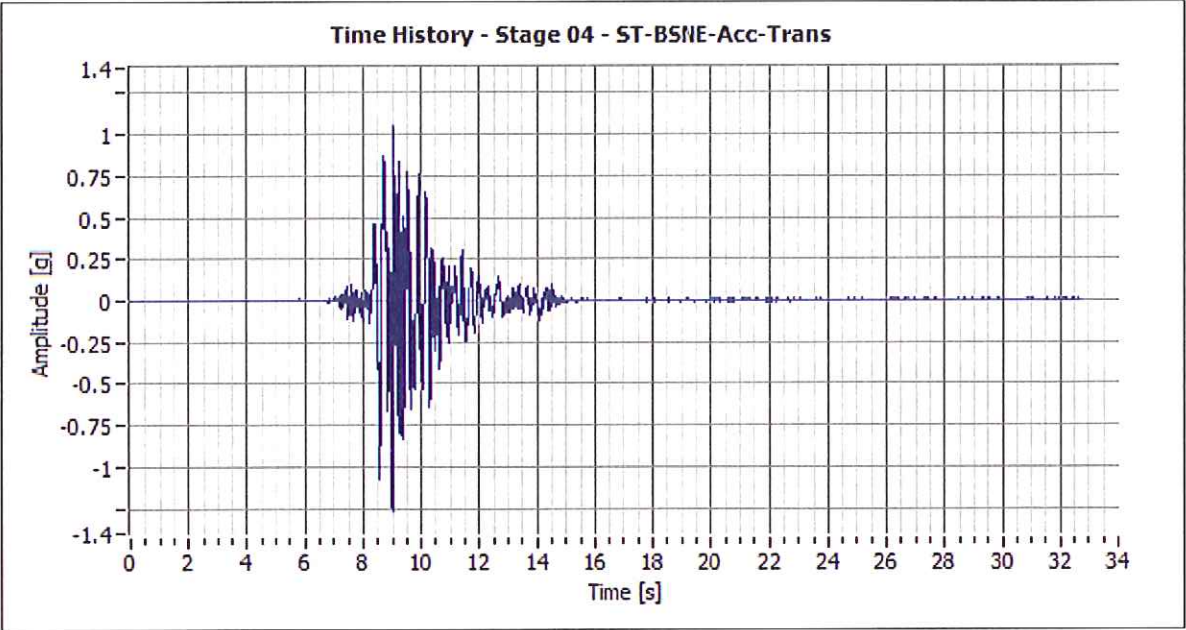


Figure H-9: Time History – Base Accelerations – Stage 04 – Transversal direction

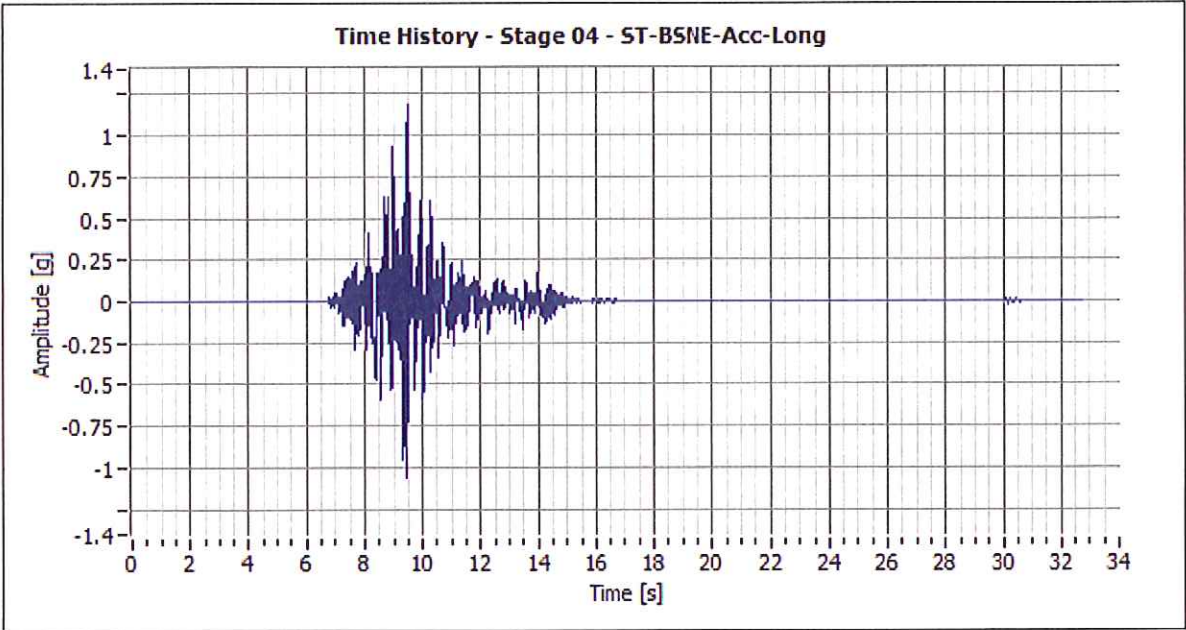


Figure H-10: Time History – Base Accelerations – Stage 04 – Longitudinal direction

H.2 Ground Motion Parameters

Ground Motion Parametes	Units	Stage 00		Stage 01		Stage 02	
		Trans.	Long.	Trans.	Long.	Trans.	Long.
Peak Ground Acceleration	g	0.095	0.101	0.275	0.176	0.453	0.289
Peak Ground Velocity	cm/s	4.593	4.322	12.893	8.305	19.604	11.750
Peak Ground Displacement	mm	3.174	3.442	8.600	8.826	13.595	14.198
Peak Velocity and Acceleration Ratio	s	0.049	0.044	0.048	0.048	0.044	0.041
Arias Intensity (Ia)	m/s	0.042	0.031	0.336	0.157	0.837	0.380
A95 Parameter	g	0.069	0.074	0.190	0.138	0.311	0.220
t05 (5% of Ia)	s	2.00	1.69	2.07	1.72	2.12	1.86
t95 (95% of Ia)	s	4.86	5.32	4.32	4.61	4.32	4.86
Significant Duration (t95-t05)	s	2.86	3.63	2.25	2.89	2.20	2.99
Characteristic Intensity	-	0.0069	0.0056	0.0331	0.0187	0.0656	0.0363
Predominant Period	s	0.082	0.082	0.082	0.091	0.082	0.091
Mean Period	s	0.215	0.194	0.232	0.172	0.231	0.167
Sustained Max Acceleraion	g	0.081	0.074	0.210	0.164	0.333	0.274
Sustained Max Velocity	cm/s	2.526	2.933	8.160	6.030	12.500	9.091
Acceleration Spectrum Intensity*	g	0.047	0.043	0.137	0.082	0.219	0.111
Velocity Spec. Int. (a.k. Housner Int.)*	cm/s	6.618	6.609	18.510	15.467	28.615	22.594

* adapted to the model time scale

Figure H-11: Ground motion parameters

Ground Motion Parametes	Units	Stage 03		Stage 04	
		Trans.	Long.	Trans.	Long.
Peak Ground Acceleration	g	0.892	0.717	1.262	1.178
Peak Ground Velocity	cm/s	34.703	22.106	48.431	30.350
Peak Ground Displacement	mm	28.077	28.756	41.560	42.819
Peak Velocity and Acceleration Ratio	s	0.040	0.031	0.039	0.026
Arias Intensity (Ia)	m/s	3.566	1.753	7.167	4.008
A95 Parameter	g	0.672	0.518	0.866	0.828
t05 (5% of Ia)	s	2.12	1.88	1.81	1.47
t95 (95% of Ia)	s	4.32	4.93	4.21	4.64
Significant Duration (t95-t05)	s	2.20	3.05	2.40	3.17
Characteristic Intensity	-	0.1945	0.1142	0.3284	0.2124
Predominant Period	s	0.082	0.091	0.082	0.091
Mean Period	s	0.223	0.161	0.228	0.161
Sustained Max Acceleraion	g	0.829	0.606	1.054	0.955
Sustained Max Velocity	cm/s	23.261	18.230	32.454	24.160
Acceleration Spectrum Intensity*	g	0.429	0.250	0.582	0.370
Velocity Spec. Int. (a.k. Housner Int.)*	cm/s	55.108	46.039	80.260	68.492

* adapted to the model time scale

Figure H-12: Ground motion parameters (cont.)

Where:

- Peak ground acceleration (PGA):

$$PGA = \max |a(t)|. \quad (H.1)$$

- Peak ground velocity (PGV):

$$PGV = \max |v(t)| \quad (H.2)$$

- Peak ground displacement (PGD):

$$PGD = \max |d(t)| \quad (H.3)$$

- Peak velocity and acceleration ratio (v_{\max}/a_{\max})

$$v_{\max}/a_{\max} = \frac{\max |v(t)|}{\max |a(t)|} \quad (H.4)$$

- Arias Intensity (I_a)

$$I_a = \frac{\pi}{2g} \int [a(t)]^2 dt \quad (H.5)$$

- A95 parameter (A95)

Is the maximum value of acceleration that corresponds to 95% of the Arias intensity value.

- t05 parameter (t05):

Is the time where 5% of the Arias intensity is reached.

- t95 parameter (t95):

Is the time where 95% of the Arias intensity is reached.

- Significant duration (SD):

$$SD = t_{95} - t_{05} \quad (H.6)$$

- Characteristic intensity (I_c):

$$I_c = (a_{rms})^2 \cdot \sqrt{t_{tot}}, \quad a_{rms} = \sqrt{\frac{1}{t_{tot}} \int [a(t)]^2 dt}. \quad (H.7)$$

- Predominant period (T_p)

Is the period where is reached the maximum spectral accelerations in an acceleration response spectra ($\xi = 5\%$)

- Mean period (T_m):

$$T_m = \frac{\sum C_i^2 / f_i}{\sum C_i^2}, \quad (\text{H.8})$$

Where: C_i are the Fourier amplitudes at f_i frequencies between 0.25 and 20 Hz.

- Sustained maximum acceleration (SMA)

Is the third maximum acceleration measured.

- Sustained maximum Velocity (SMV)

Is the third maximum velocity measured.

- Acceleration Spectrum Intensity (ASI)

$$ASI = \int_{0.1}^{0.5} S_a(\xi = 5\%, T) dT. \quad (\text{H.9})$$

- Velocity Spectrum Intensity, also known as Housner Intensity (VSI)

$$VSI = \int_{0.1}^{2.5} S_v(\xi = 5\%, T) dT. \quad (\text{H.10})$$

H.3 Response Spectra

H.3.1 Stage 00

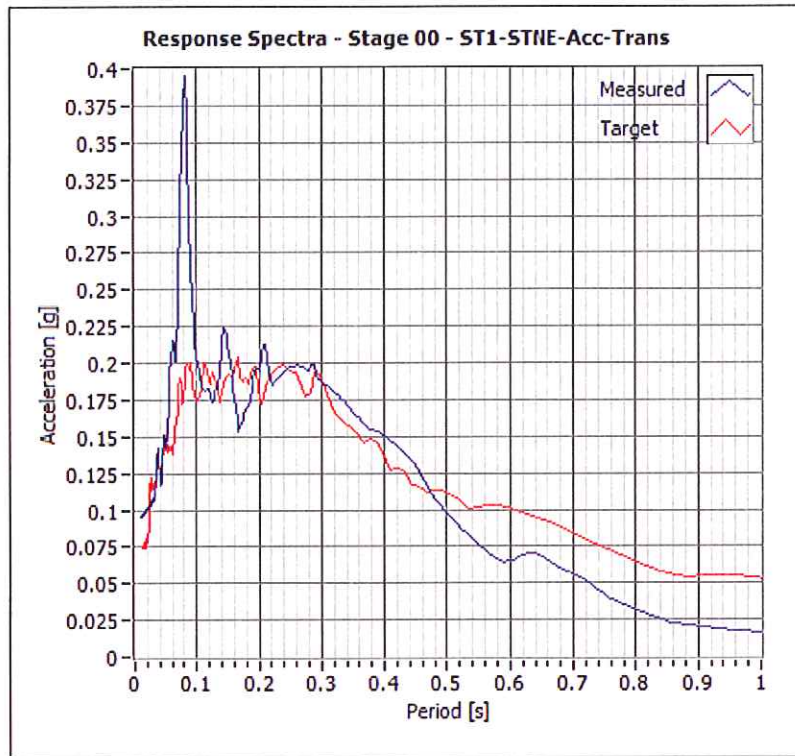


Figure H-13: Response Spectra – Base Accelerations – Stage 00 – Transversal direction

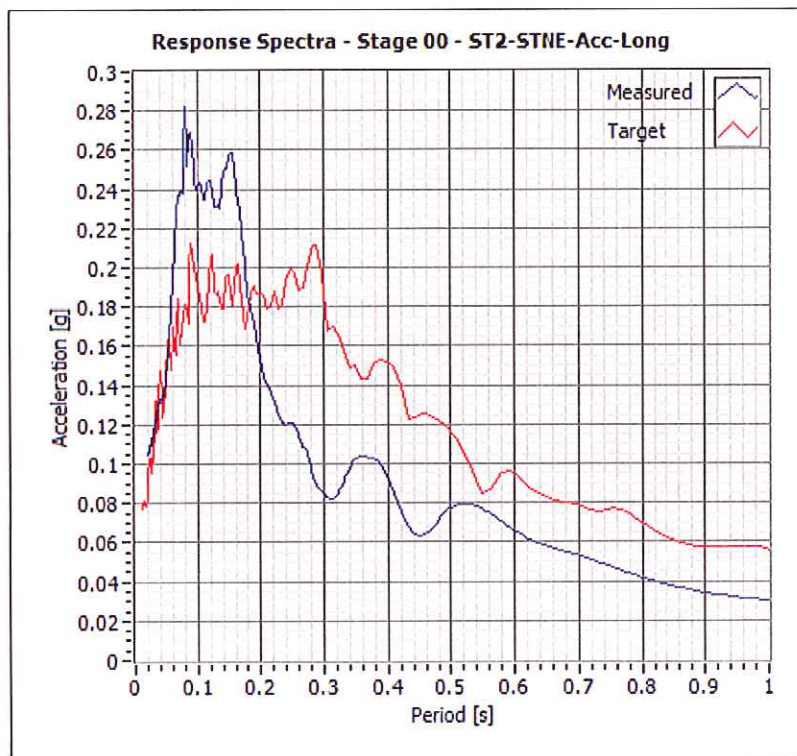


Figure H-14: Response Spectra – Base Accelerations – Stage 00 – Longitudinal direction

H.3.2 Stage 01

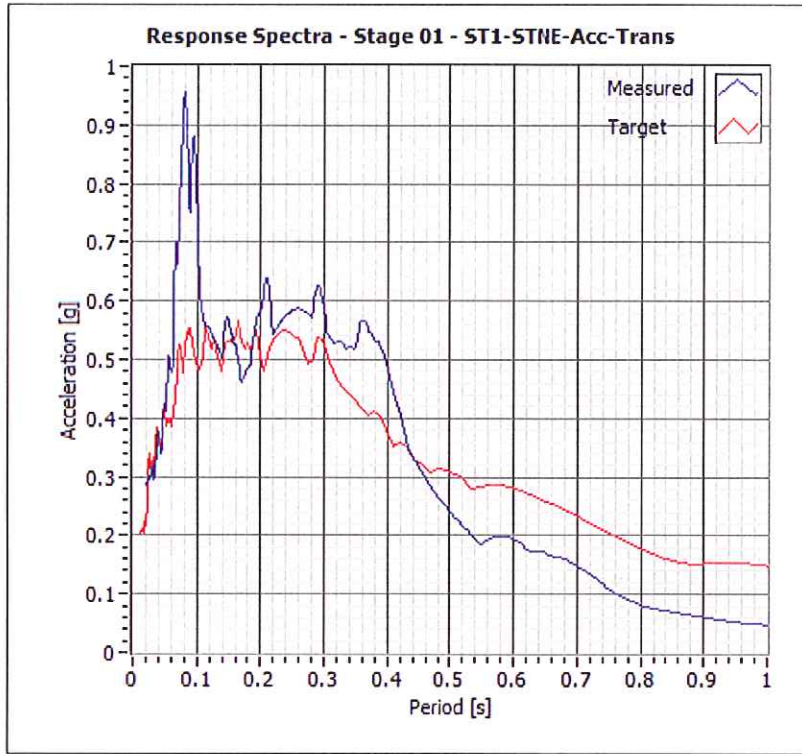


Figure H-15: Response Spectra – Base Accelerations – Stage 01 – Transversal direction

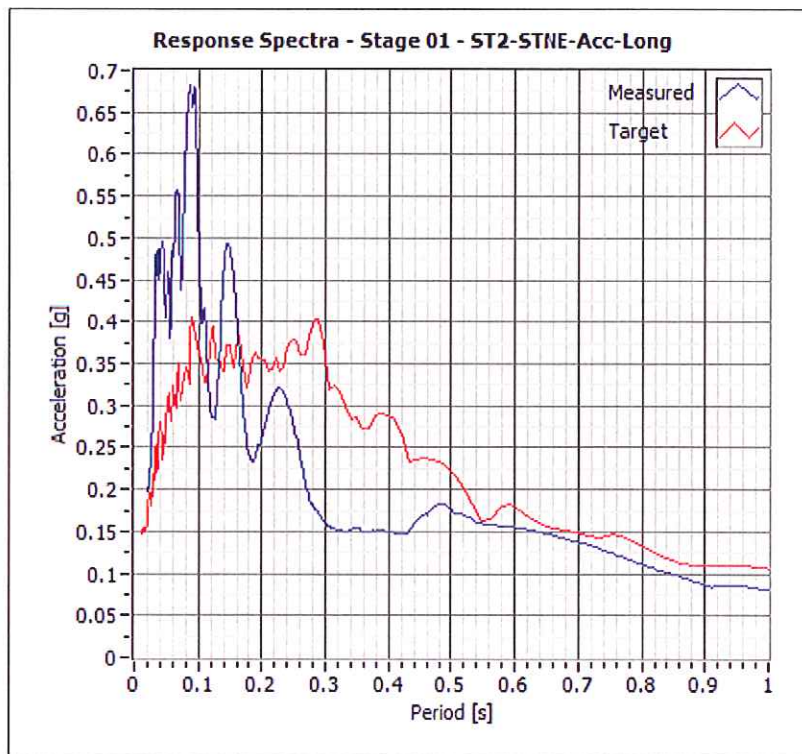


Figure H-16: Response Spectra – Base Accelerations – Stage 01 – Longitudinal direction

H.3.3 Stage 02

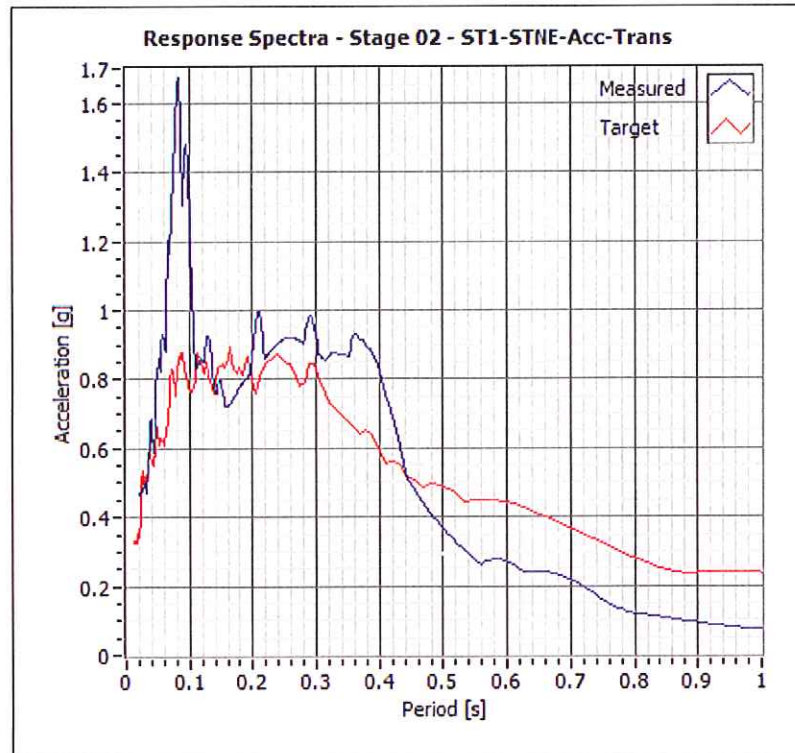


Figure H-17: Response Spectra – Base Accelerations – Stage 02 – Transversal direction

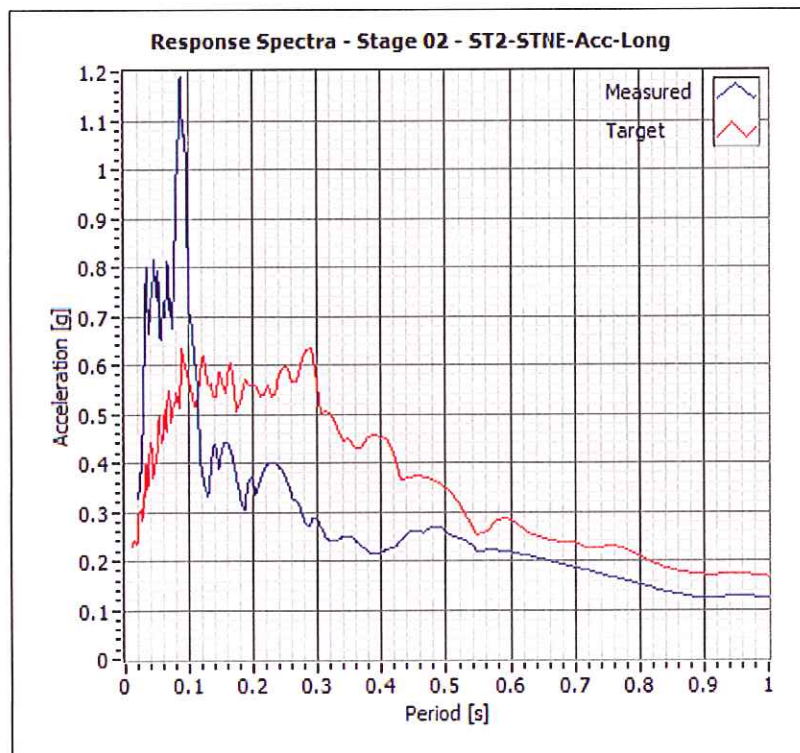


Figure H-18: Response Spectra – Base Accelerations – Stage 02 – Longitudinal direction

H.3.4 Stage 03

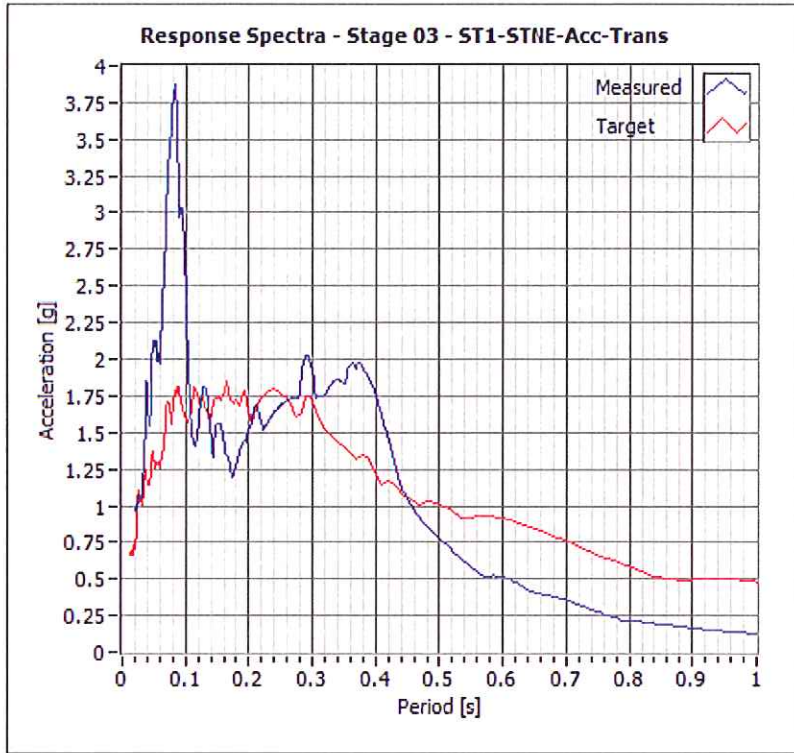


Figure H-19: Response Spectra – Base Accelerations – Stage 03 – Transversal direction

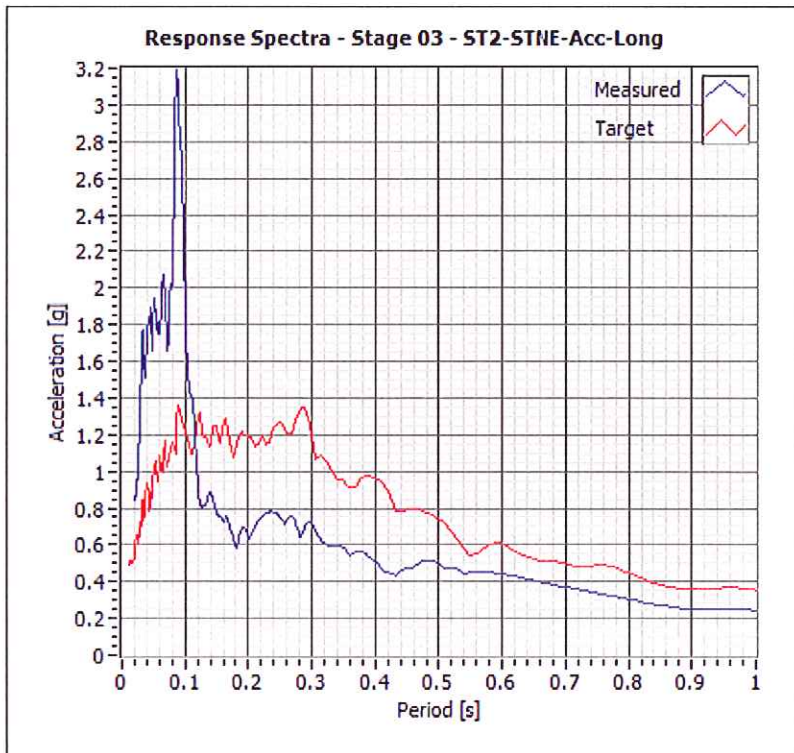


Figure H-20: Response Spectra – Base Accelerations – Stage 03 – Longitudinal direction

H.3.5 Stage 04

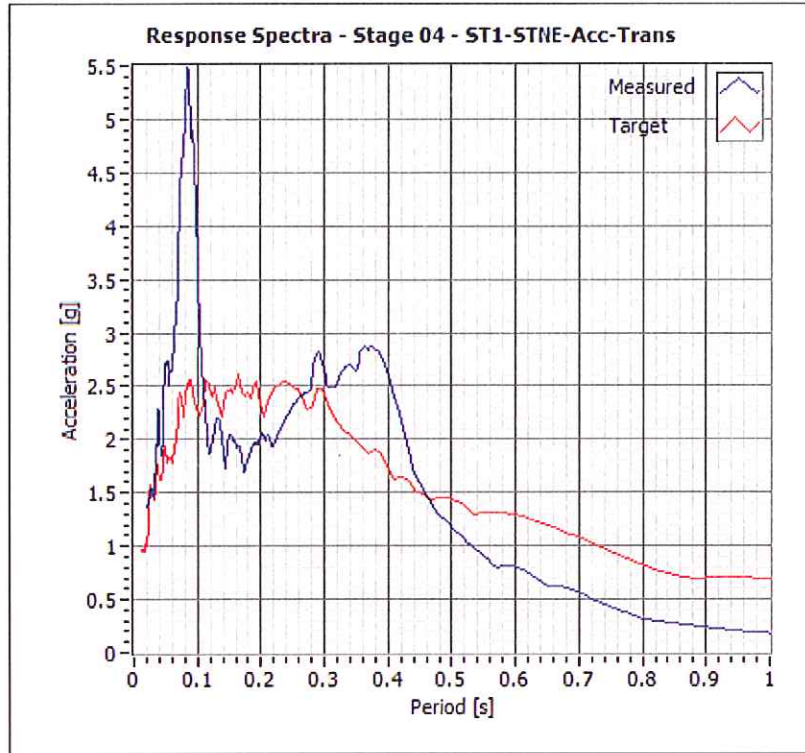


Figure H-21: Response Spectra – Base Accelerations – Stage 04 – Transversal direction

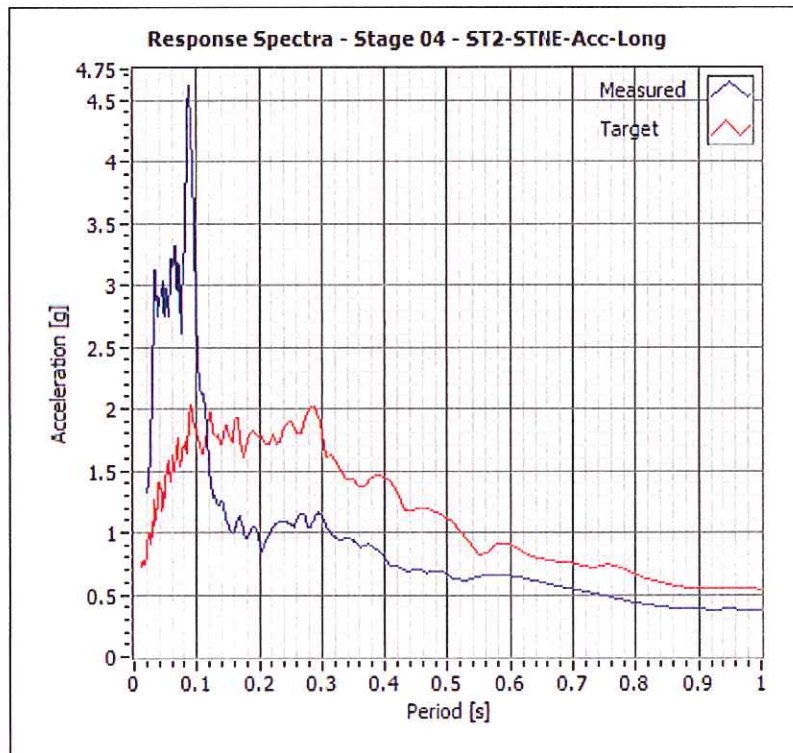
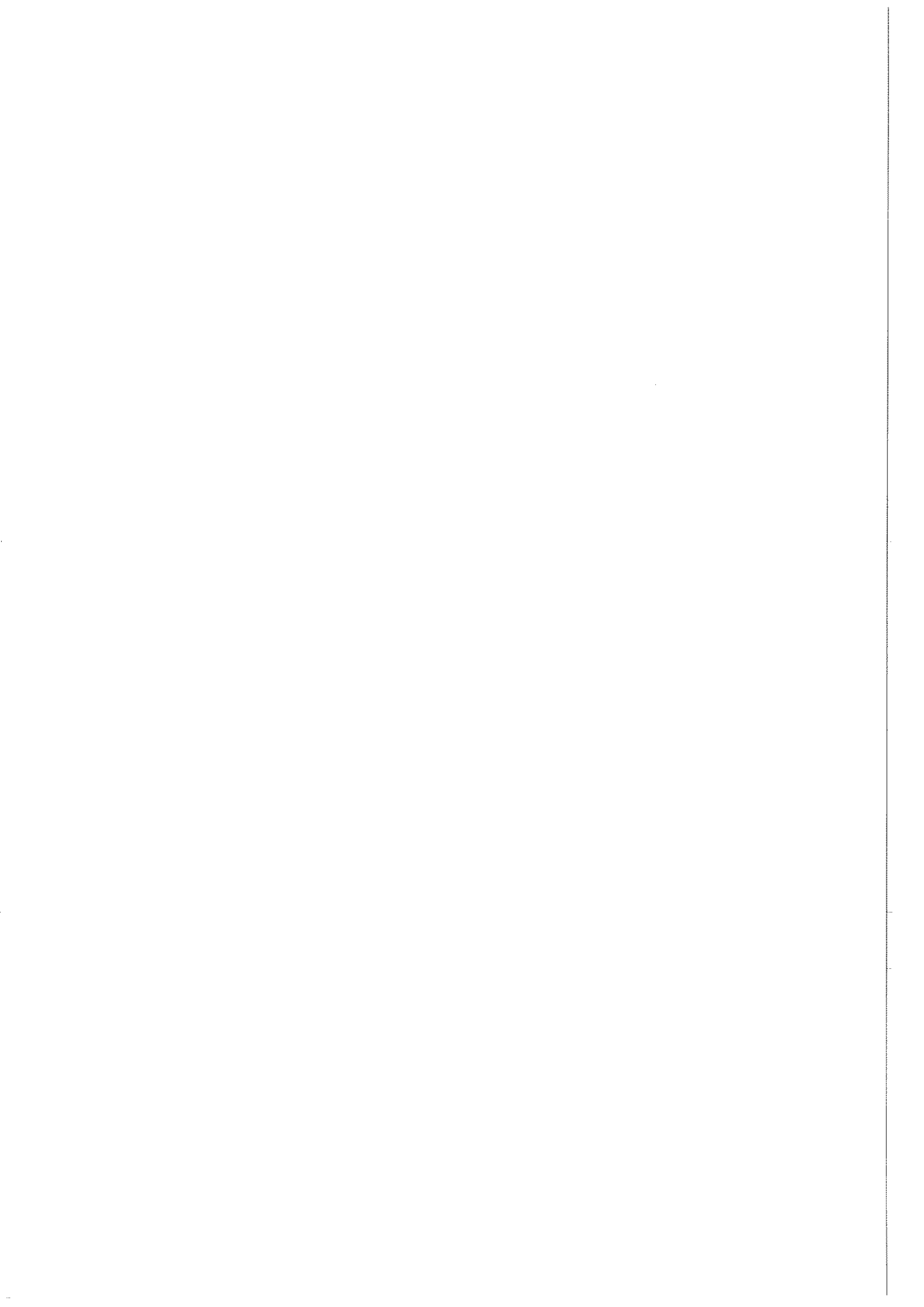


Figure H-22: Response Spectra – Base Accelerations – Stage 04 – Longitudinal direction



ANNEX I FLOOR DIAPHRAGM

In this annex is presented the time histories of the diagonals RD1, RD2, RD3, RD4 deformations.

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Figure I-15: Time History – RD2 – Diagonal deformation Accelerations – Stage 03.....I-6

Figure I-16: Time History – RD3 – Diagonal deformation Accelerations – Stage 03.....I-6

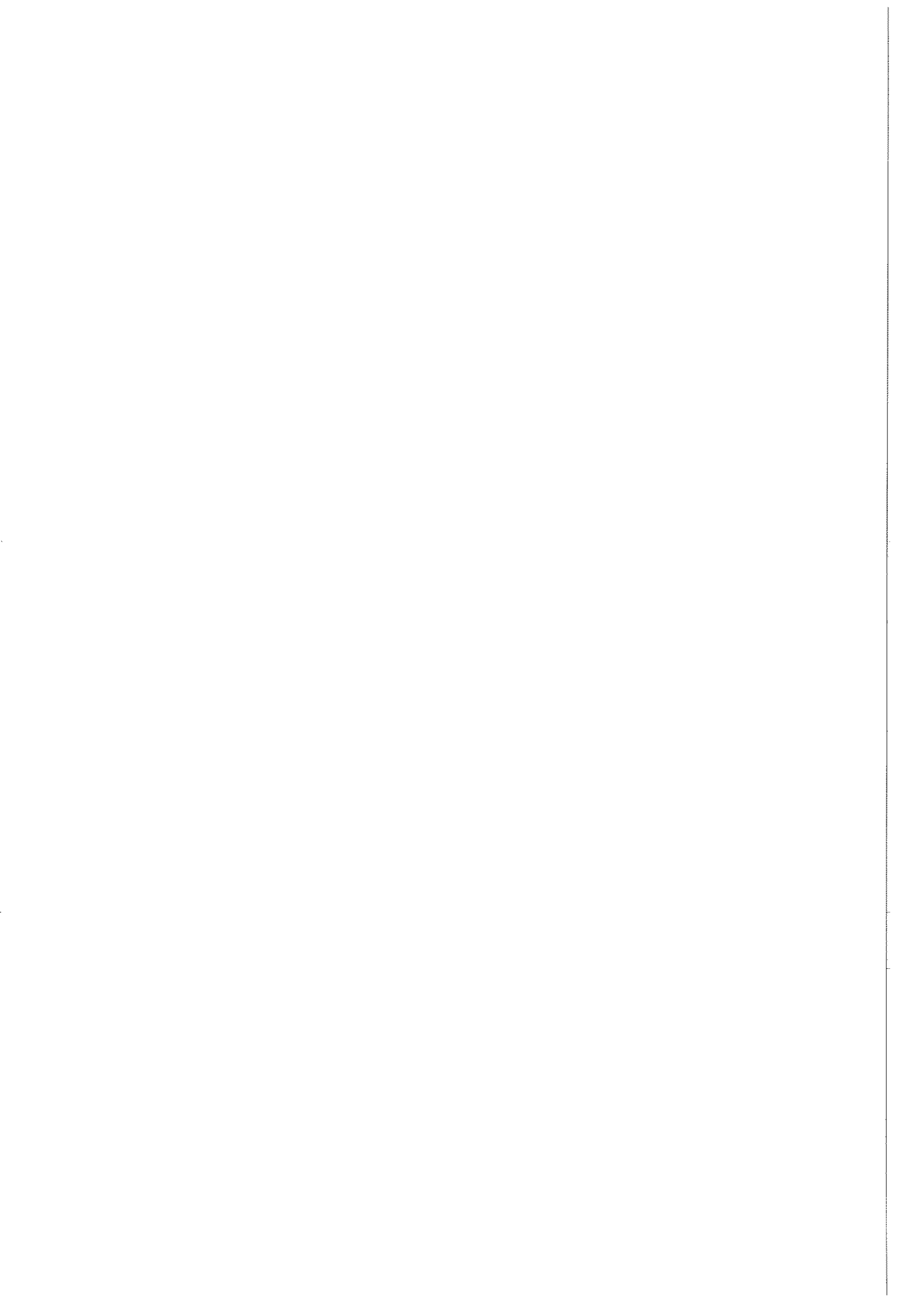
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Figure I-20: Time History – RD3 – Diagonal deformation Accelerations – Stage 04.....I-7

Figure I-21: Time History – RD4 – Diagonal deformation Accelerations – Stage 04.....I-7



I.1 Math channels

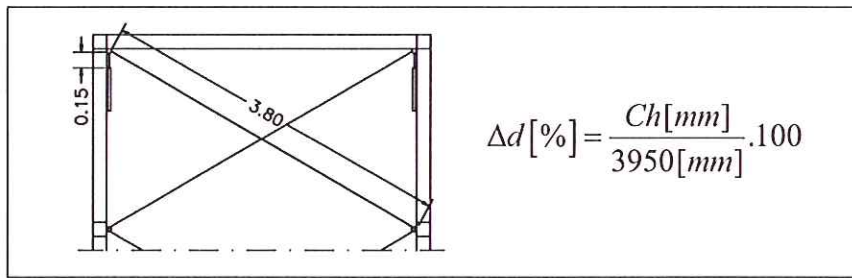


Figure I-1: Math channels calculation scheme

I.2 Stage 00

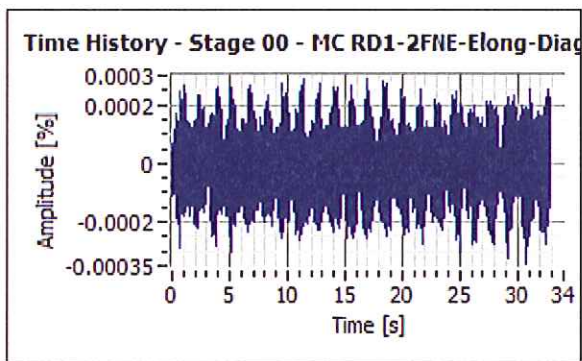


Figure I-2: Time History – RD1 – Diagonal deformation – Stage 00

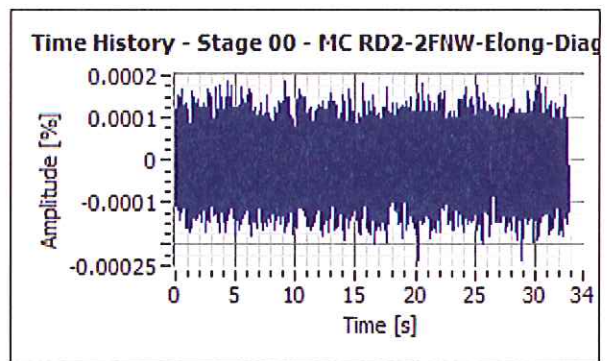


Figure I-3: Time History – RD2 – Diagonal deformation – Stage 00

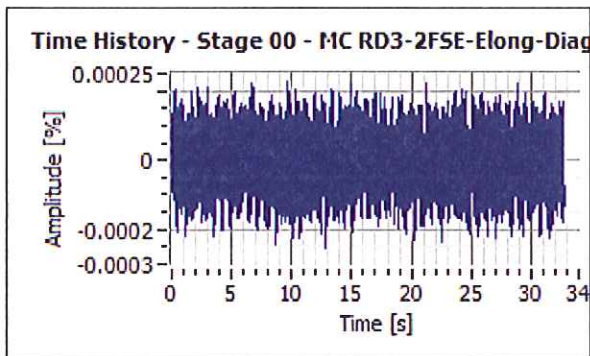


Figure I-4: Time History – RD3 – Diagonal deformation – Stage 00

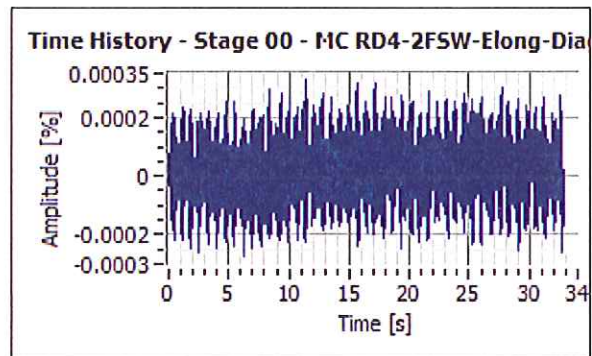


Figure I-5: Time History – RD4 – Diagonal deformation – Stage 00

I.3 Stage 01

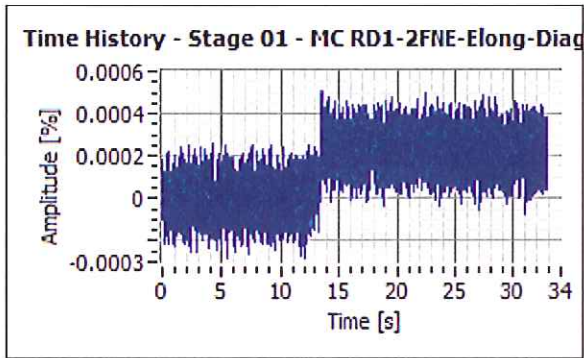


Figure I-6: Time History – RD1 – Diagonal deformation – Stage 01

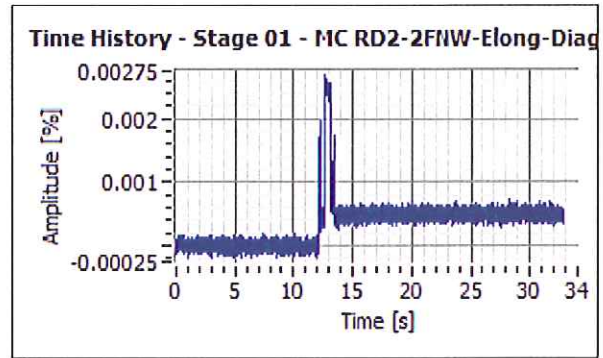


Figure I-7: Time History – RD2 – Diagonal deformation – Stage 01

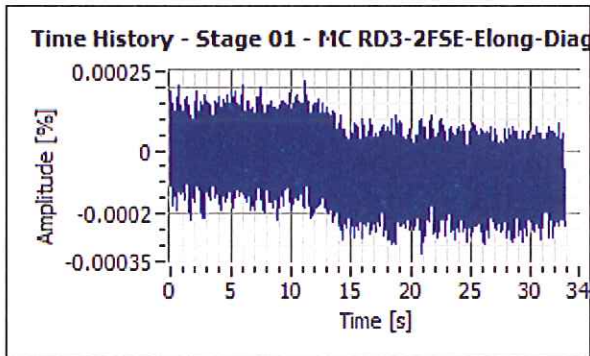


Figure I-8: Time History – RD3 – Diagonal deformation – Stage 01

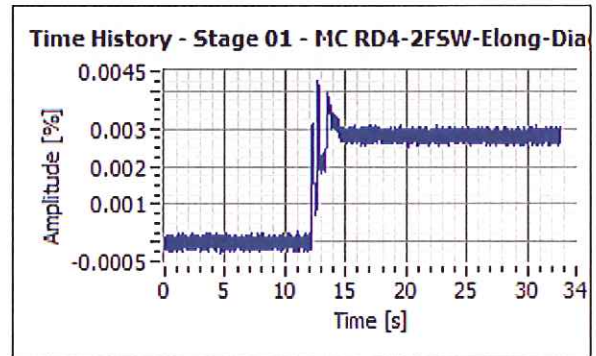


Figure I-9: Time History – RD4 – Diagonal deformation – Stage 01

I.4 Stage 02

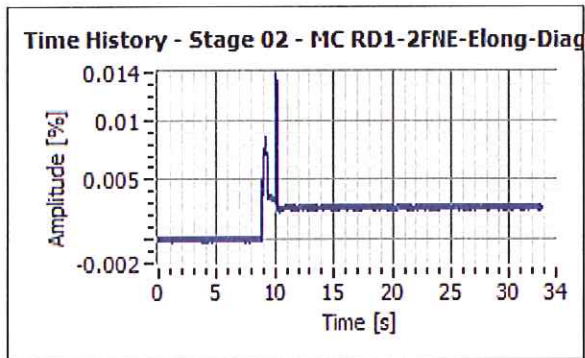


Figure I-10: Time History – RD1 – Diagonal deformation – Stage 02

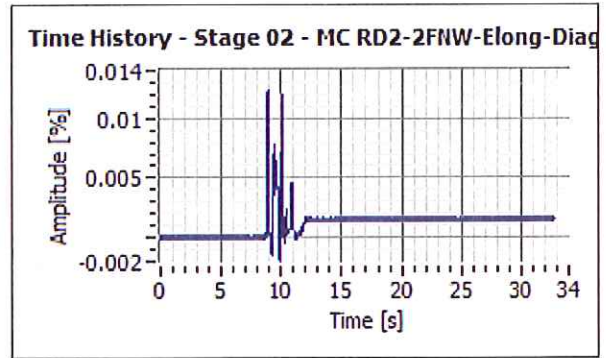


Figure I-11: Time History – RD2 – Diagonal deformation – Stage 02

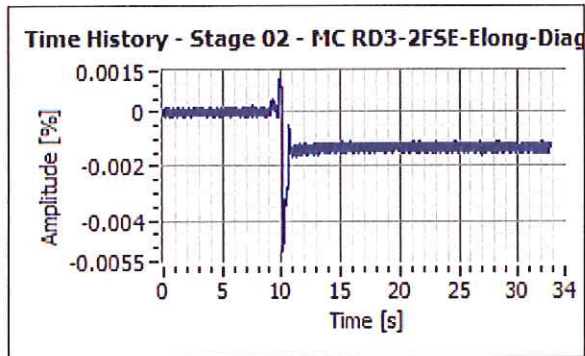


Figure I-12: Time History – RD3 – Diagonal deformation – Stage 02

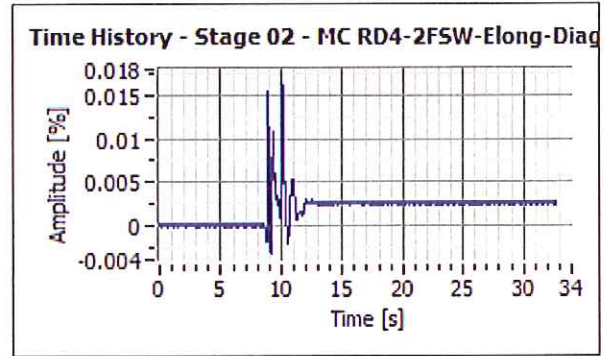


Figure I-13: Time History – RD4 – Diagonal deformation – Stage 02

I.5 Stage 03

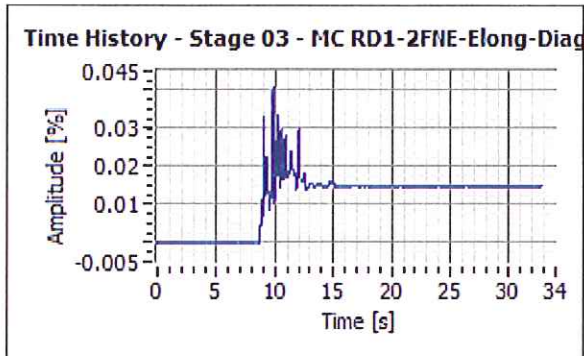


Figure I-14: Time History – RD1 – Diagonal deformation – Stage 03

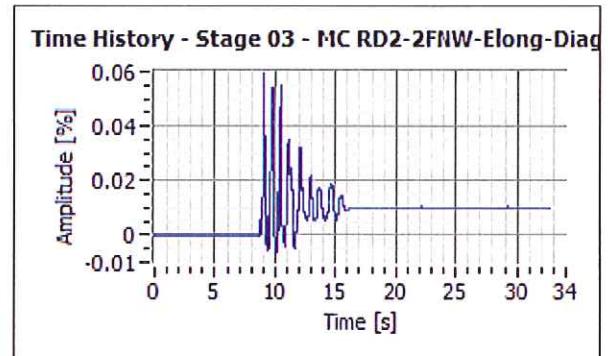


Figure I-15: Time History – RD2 – Diagonal deformation – Stage 03

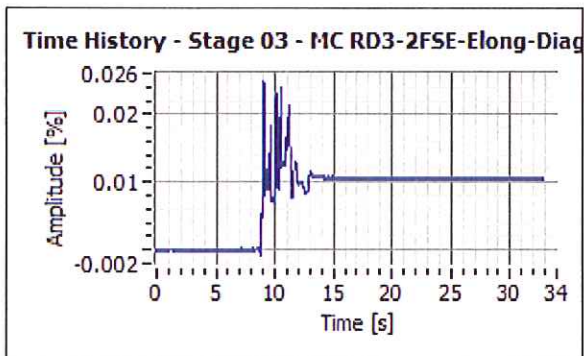


Figure I-16: Time History – RD3 – Diagonal deformation – Stage 03

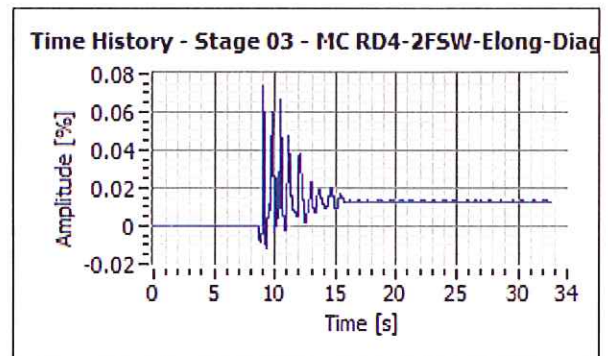


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I.6 Stage 04

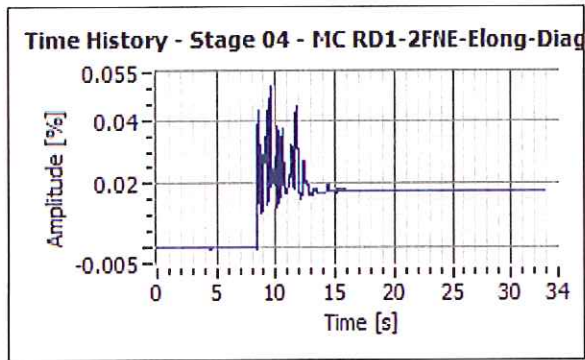


Figure I-18: Time History – RD1 – Diagonal deformation – Stage 04

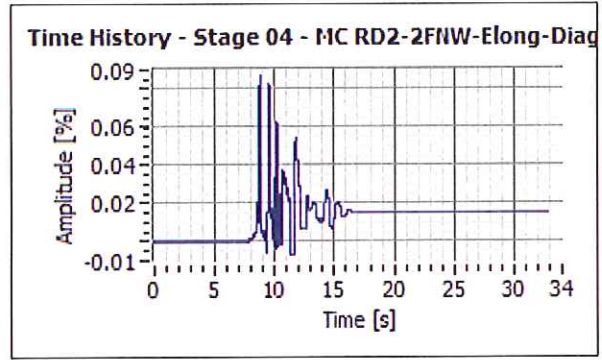


Figure I-19: Time History – RD2 – Diagonal deformation – Stage 04

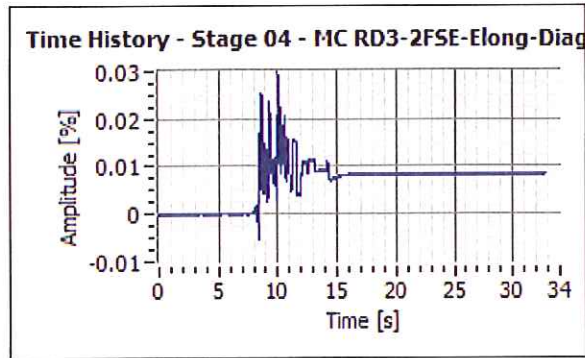


Figure I-20: Time History – RD3 – Diagonal deformation – Stage 04

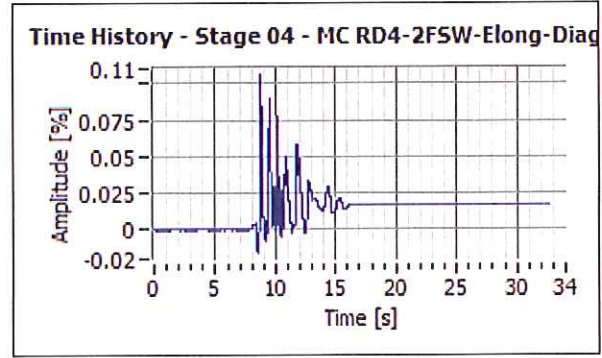
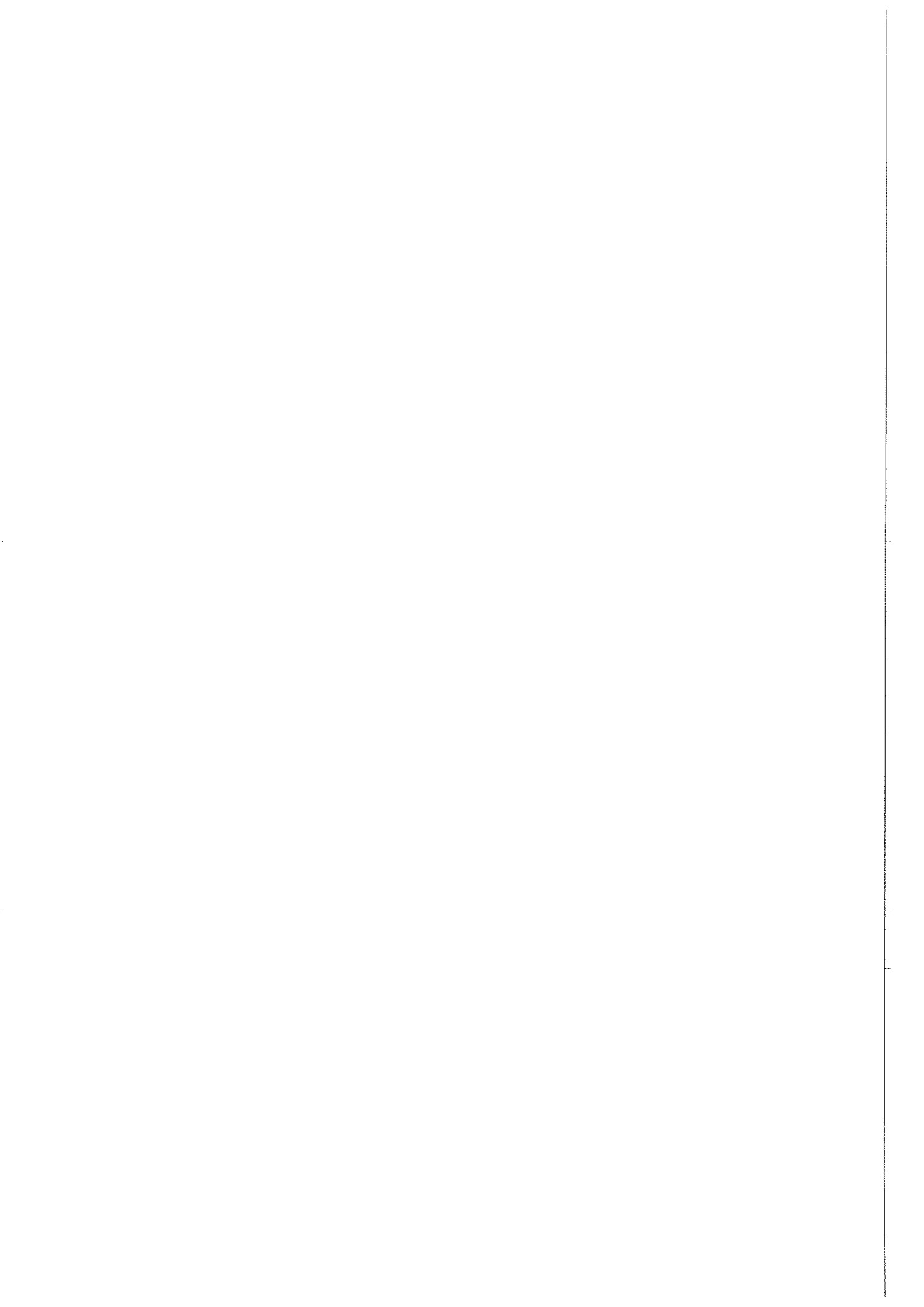


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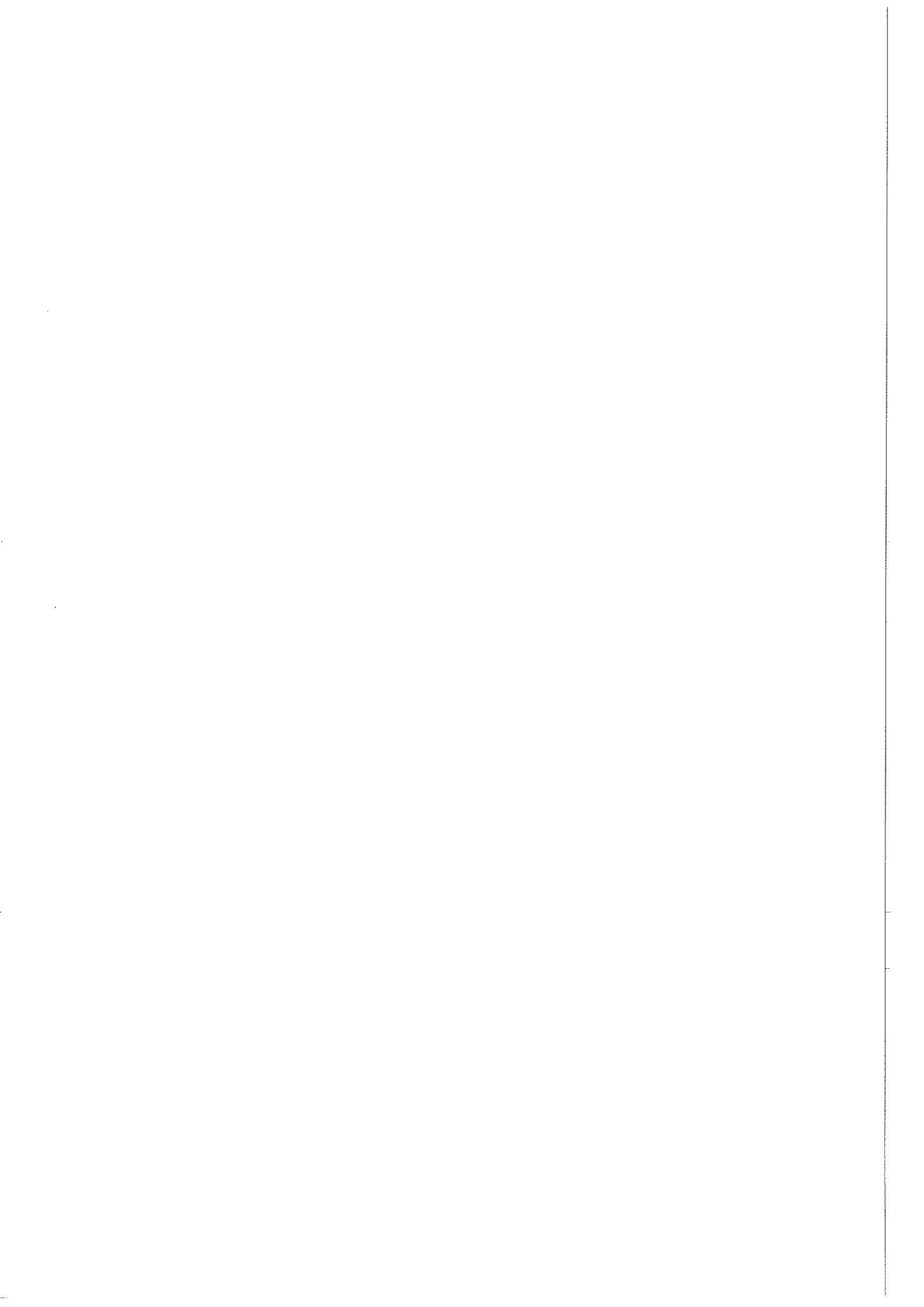
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J.1 Center of Mass Displacements and Rotations

J.1.1 Stage 00

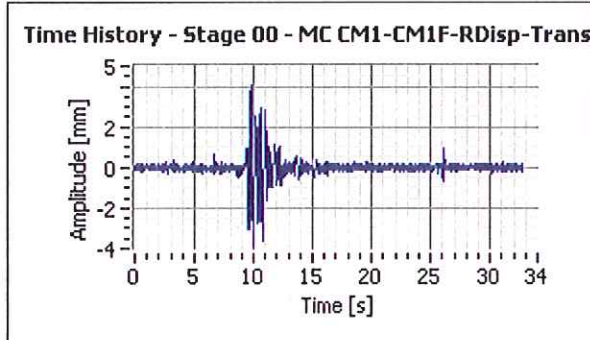


Figure J-1: Time History – 1st Floor Center of Mass Relative Displacement –Trans. – Stage 00

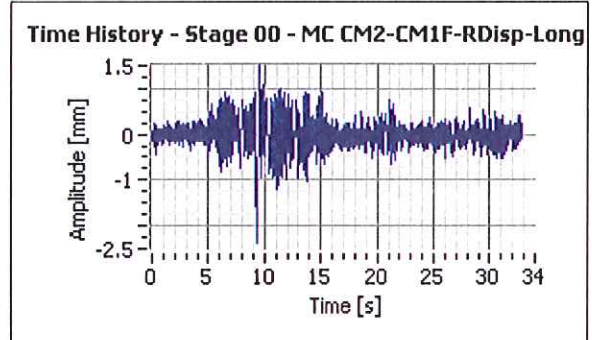


Figure J-2: Time History – 1st Floor Center of Mass Relative Displacement –Long. – Stage 00

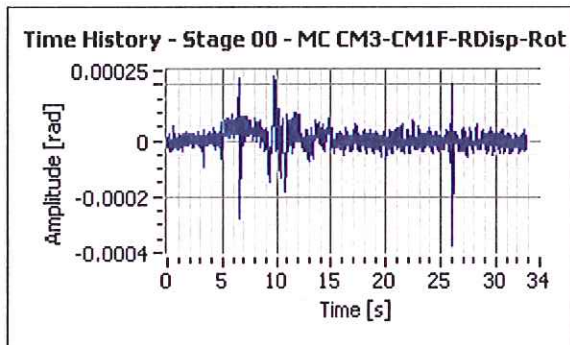


Figure J-3: Time History – 1st Floor Center of Mass Relative Rotation – Stage 00

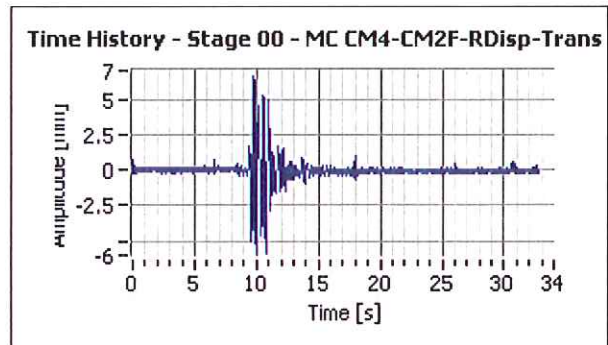


Figure J-4: Time History – 2nd Floor Center of Mass Relative Displacement –Trans. – Stage 00

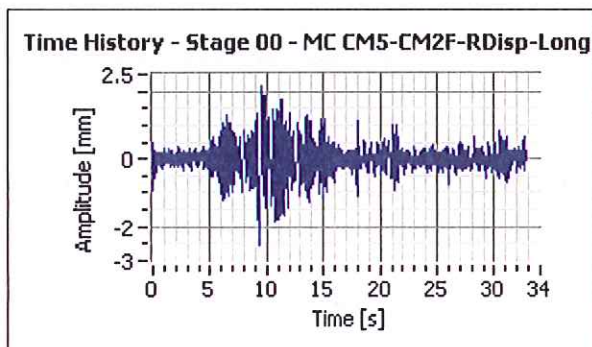


Figure J-5: Time History – 2nd Floor Center of Mass Relative Displacement –Long. – Stage 00

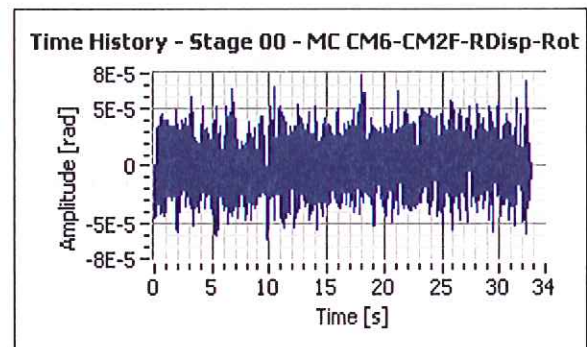


Figure J-6: Time History – 2nd Floor Center of Mass Relative Rotation – Stage 00

J.1.2 Stage 01

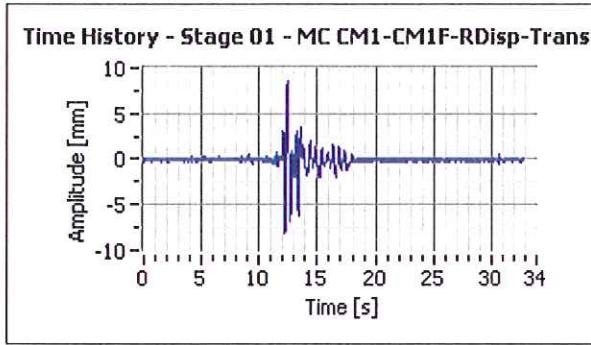


Figure J-7: Time History – 1st Floor Center of Mass Relative Displacement –Trans. – Stage 01

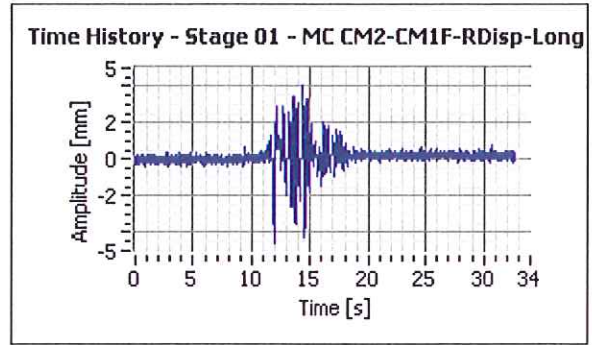


Figure J-8: Time History – 1st Floor Center of Mass Relative Displacement –Long. – Stage 01

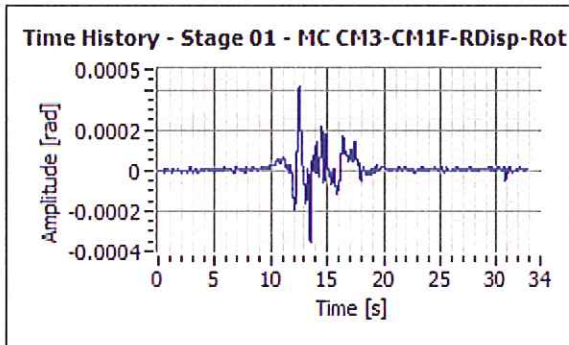


Figure J-9: Time History – 1st Floor Center of Mass Relative Rotation – Stage 01

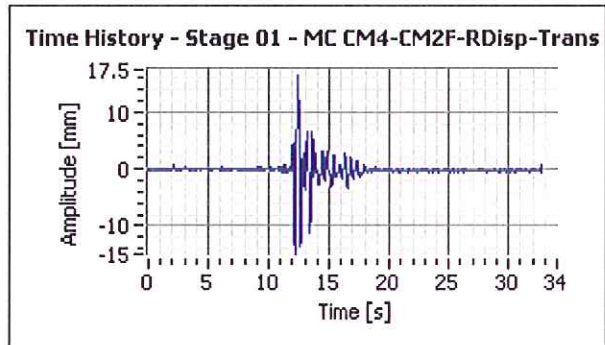


Figure J-10: Time History – 2nd Floor Center of Mass Relative Displacement –Trans. – Stage 01

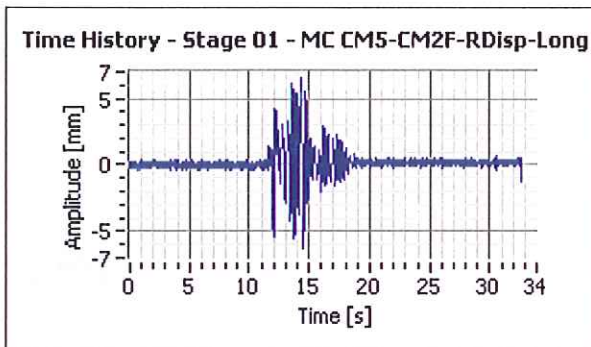


Figure J-11: Time History – 2nd Floor Center of Mass Relative Displacement –Long. – Stage 01

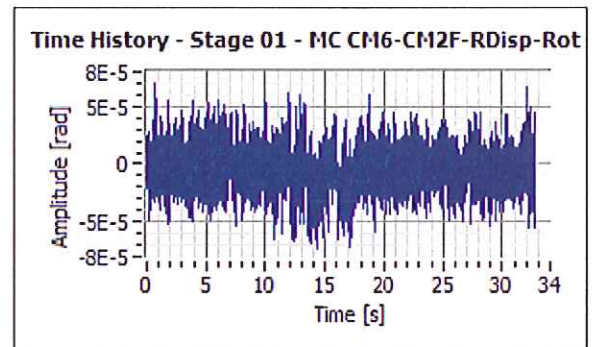


Figure J-12: Time History – 2nd Floor Center of Mass Relative Rotation – Stage 01

J.1.3 Stage 02

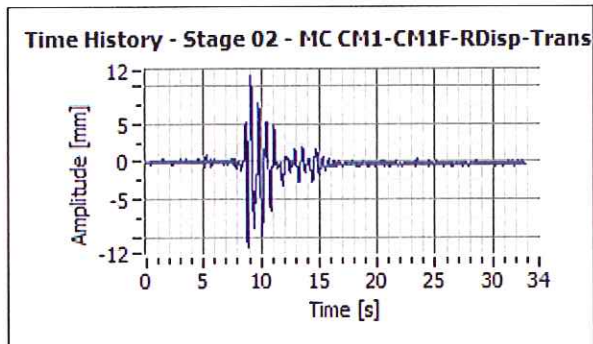


Figure J-13: Time History – 1st Floor Center of Mass Relative Displacement –Trans. – Stage 02

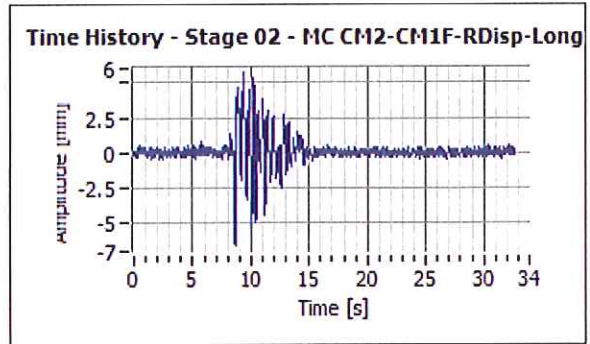


Figure J-14: Time History – 1st Floor Center of Mass Relative Displacement –Long. – Stage 02

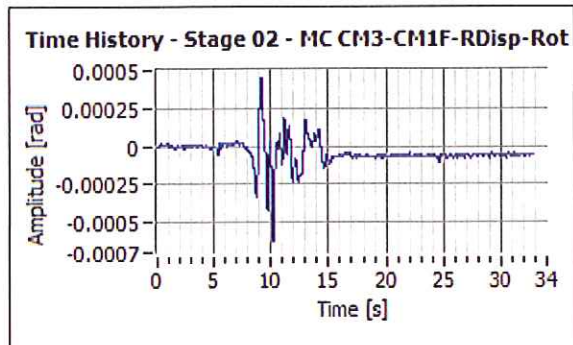


Figure J-15: Time History – 1st Floor Center of Mass Relative Rotation – Stage 02

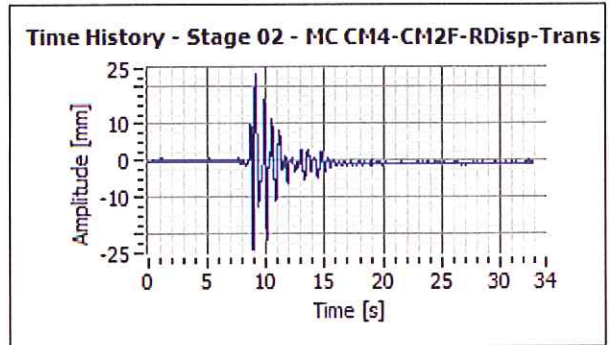


Figure J-16: Time History – 2nd Floor Center of Mass Relative Displacement –Trans. – Stage 02

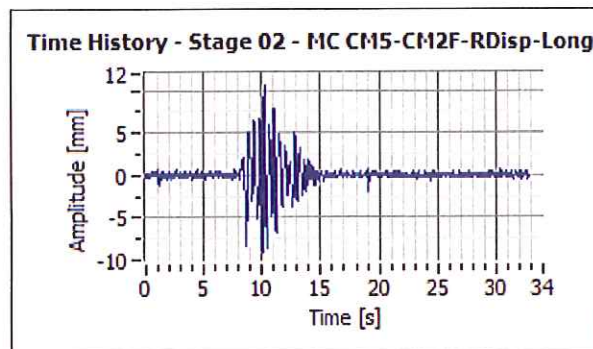


Figure J-17: Time History – 2nd Floor Center of Mass Relative Displacement –Long. – Stage 02

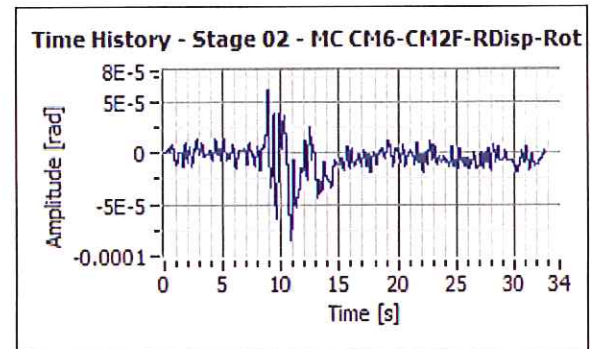


Figure J-18: Time History – 2nd Floor Center of Mass Relative Rotation – Stage 02

J.1.4 Stage 03

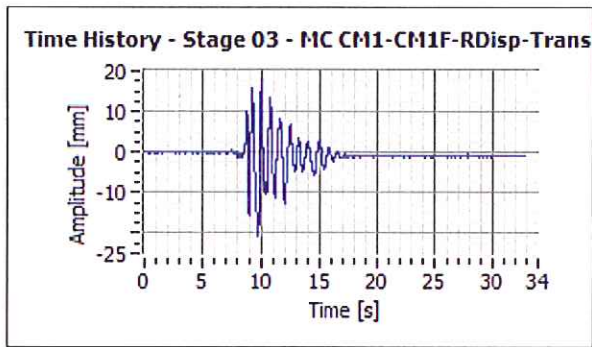


Figure J-19: Time History – 1st Floor Center of Mass Relative Displacement –Trans. – Stage 03

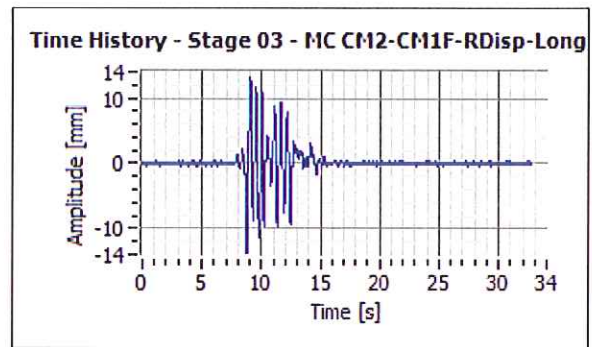


Figure J-20: Time History – 1st Floor Center of Mass Relative Displacement –Long. – Stage 03

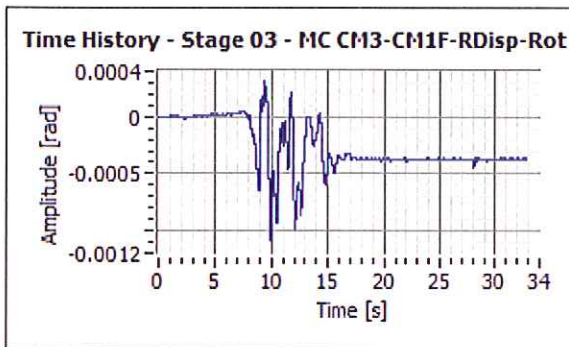


Figure J-21: Time History – 1st Floor Center of Mass Relative Rotation – Stage 03

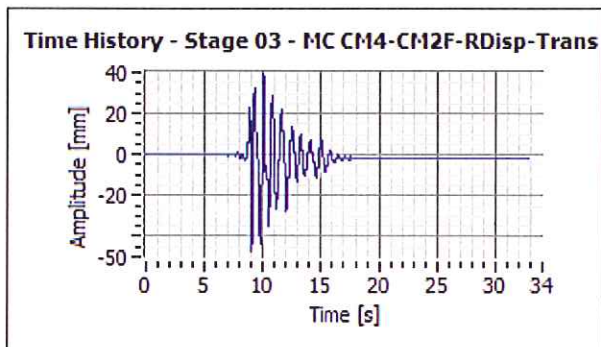


Figure J-22: Time History – 2nd Floor Center of Mass Relative Displacement –Trans. – Stage 03

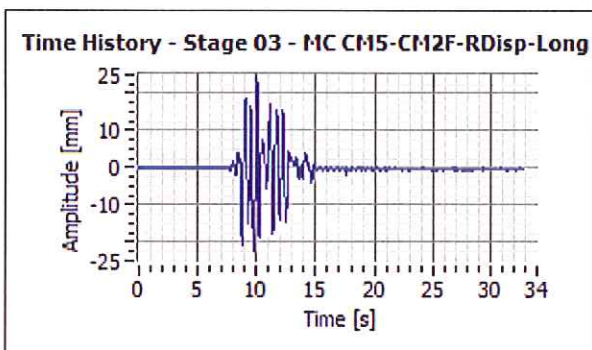


Figure J-23: Time History – 2nd Floor Center of Mass Relative Displacement –Long. – Stage 03

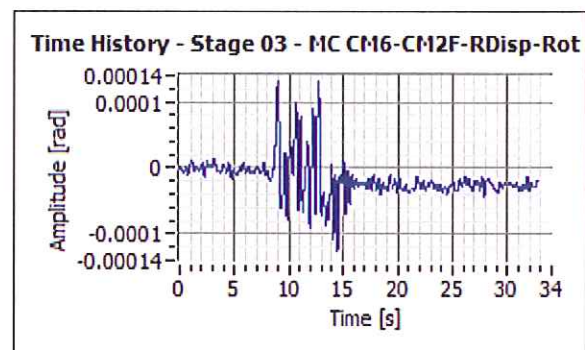


Figure J-24: Time History – 2nd Floor Center of Mass Relative Rotation – Stage 03

J.1.5 Stage 04

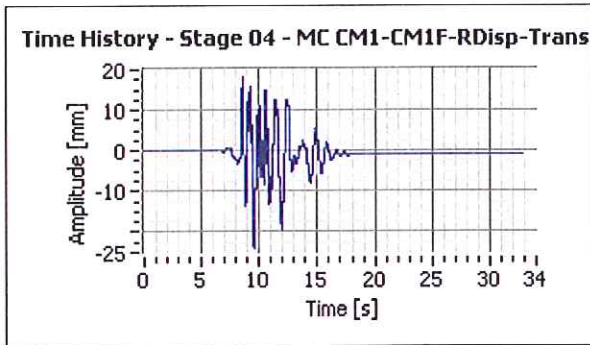


Figure J-25: Time History – 1st Floor Center of Mass Relative Displacement –Trans. – Stage 04

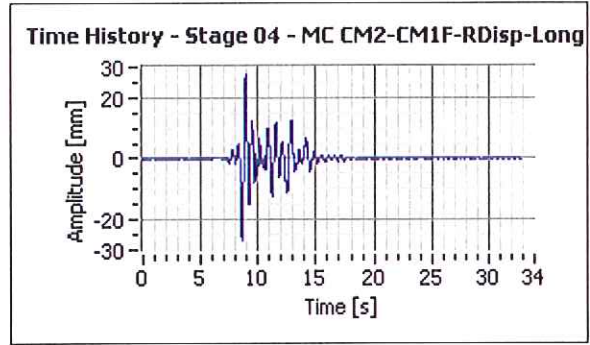


Figure J-26: Time History – 1st Floor Center of Mass Relative Displacement –Long. – Stage 04

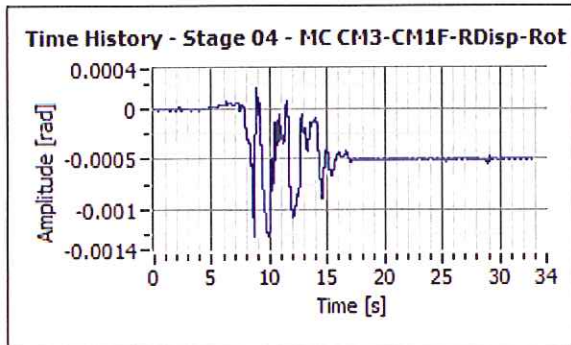


Figure J-27: Time History – 1st Floor Center of Mass Relative Rotation – Stage 04

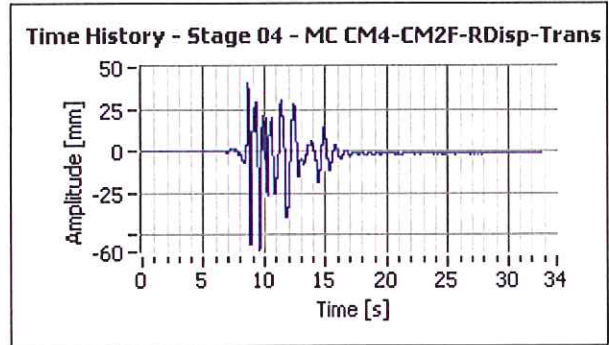


Figure J-28: Time History – 2nd Floor Center of Mass Relative Displacement –Trans. – Stage 04

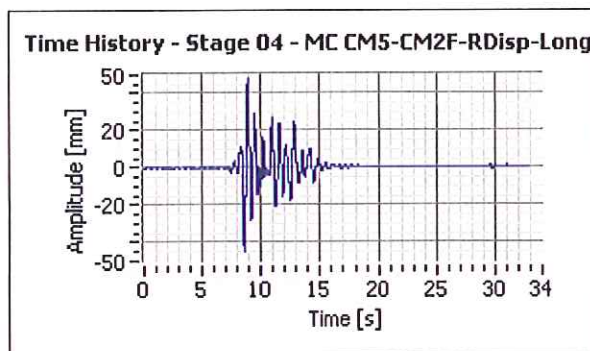


Figure J-29: Time History – 2nd Floor Center of Mass Relative Displacement –Long. – Stage 04

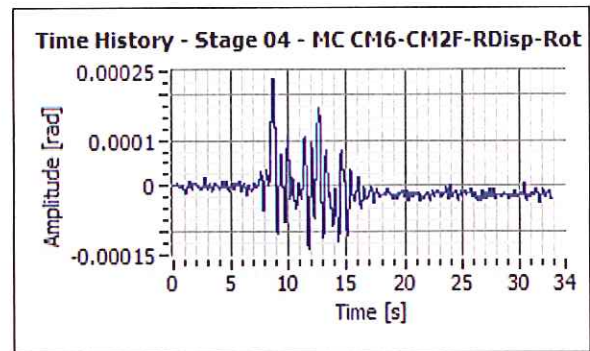


Figure J-30: Time History – 2nd Floor Center of Mass Relative Rotation – Stage 04

J.2 Center of Mass Accelerations

J.2.1 Stage 00

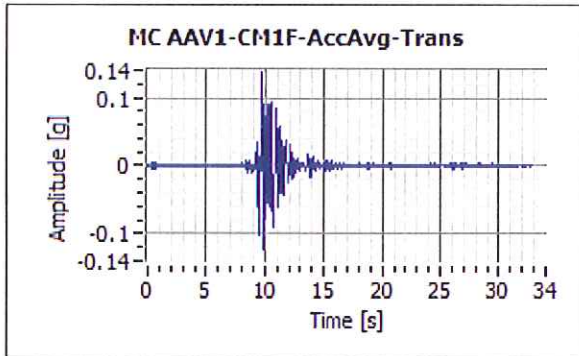


Figure J-31: Time History – 1st Floor Center of Mass Accelerations –Trans. – Stage 00

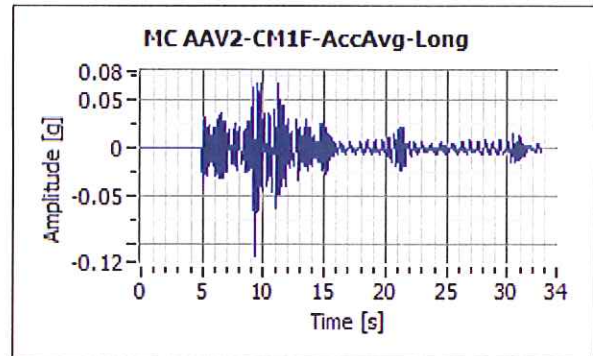


Figure J-32: Time History – 1st Floor Center of Mass Accelerations –Long. – Stage 00

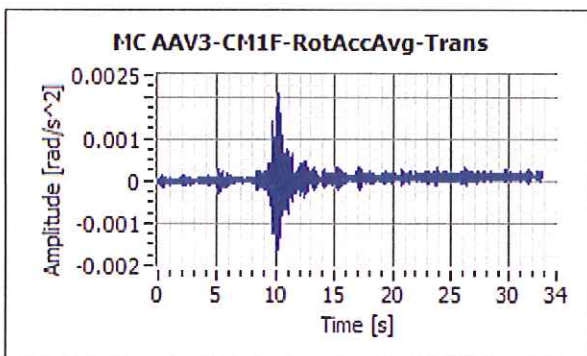


Figure J-33: Time History – 1st Floor Center of Mass Rotational Acceleration – Stage 00

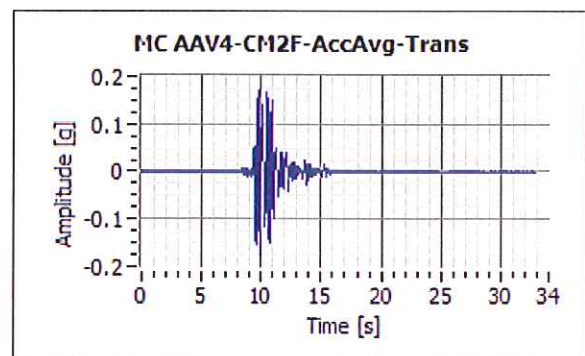


Figure J-34: Time History – 2nd Floor Center of Mass Accelerations –Trans. – Stage 00

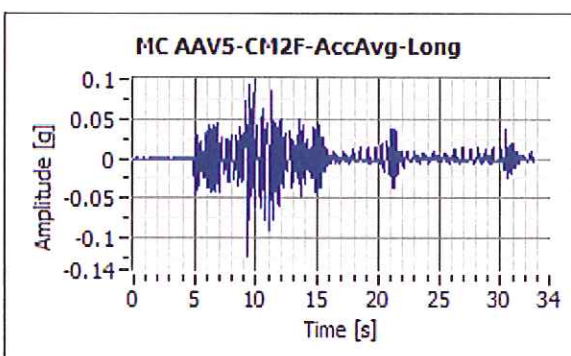


Figure J-35: Time History – 2nd Floor Center of Mass Accelerations –Long. – Stage 00

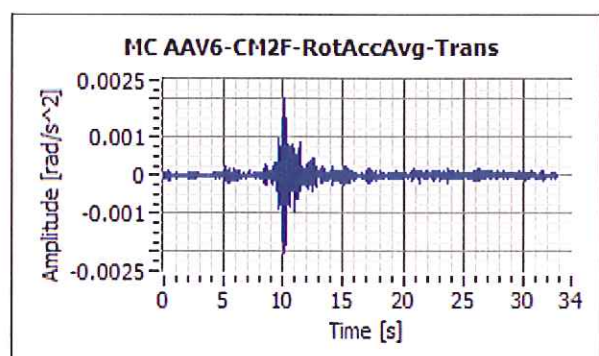


Figure J-36: Time History – 2nd Floor Center of Mass Rotational Acceleration – Stage 00

J.2.2 Stage 01

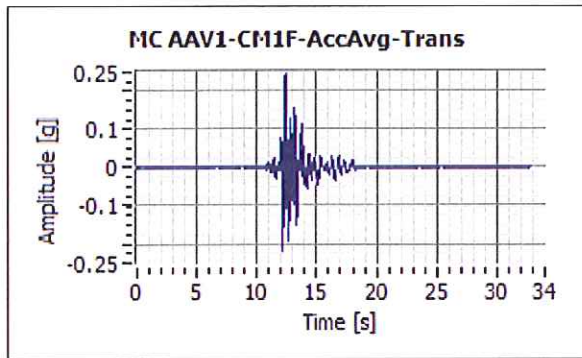


Figure J-37: Time History – 1st Floor Center of Mass Accelerations –Trans. – Stage 01

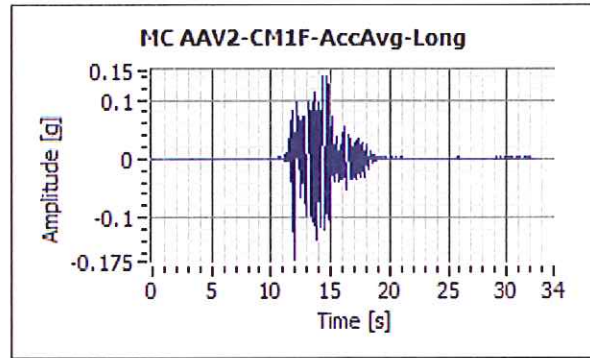


Figure J-38: Time History – 1st Floor Center of Mass Accelerations –Long. – Stage 01

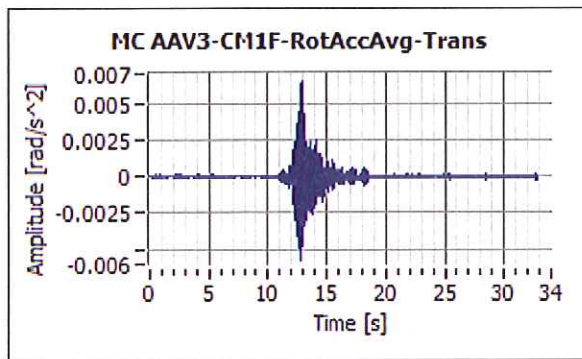


Figure J-39: Time History – 1st Floor Center of Mass Rotational Acceleration – Stage 01

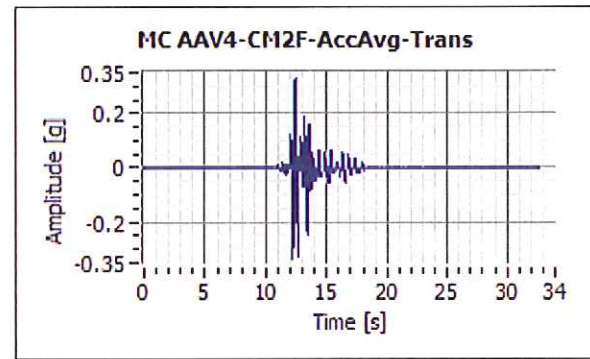


Figure J-40: Time History – 2nd Floor Center of Mass Accelerations –Trans. – Stage 01

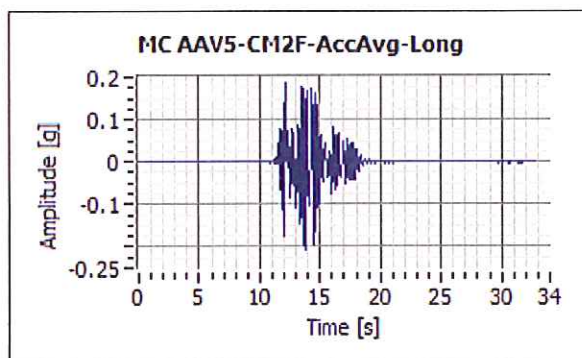


Figure J-41: Time History – 2nd Floor Center of Mass Displacement –Long. – Stage 01

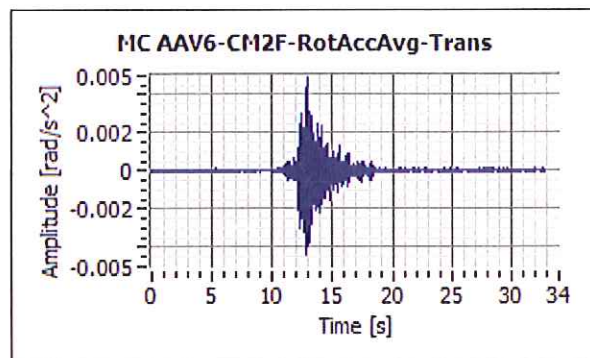


Figure J-42: Time History – 2nd Floor Center of Mass Rotational Acceleration – Stage 01

J.2.3 Stage 02

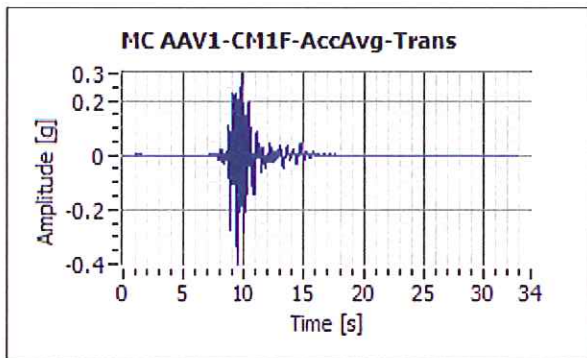


Figure J-43: Time History – 1st Floor Center of Mass Accelerations –Trans. – Stage 02

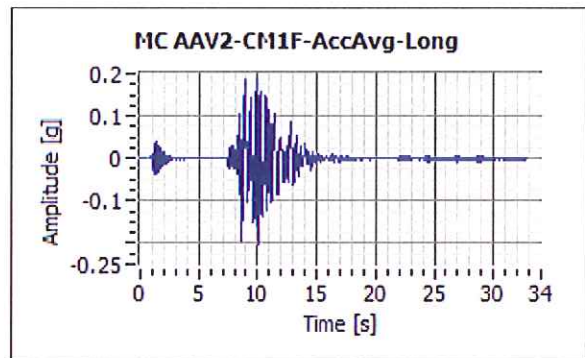


Figure J-44: Time History – 1st Floor Center of Mass Accelerations –Long. – Stage 02

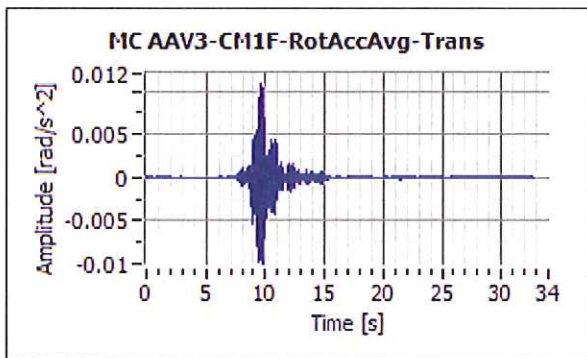


Figure J-45: Time History – 1st Floor Center of Mass Rotational Acceleration – Stage 02

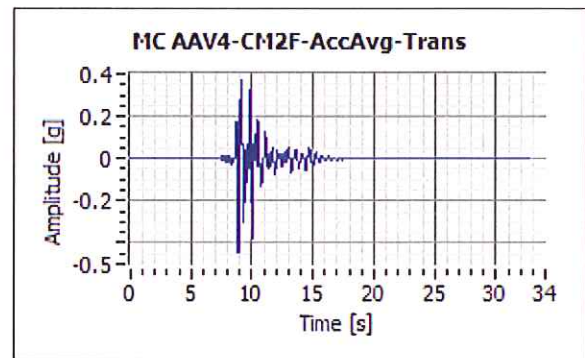


Figure J-46: Time History – 2nd Floor Center of Mass Accelerations –Trans. – Stage 02

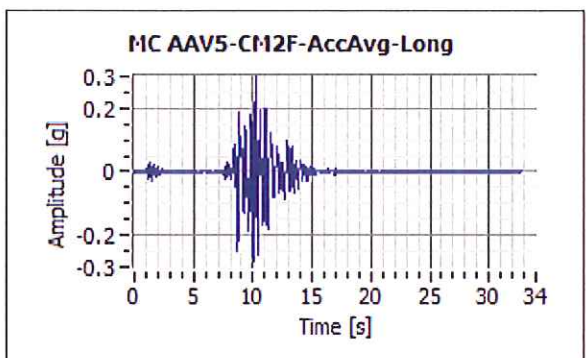


Figure J-47: Time History – 2nd Floor Center of Mass Accelerations –Long. – Stage 02

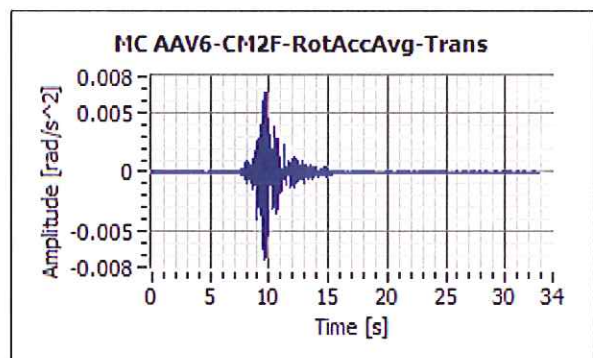


Figure J-48: Time History – 2nd Floor Center of Mass Rotational Acceleration – Stage 02

J.2.4 Stage 03

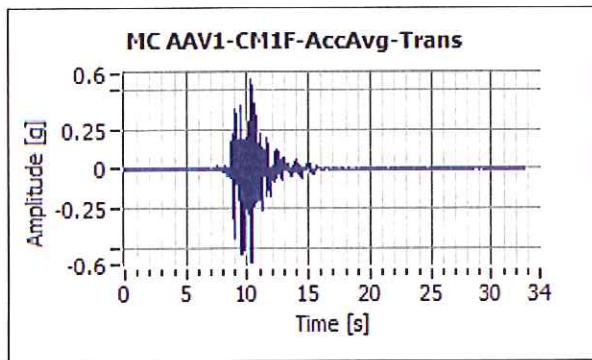


Figure J-49: Time History – 1st Floor Center of Mass Accelerations –Trans. – Stage 03

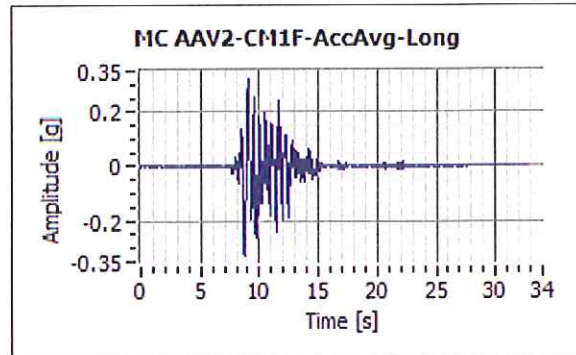


Figure J-50: Time History – 1st Floor Center of Mass Accelerations –Long. – Stage 03

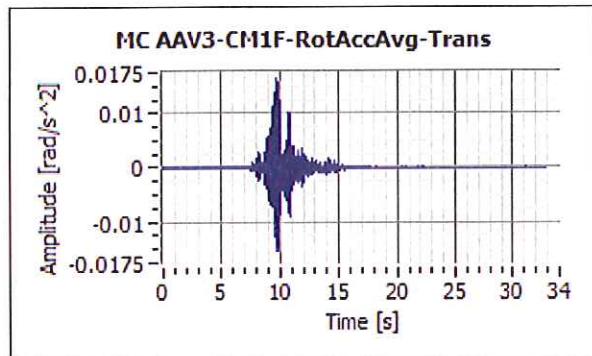


Figure J-51: Time History – 1st Floor Center of Mass Rotational Acceleration – Stage 03

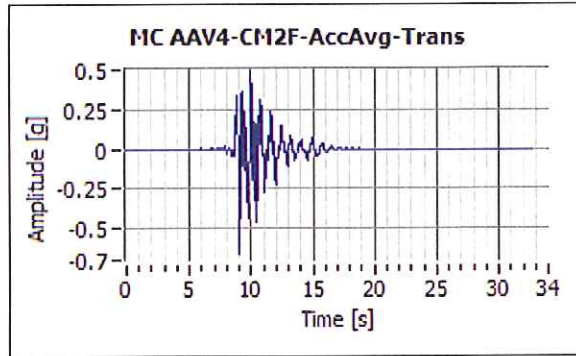


Figure J-52: Time History – 2nd Floor Center of Mass Accelerations –Trans. – Stage 03

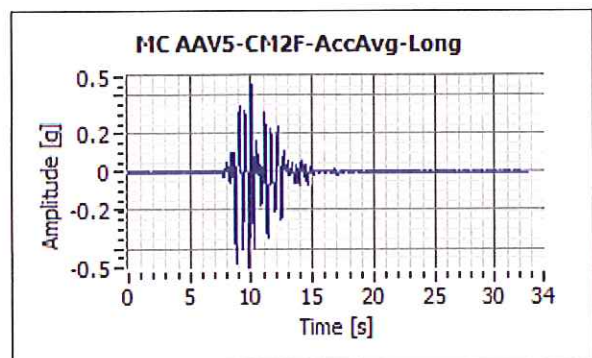


Figure J-53: Time History – 2nd Floor Center of Mass Accelerations –Long. – Stage 03

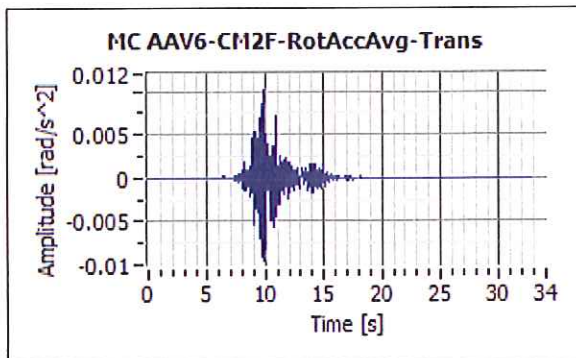


Figure J-54: Time History – 2nd Floor Center of Mass Rotational Acceleration – Stage 03

J.2.5 Stage 04

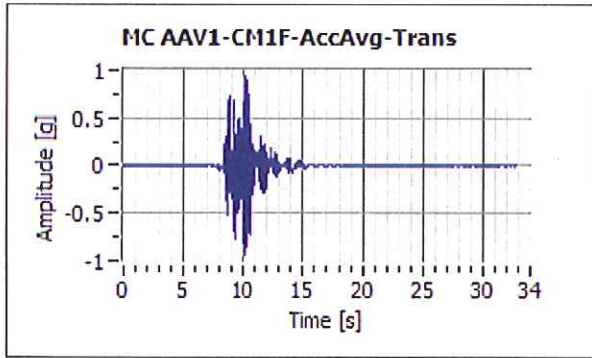


Figure J-55: Time History – 1st Floor Center of Mass Accelerations –Trans. – Stage 04

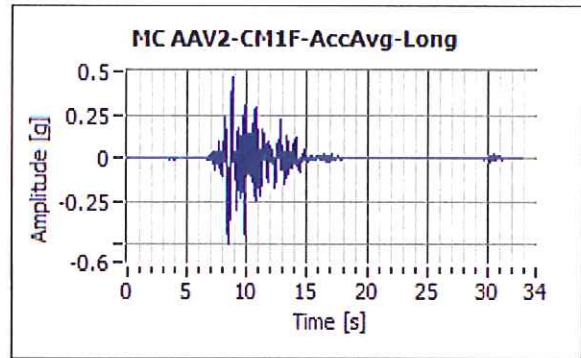


Figure J-56: Time History – 1st Floor Center of Mass Accelerations –Long. – Stage 04

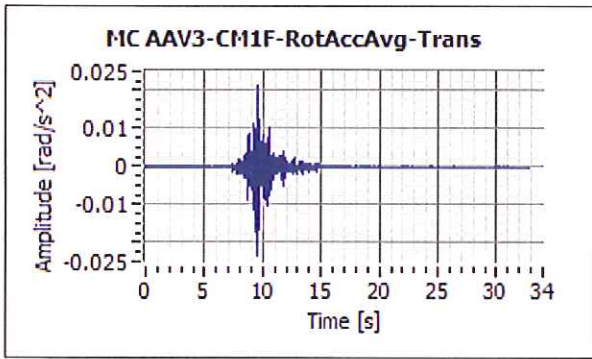


Figure J-57: Time History – 1st Floor Center of Mass Rotational Acceleration – Stage 04

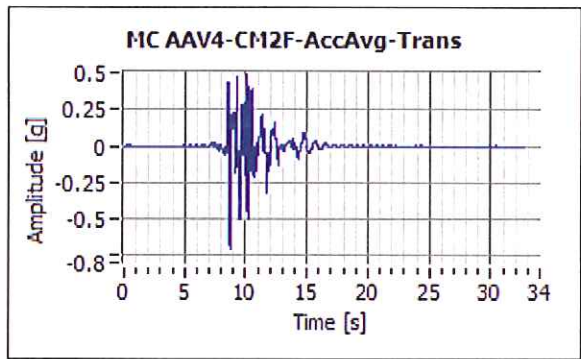


Figure J-58: Time History – 2nd Floor Center of Mass Accelerations –Trans. – Stage 04

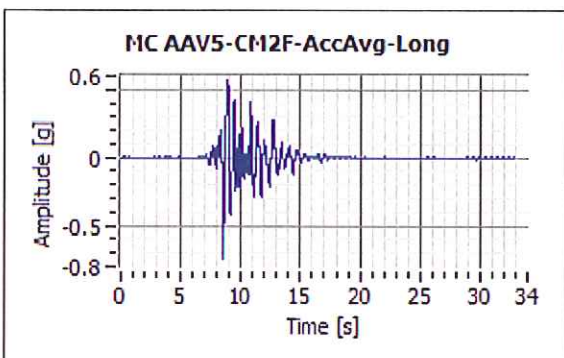


Figure J-59: Time History – 2nd Floor Center of Mass Accelerations –Long. – Stage 04

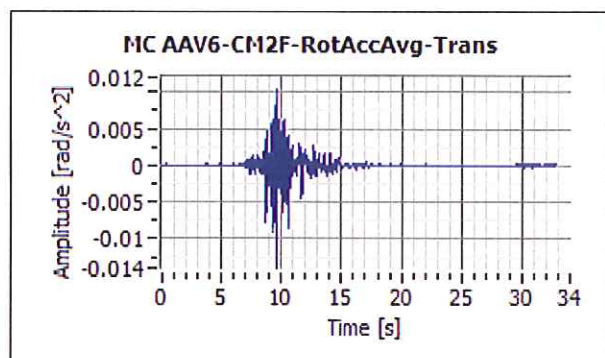


Figure J-60: Time History – 2nd Floor Center of Mass Rotational Acceleration – Stage 04

J.3 Center of Mass Polar Diagrams

J.3.1 Stage 00

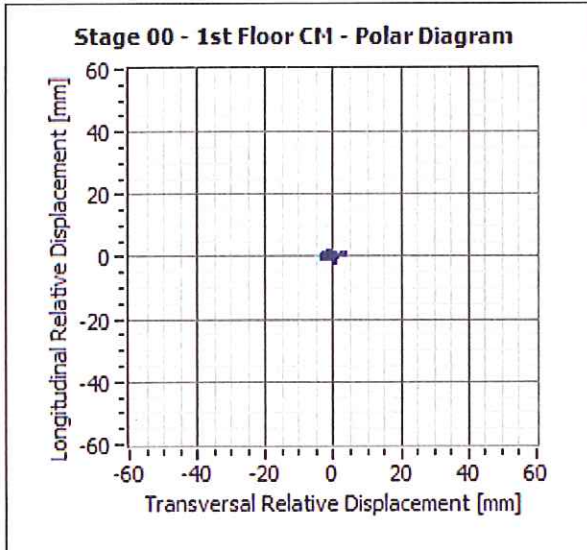


Figure J-61: Time History -1st Floor Center of Mass Polar Diagram – Stage 00

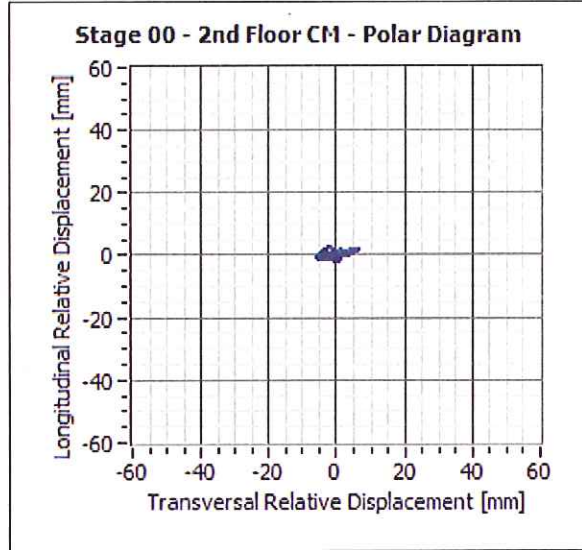


Figure J-62: Time History -2nd Floor Center of Mass Polar Diagram – Stage 00

J.3.2 Stage 01

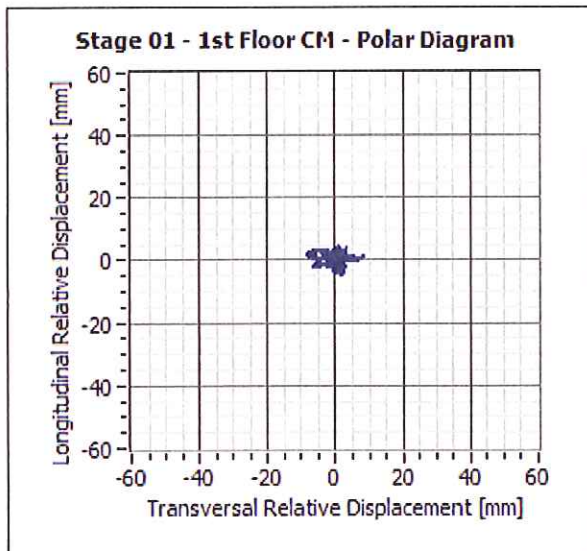


Figure J-63: Time History -1st Floor Center of Mass Polar Diagram – Stage 01

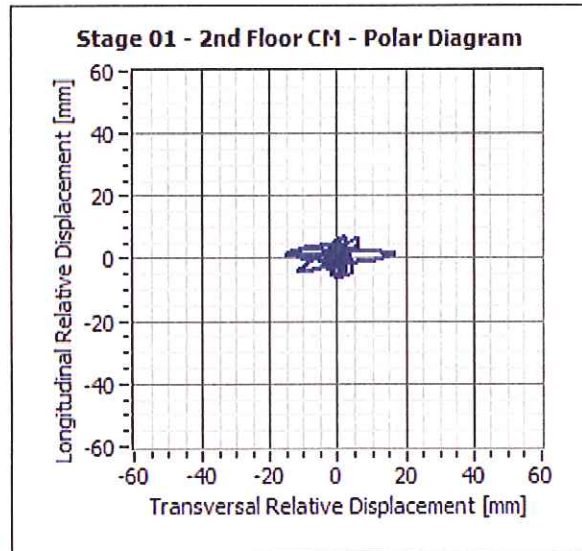


Figure J-64: Time History -2nd Floor Center of Mass Polar Diagram – Stage 01

J.3.3 Stage 02

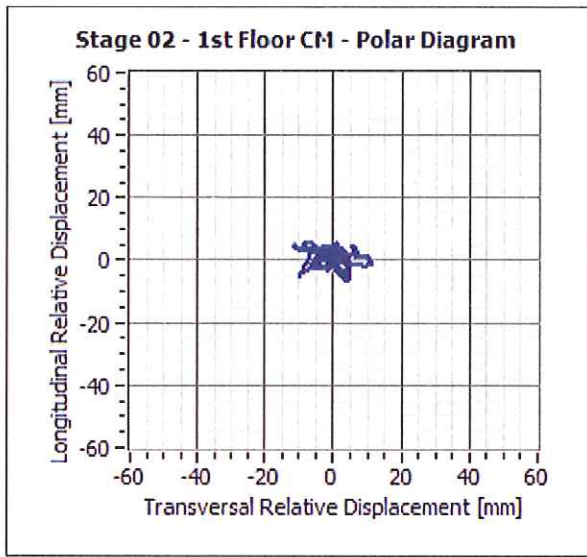


Figure J-65: Time History –1st Floor Center of Mass Polar Diagram – Stage 02

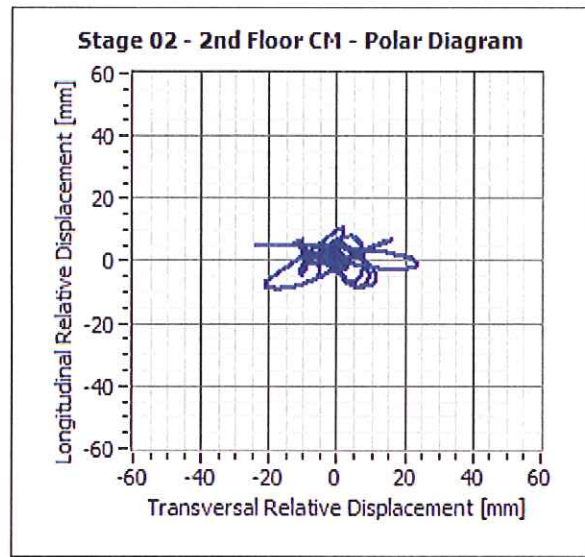


Figure J-66: Time History –2nd Floor Center of Mass Polar Diagram – Stage 02

J.3.4 Stage 03

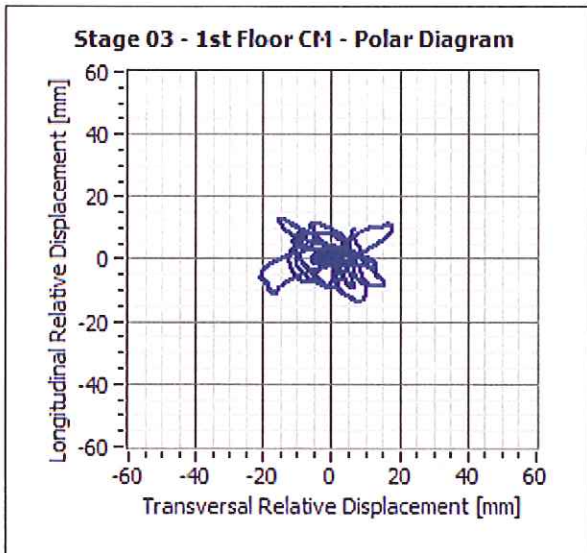


Figure J-67: Time History –1st Floor Center of Mass Polar Diagram – Stage 03

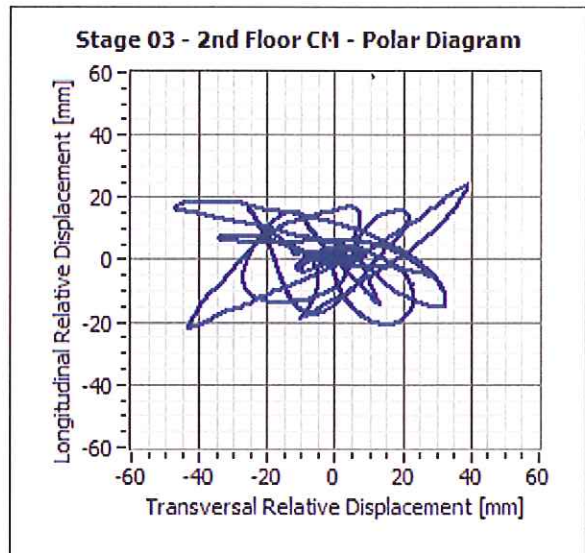


Figure J-68: Time History –2nd Floor Center of Mass Polar Diagram – Stage 03

J.3.5 Stage 04

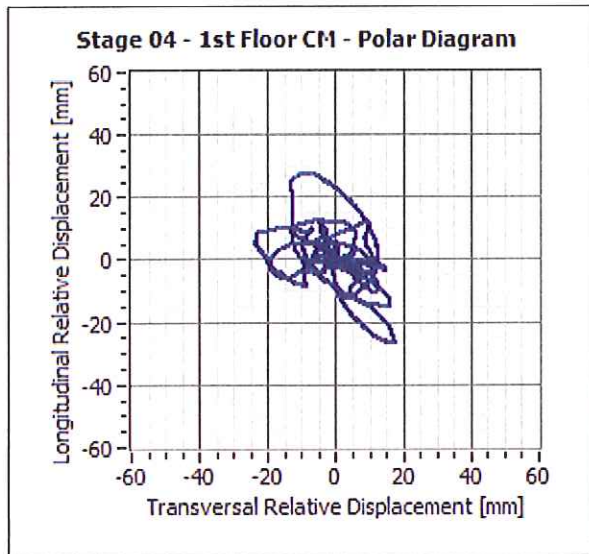


Figure J-69: Time History –1st Floor Center of Mass Polar Diagram – Stage 04

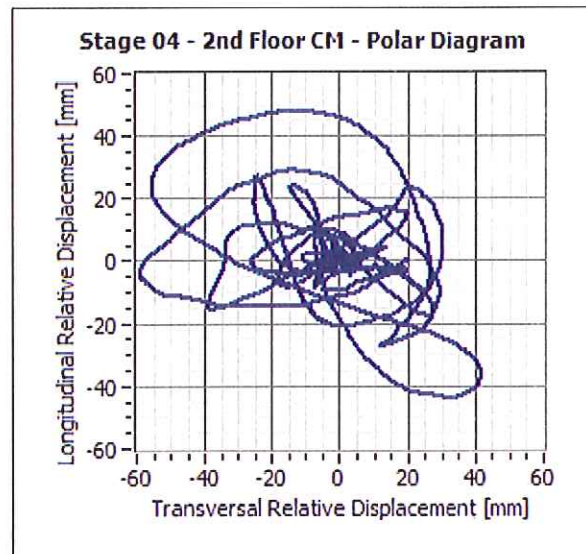
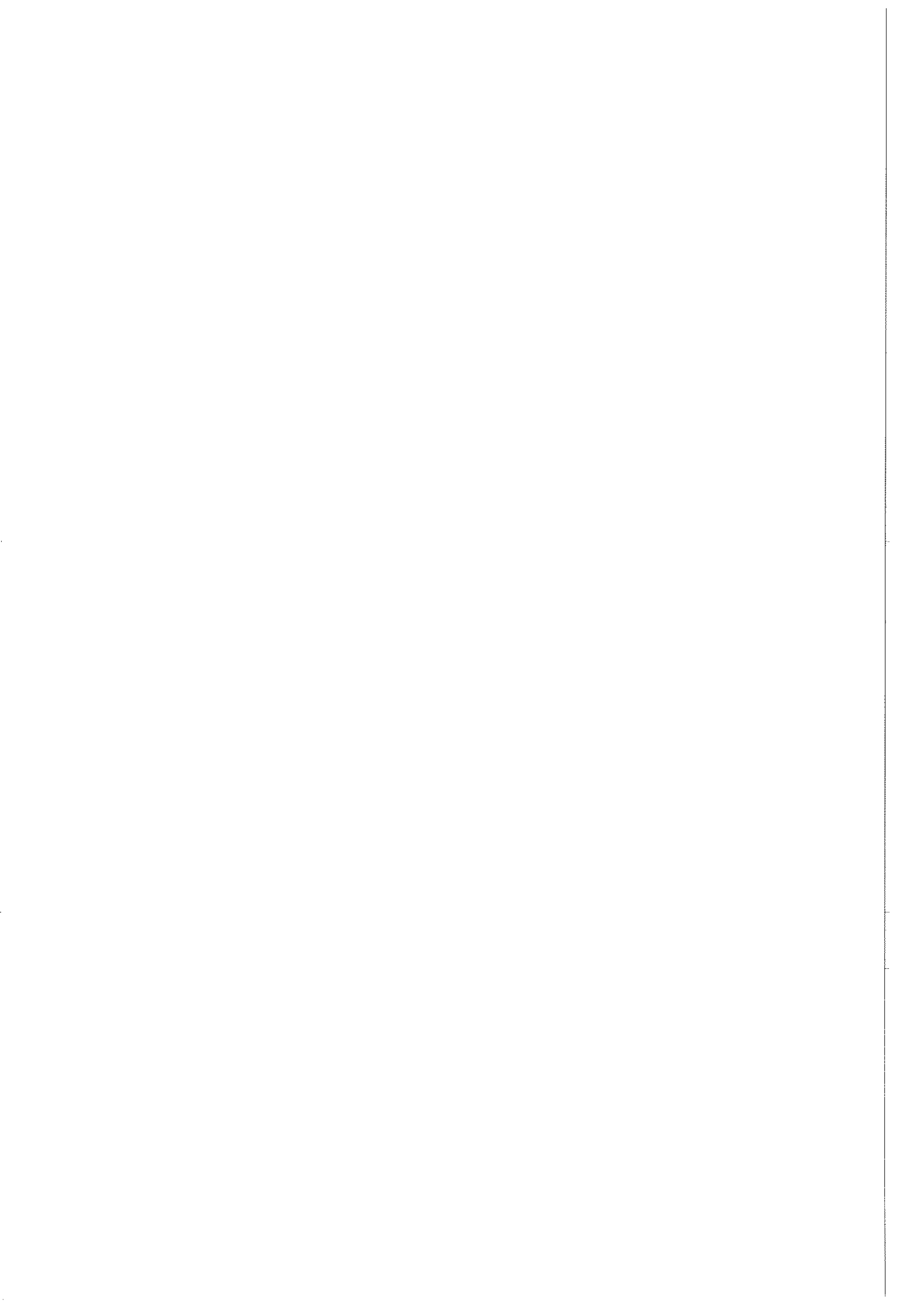


Figure J-70: Time History –2nd Floor Center of Mass Polar Diagram – Stage 04



ANNEX K INTERSTOREY DRIFT

In this annex is presented the interstorey drifts time histories of the 8 corner columns and of the center of mass of each floor.

		Position	Label
Columns	First floor	NW	ID1
	First floor	SE	ID2
	First floor	NE	ID3
	First floor	SW	ID4
	Second floor	NE	ID5
	Second floor	NW	ID6
	Second floor	SE	ID7
	Second floor	SW	ID8
CM	First floor	CM	ID9
	Second floor	CM	ID10

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K.1 Stage 00

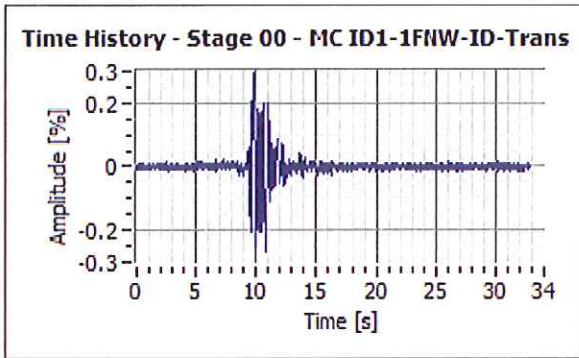


Figure K-1: Time History – Interstorey Drift ID1 – Stage 00 – Transversal direction

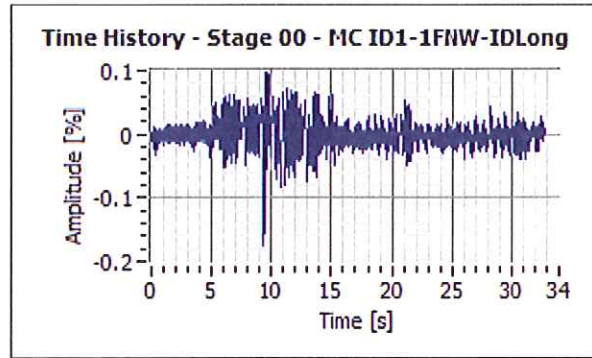


Figure K-2: Time History – Interstorey Drift ID1 – Stage 00 – Longitudinal direction

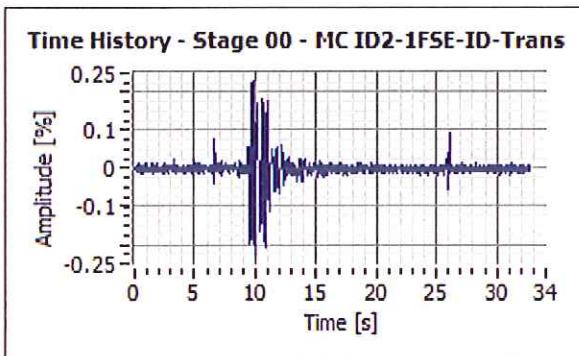


Figure K-3: Time History – Interstorey Drift ID2 – Stage 00 – Transversal direction

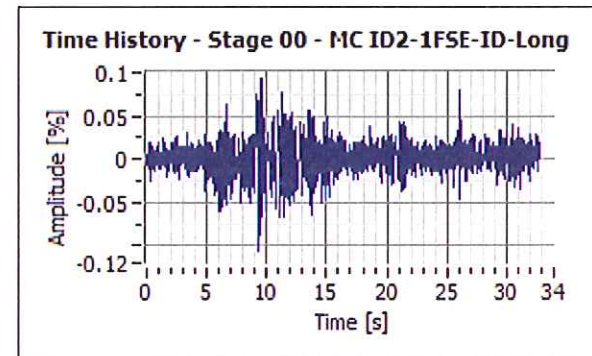


Figure K-4: Time History – Interstorey Drift ID2 – Stage 00 – Longitudinal direction

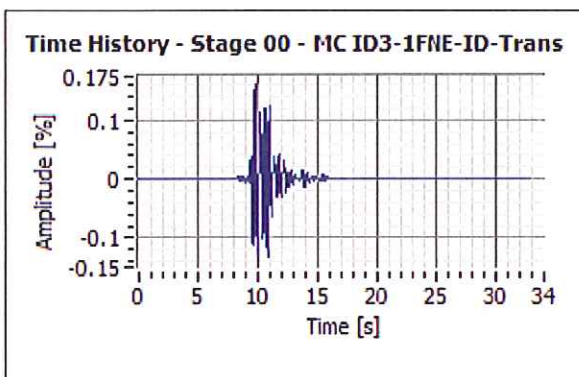


Figure K-5: Time History – Interstorey Drift ID3 – Stage 00 – Transversal direction

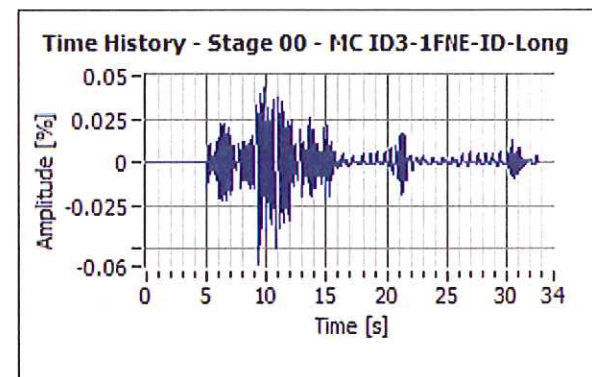


Figure K-6: Time History – Interstorey Drift ID3 – Stage 00 – Longitudinal direction

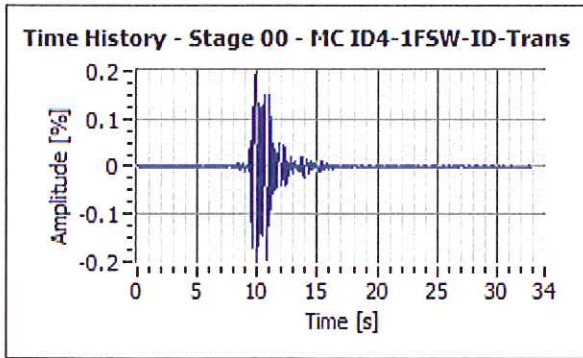


Figure K-7: Time History – Interstorey Drift ID4 – Stage 00 – Transversal direction

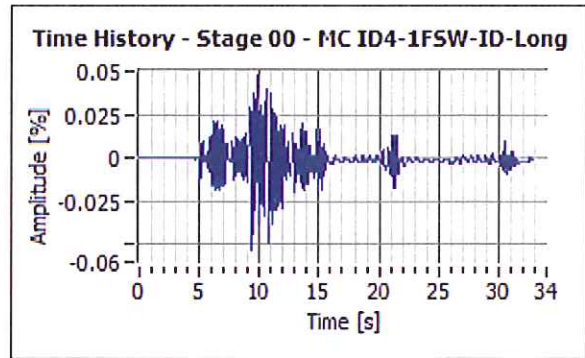


Figure K-8: Time History – Interstorey Drift ID4 – Stage 00 – Longitudinal direction

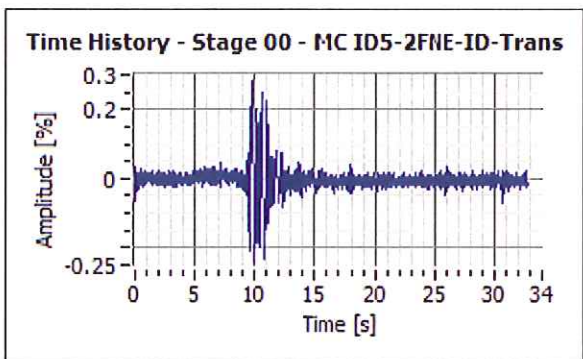


Figure K-9: Time History – Interstorey Drift ID5 – Stage 00 – Transversal direction

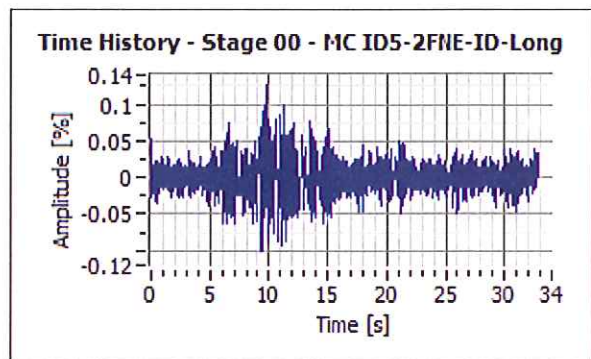


Figure K-10: Time History – Interstorey Drift ID5 – Stage 00 – Longitudinal direction

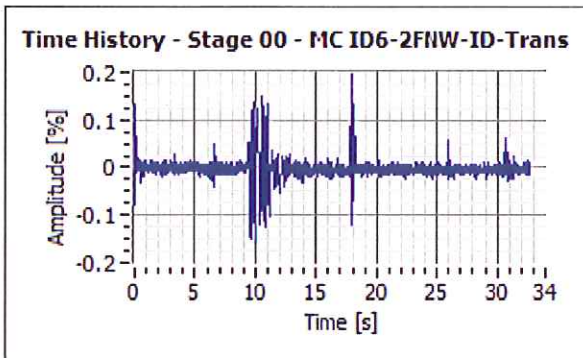


Figure K-11: Time History – Interstorey Drift ID6 – Stage 00 – Transversal direction

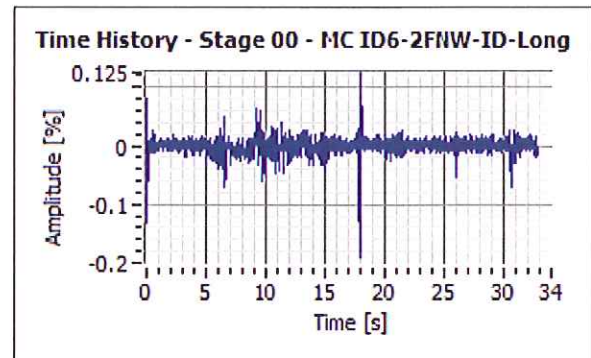


Figure K-12: Time History – Interstorey Drift ID6 – Stage 00 – Longitudinal direction

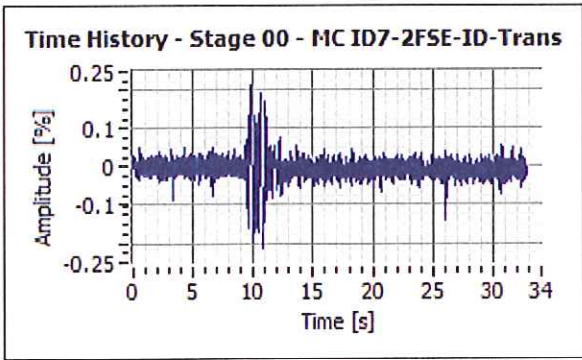


Figure K-13: Time History – Interstorey Drift ID7 – Stage 00 – Transversal direction

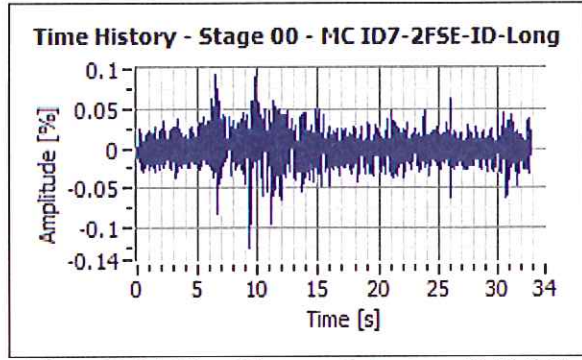


Figure K-14: Time History – Interstorey Drift ID7 – Stage 00 – Longitudinal direction

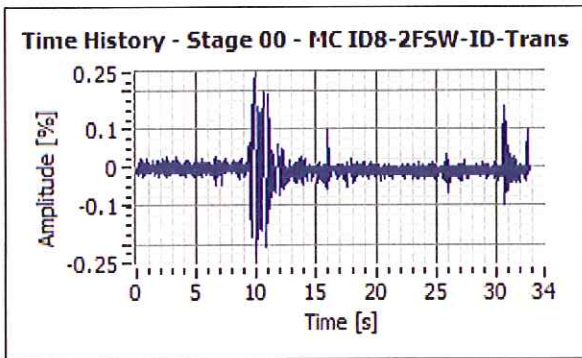


Figure K-15: Time History – Interstorey Drift ID8 – Stage 00 – Transversal direction

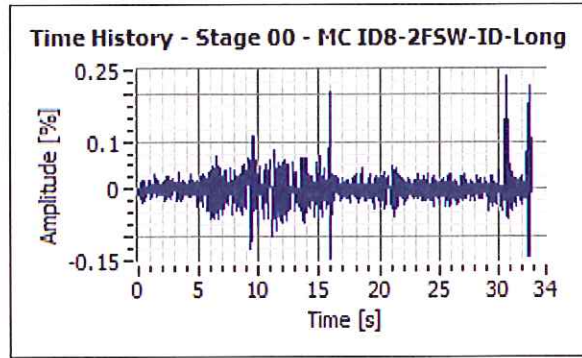


Figure K-16: Time History – Interstorey Drift ID8 – Stage 00 – Longitudinal direction

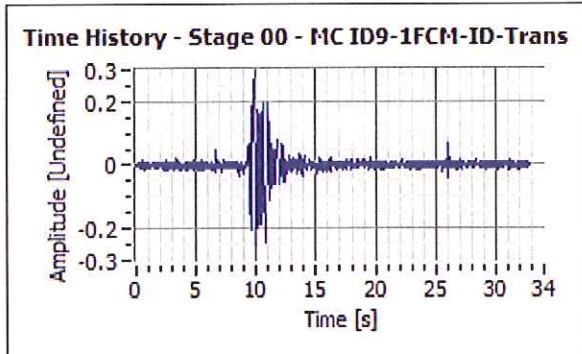


Figure K-17: Time History – Interstorey Drift ID9 – Stage 00 – Transversal direction

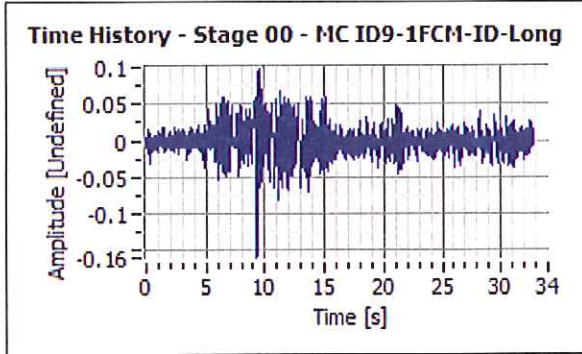


Figure K-18: Time History – Interstorey Drift ID9 – Stage 00 – Longitudinal direction

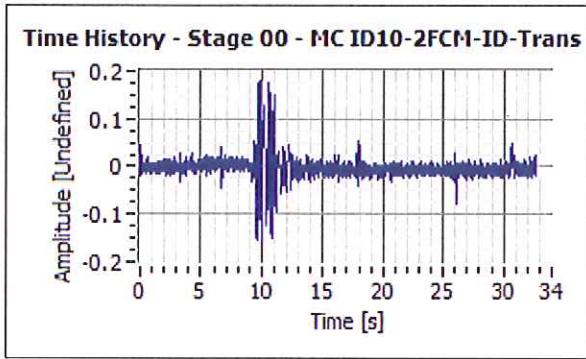


Figure K-19: Time History – Interstorey Drift ID10 – Stage 00 – Transversal direction

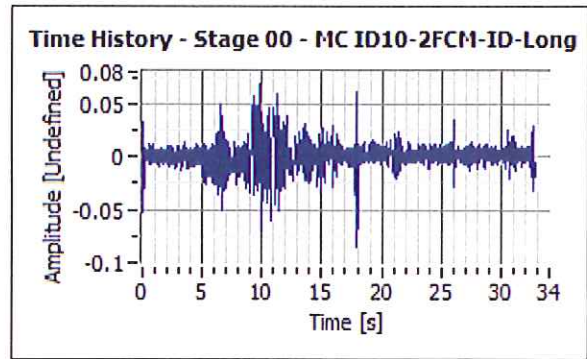


Figure K-20: Time History – Interstorey Drift ID10 – Stage 00 – Longitudinal direction

K.2 Stage 01

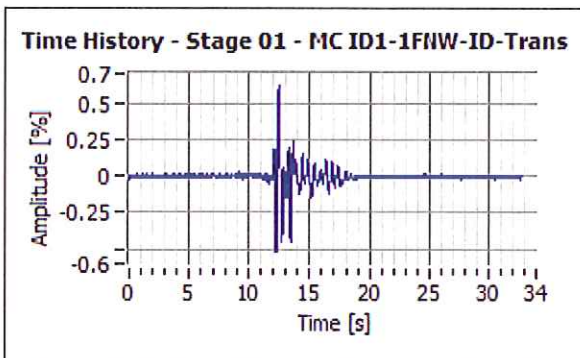


Figure K-21: Time History – Interstorey Drift ID1 – Stage 01 – Transversal direction

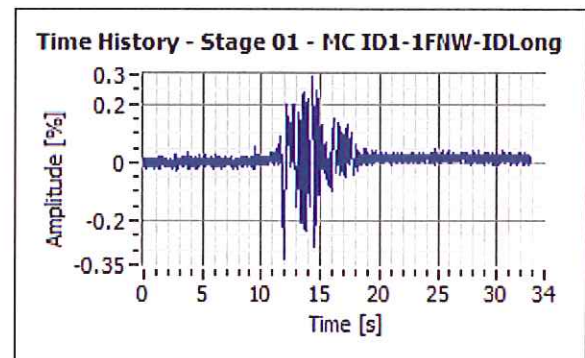


Figure K-22: Time History – Interstorey Drift ID1 – Stage 01 – Longitudinal direction

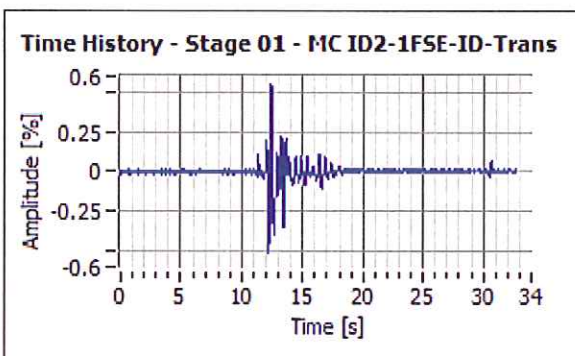


Figure K-23: Time History – Interstorey Drift ID2 – Stage 01 – Transversal direction

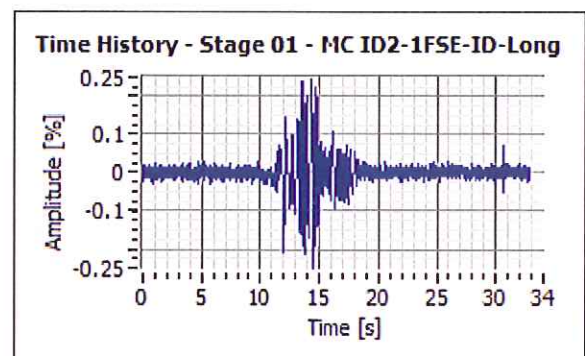


Figure K-24: Time History – Interstorey Drift ID2 – Stage 01 – Longitudinal direction

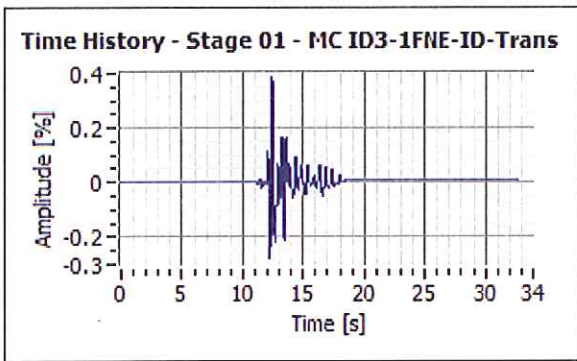


Figure K-25: Time History – Interstorey Drift ID3 – Stage 01 – Transversal direction

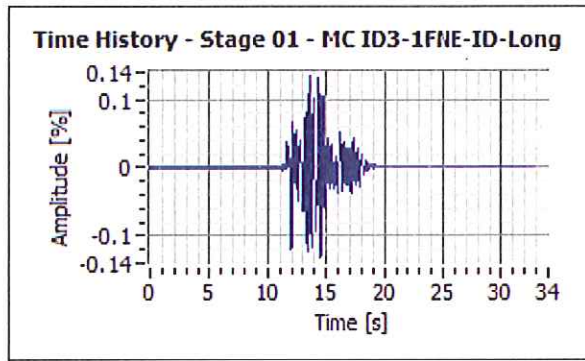


Figure K-26: Time History – Interstorey Drift ID3 – Stage 01 – Longitudinal direction

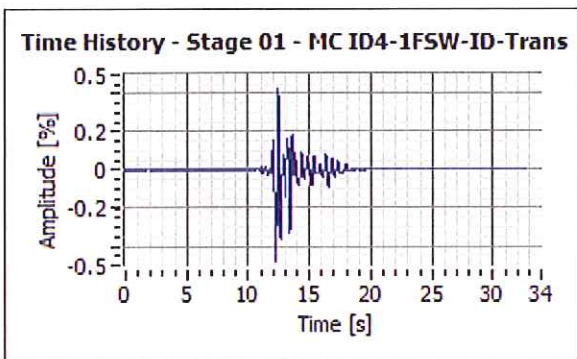


Figure K-27: Time History – Interstorey Drift ID4 – Stage 01 – Transversal direction

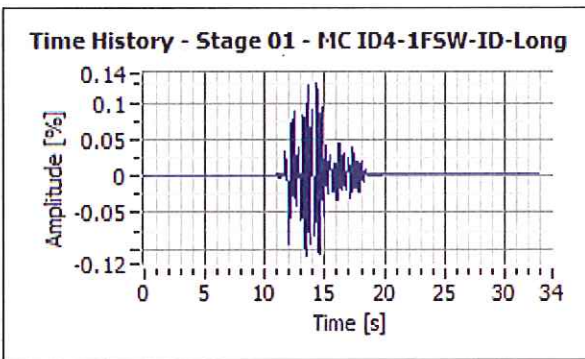


Figure K-28: Time History – Interstorey Drift ID4 – Stage 01 – Longitudinal direction

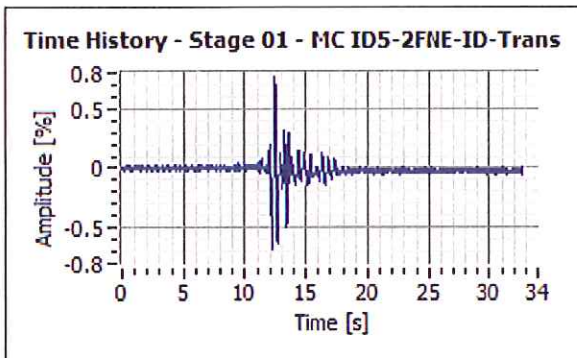


Figure K-29: Time History – Interstorey Drift ID5 – Stage 01 – Transversal direction

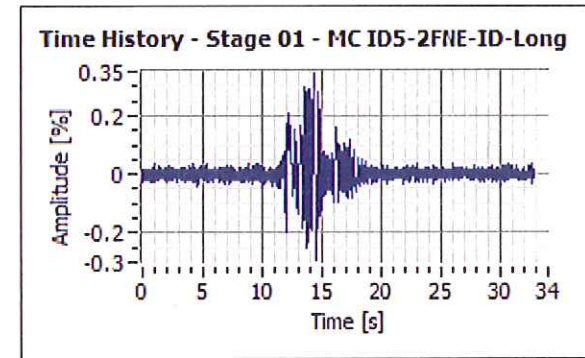


Figure K-30: Time History – Interstorey Drift ID5 – Stage 01 – Longitudinal direction

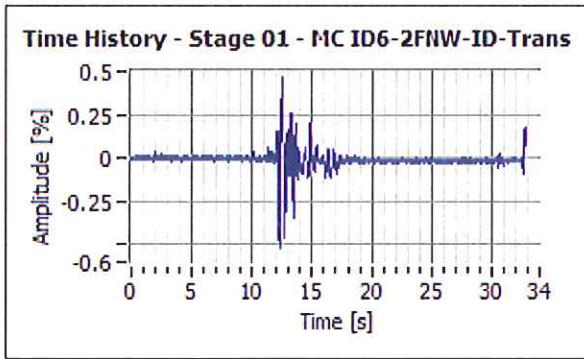


Figure K-31: Time History – Interstorey Drift ID6 – Stage 01 – Transversal direction

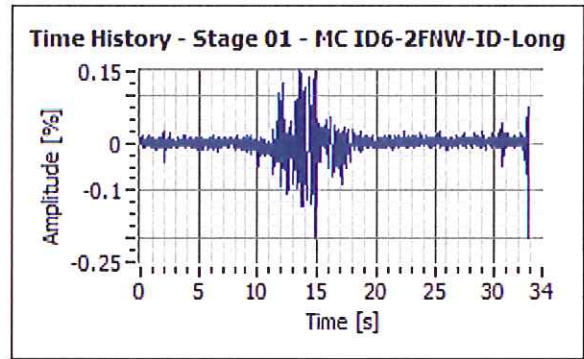


Figure K-32: Time History – Interstorey Drift ID6 – Stage 01 – Longitudinal direction

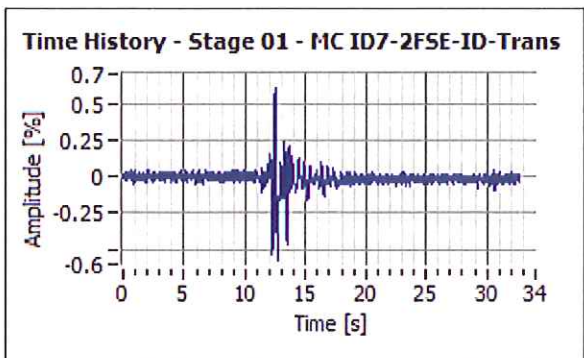


Figure K-33: Time History – Interstorey Drift ID7 – Stage 01 – Transversal direction

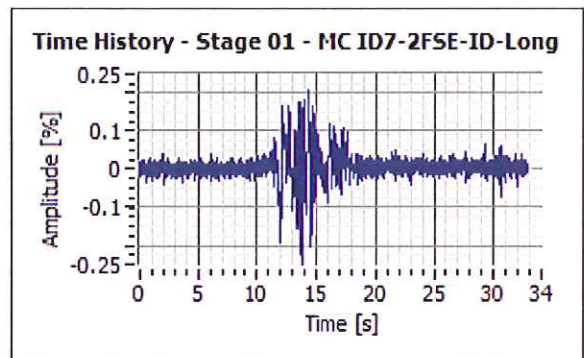


Figure K-34: Time History – Interstorey Drift ID7 – Stage 01 – Longitudinal direction

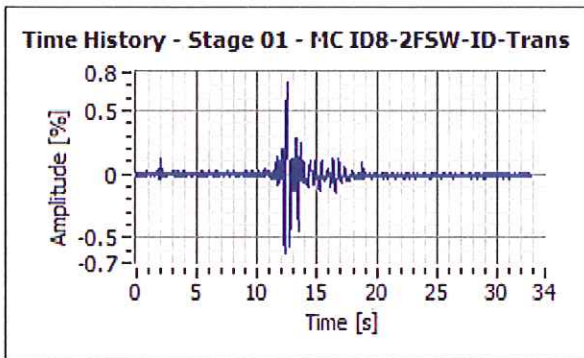


Figure K-35: Time History – Interstorey Drift ID8 – Stage 01 – Transversal direction

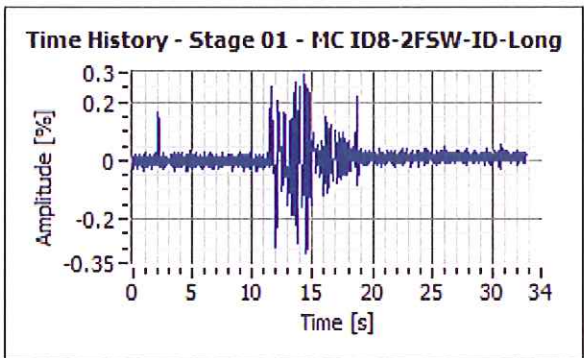


Figure K-36: Time History – Interstorey Drift ID8 – Stage 01 – Longitudinal direction

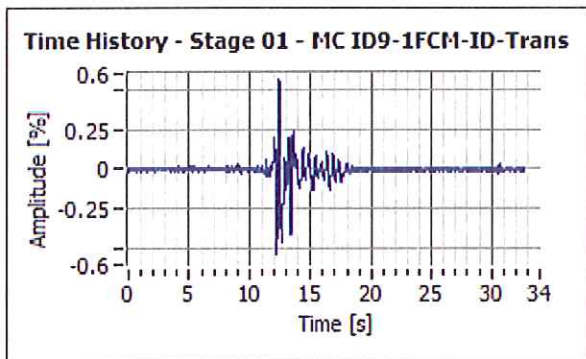


Figure K-37: Time History – Interstorey Drift ID9 – Stage 01 – Transversal direction

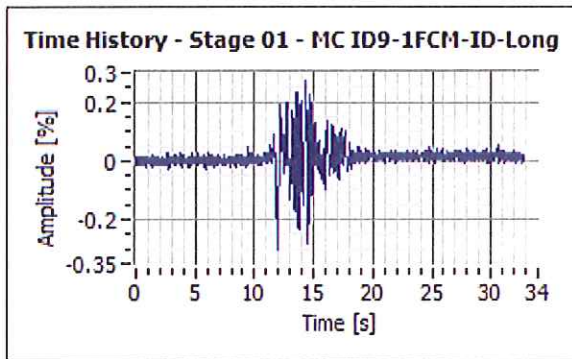


Figure K-38: Time History – Interstorey Drift ID9 – Stage 01 – Longitudinal direction

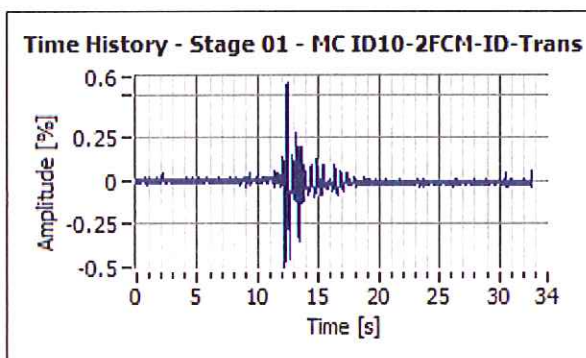


Figure K-39: Time History – Interstorey Drift ID10 – Stage 01 – Transversal direction

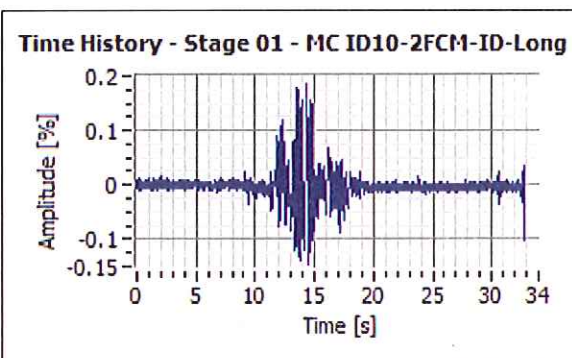


Figure K-40: Time History – Interstorey Drift ID10 – Stage 01 – Longitudinal direction

K.3 Stage 02

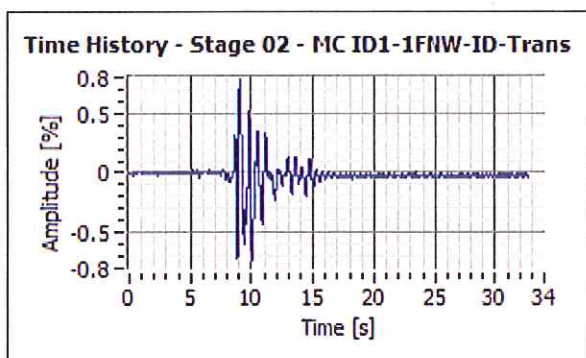


Figure K-41: Time History – Interstorey Drift ID1 – Stage 02 – Transversal direction

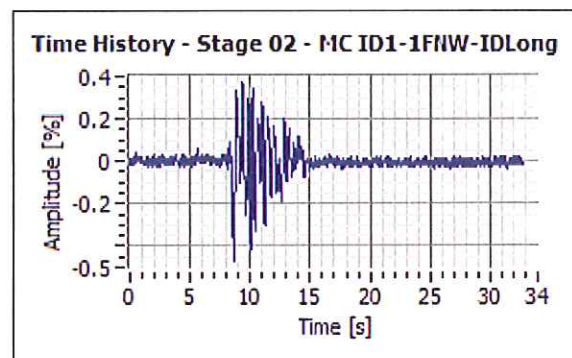


Figure K-42: Time History – Interstorey Drift ID1 – Stage 02 – Longitudinal direction

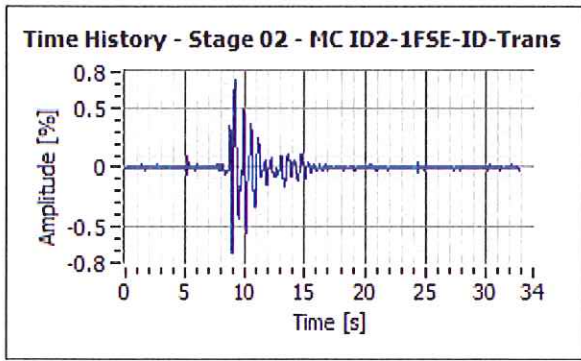


Figure K-43: Time History – Interstorey Drift ID2 – Stage 02 – Transversal direction

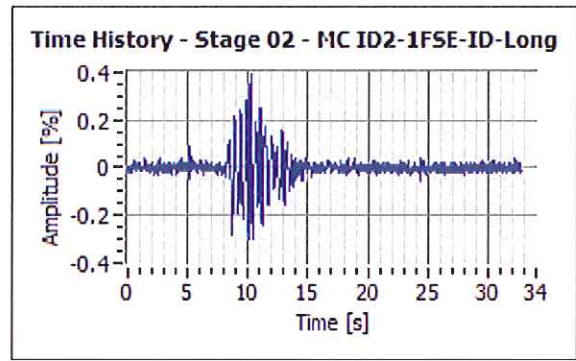


Figure K-44: Time History – Interstorey Drift ID2 – Stage 02 – Longitudinal direction

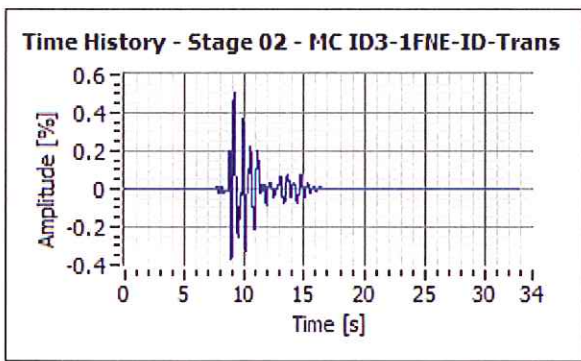


Figure K-45: Time History – Interstorey Drift ID3 – Stage 02 – Transversal direction

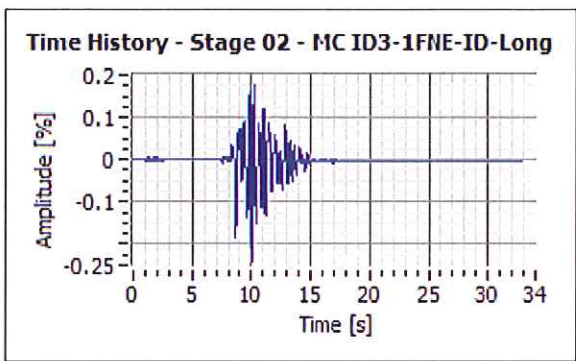


Figure K-46: Time History – Interstorey Drift ID3 – Stage 02 – Longitudinal direction

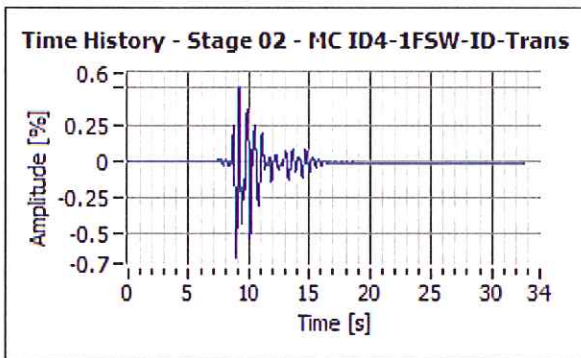


Figure K-47: Time History – Interstorey Drift ID4 – Stage 02 – Transversal direction

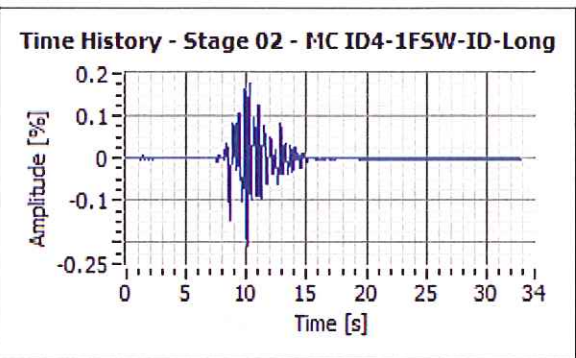


Figure K-48: Time History – Interstorey Drift ID4 – Stage 02 – Longitudinal direction

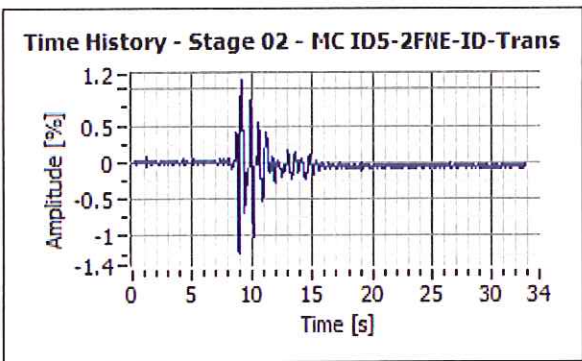


Figure K-49: Time History – Interstorey Drift ID5 – Stage 02 – Transversal direction

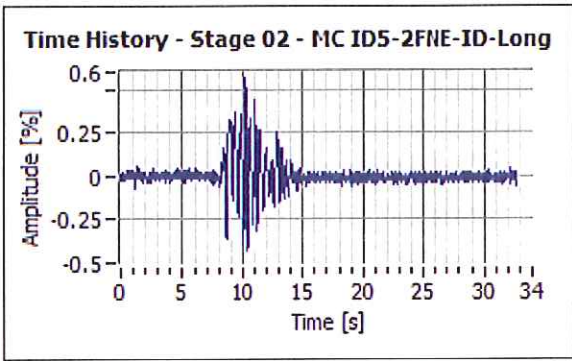


Figure K-50: Time History – Interstorey Drift ID5 – Stage 02 – Longitudinal direction

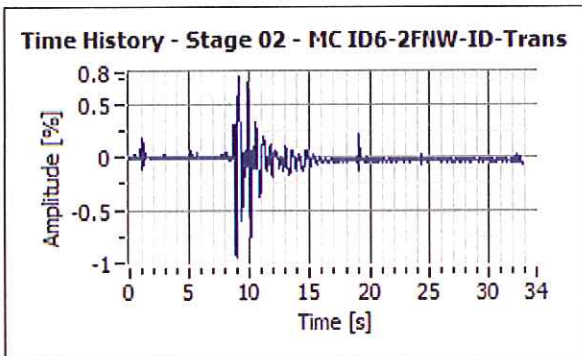


Figure K-51: Time History – Interstorey Drift ID6 – Stage 02 – Transversal direction

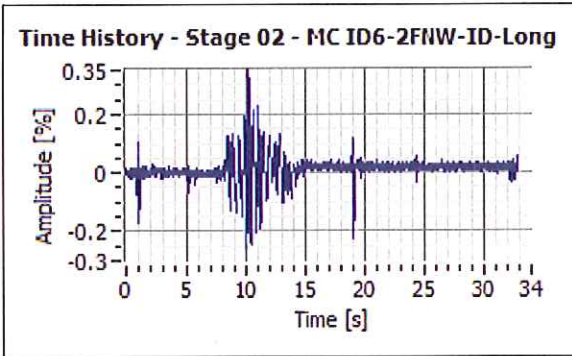


Figure K-52: Time History – Interstorey Drift ID6 – Stage 02 – Transversal direction

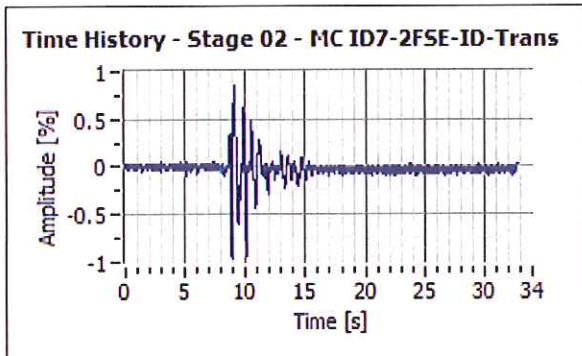


Figure K-53: Time History – Interstorey Drift ID7 – Stage 02 – Transversal direction

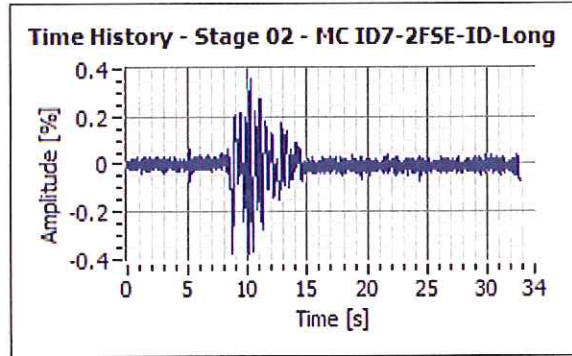


Figure K-54: Time History – Interstorey Drift ID7 – Stage 02 – Longitudinal direction

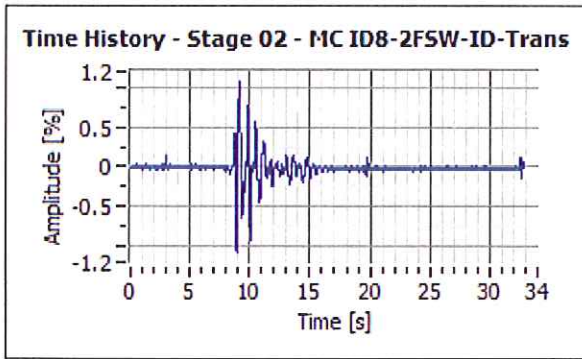


Figure K-55: Time History – Interstorey Drift ID8 – Stage 02 – Transversal direction

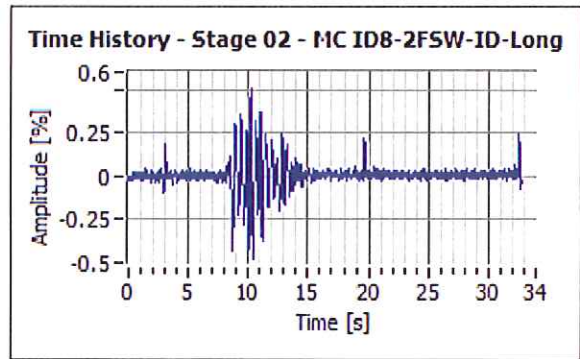


Figure K-56: Time History – Interstorey Drift ID8 – Stage 02 – Longitudinal direction

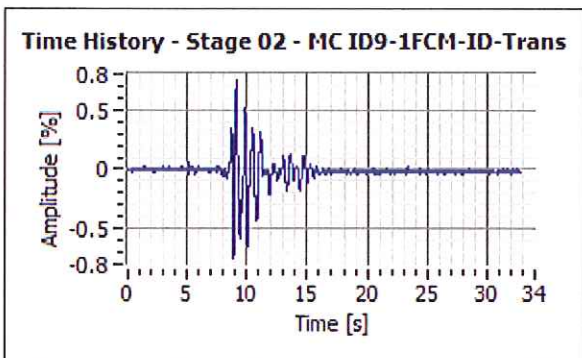


Figure K-57: Time History – Interstorey Drift ID9 – Stage 02 – Transversal direction

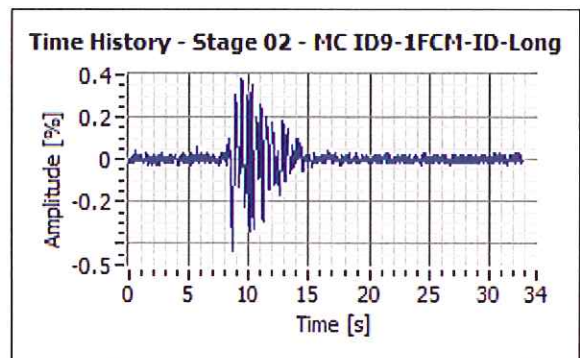


Figure K-58: Time History – Interstorey Drift ID9 – Stage 02 – Longitudinal direction

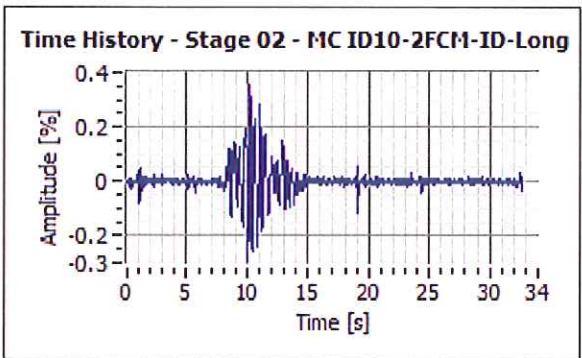


Figure K-59: Time History – Interstorey Drift ID10 – Stage 02 – Transversal direction

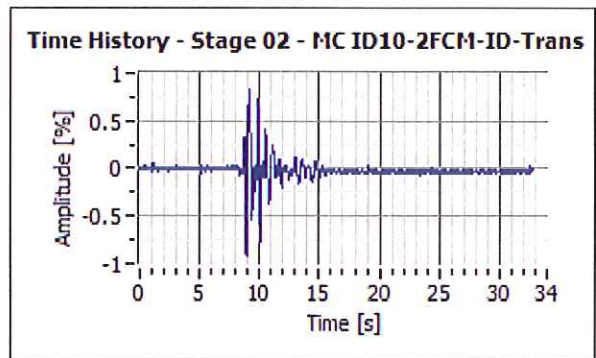


Figure K-60: Time History – Interstorey Drift ID10 – Stage 02 – Longitudinal direction

K.4 Stage 03

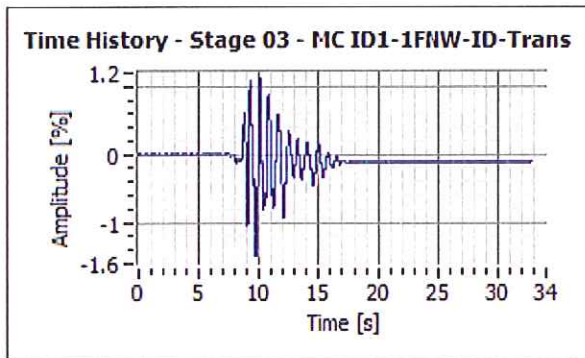


Figure K-61: Time History – Interstorey Drift ID1 – Stage 03 – Transversal direction

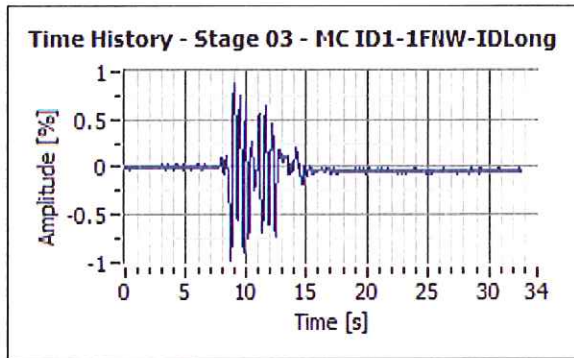


Figure K-62: Time History – Interstorey Drift ID1 – Stage 03 – Longitudinal direction

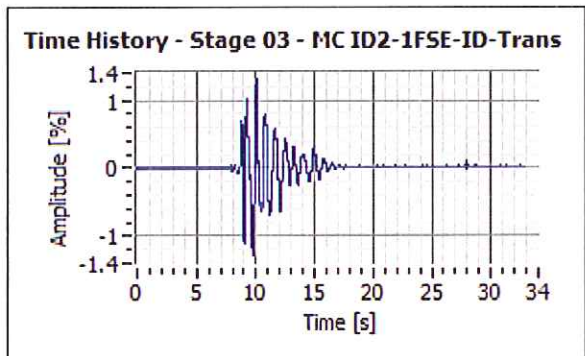


Figure K-63: Time History – Interstorey Drift ID2 – Stage 03 – Transversal direction

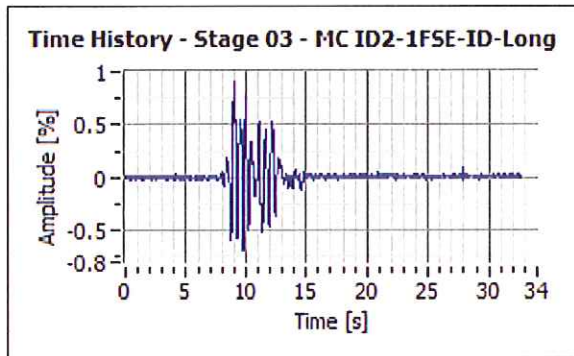


Figure K-64: Time History – Interstorey Drift ID2 – Stage 03 – Longitudinal direction

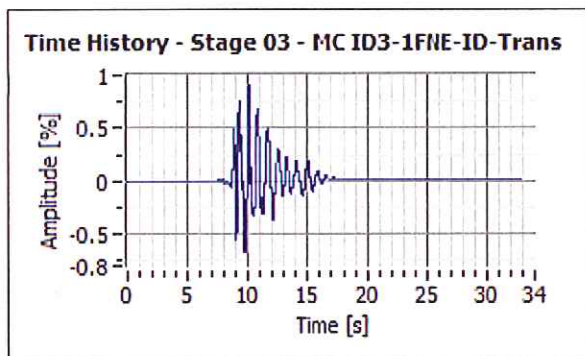


Figure K-65: Time History – Interstorey Drift ID3 – Stage 03 – Transversal direction

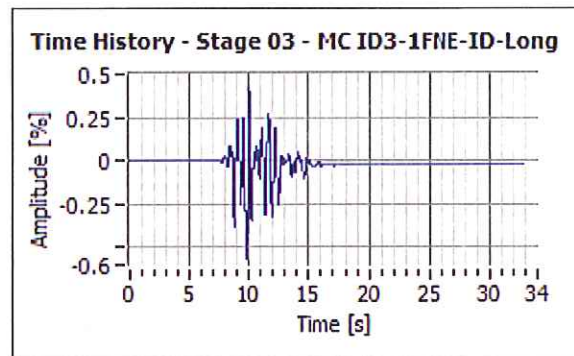


Figure K-66: Time History – Interstorey Drift ID3 – Stage 03 – Longitudinal direction

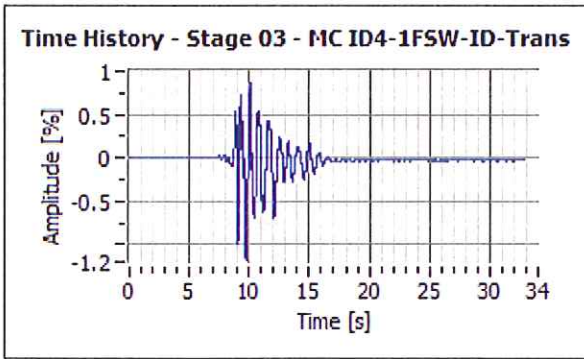


Figure K-67: Time History – Interstorey Drift ID4 – Stage 03 – Transversal direction

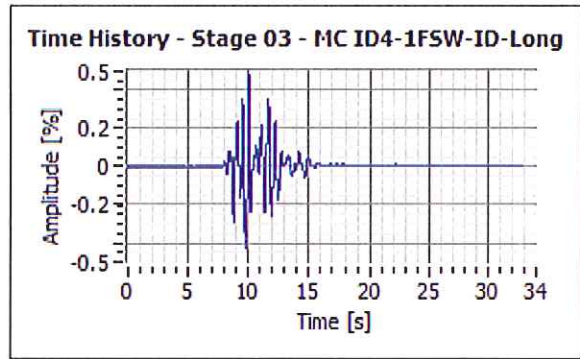


Figure K-68: Time History – Interstorey Drift ID4 – Stage 03 – Longitudinal direction

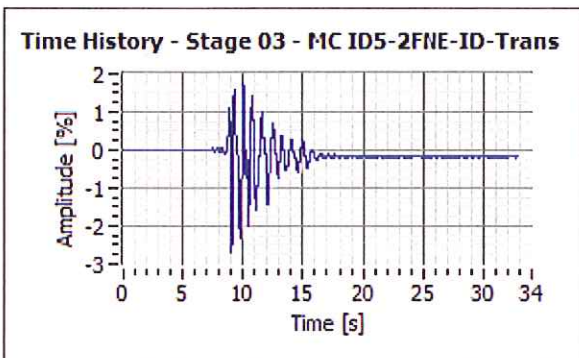


Figure K-69: Time History – Interstorey Drift ID5 – Stage 03 – Transversal direction

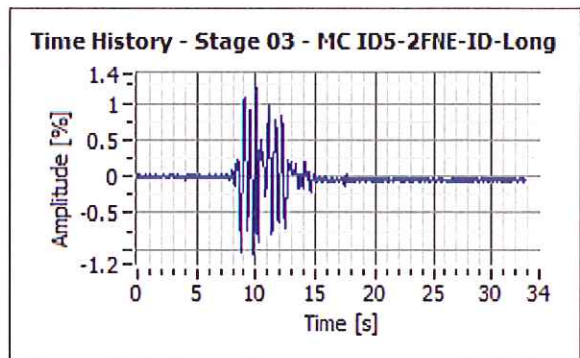


Figure K-70: Time History – Interstorey Drift ID5 – Stage 03 – Longitudinal direction

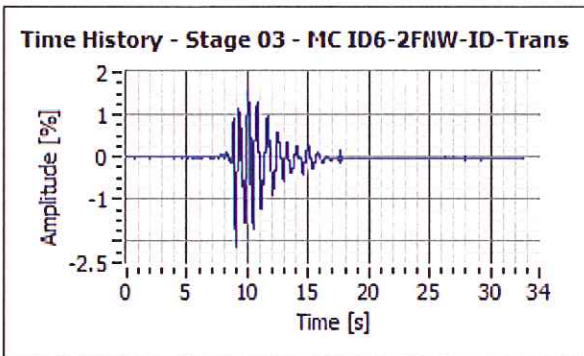


Figure K-71: Time History – Interstorey Drift ID6 – Stage 03 – Transversal direction

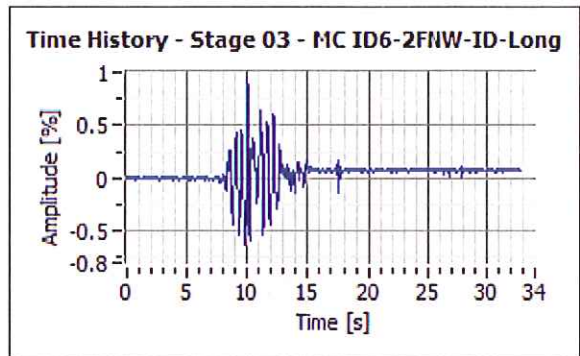


Figure K-72: Time History – Interstorey Drift ID6 – Stage 03 – Longitudinal direction

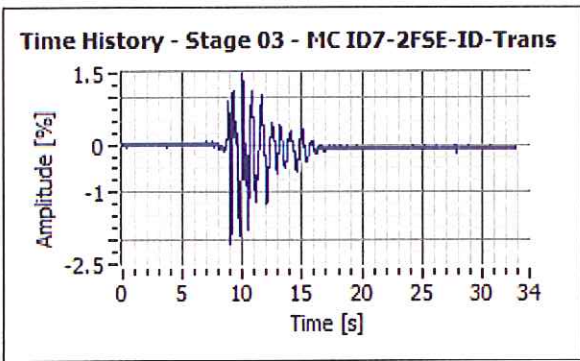


Figure K-73: Time History – Interstorey Drift ID7 – Stage 03 – Transversal direction

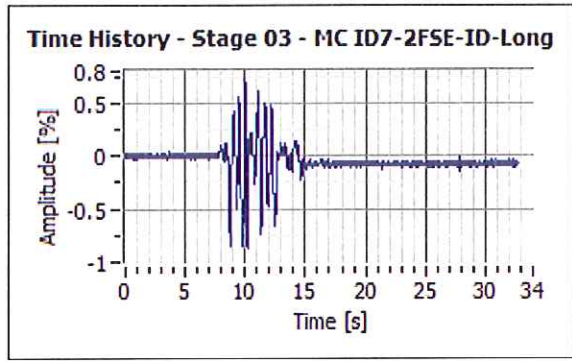


Figure K-74: Time History – Interstorey Drift ID7 – Stage 03 – Longitudinal direction

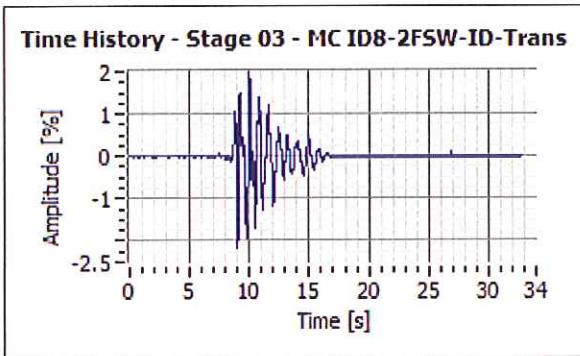


Figure K-75: Time History – Interstorey Drift ID8 – Stage 03 – Transversal direction

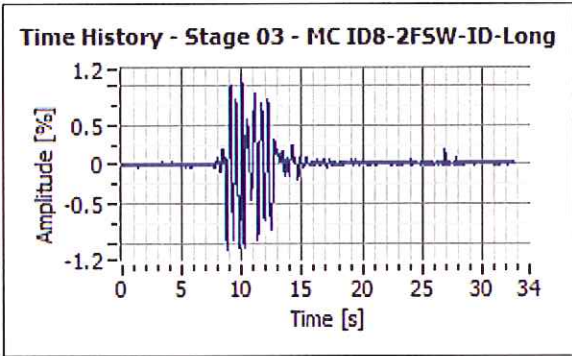


Figure K-76: Time History – Interstorey Drift ID8 – Stage 03 – Longitudinal direction

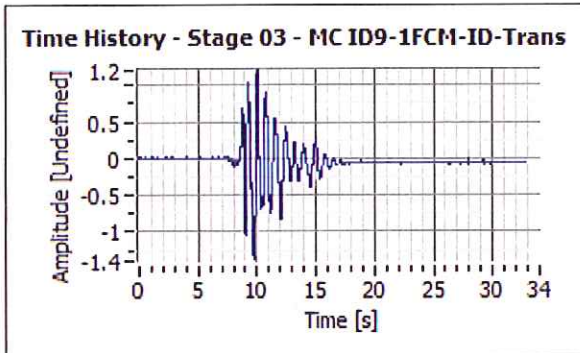


Figure K-77: Time History – Interstorey Drift ID9 – Stage 03 – Transversal direction

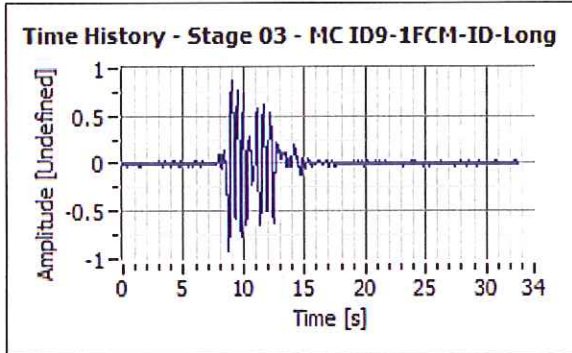


Figure K-78: Time History – Interstorey Drift ID9 – Stage 03 – Longitudinal direction

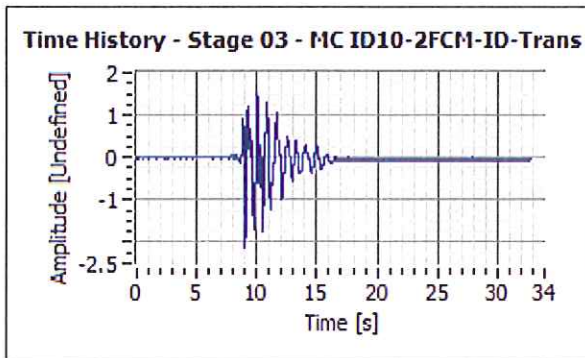


Figure K-79: Time History – Interstorey Drift ID10 – Stage 03 – Transversal direction

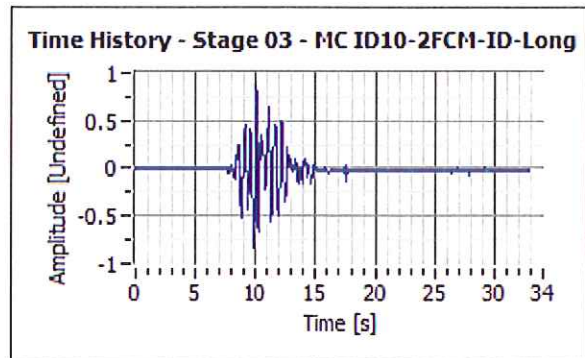


Figure K-80: Time History – Interstorey Drift ID10 – Stage 03 – Longitudinal direction

K.5 Stage 04

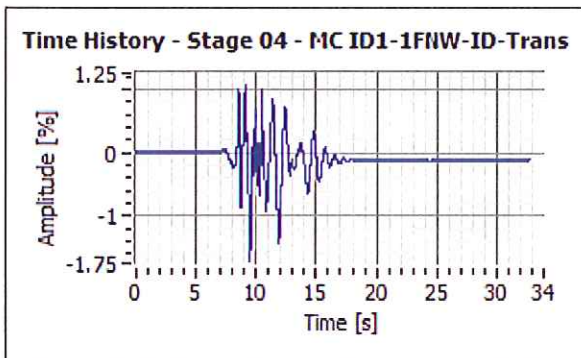


Figure K-81: Time History – Interstorey Drift ID1 – Stage 04 – Transversal direction

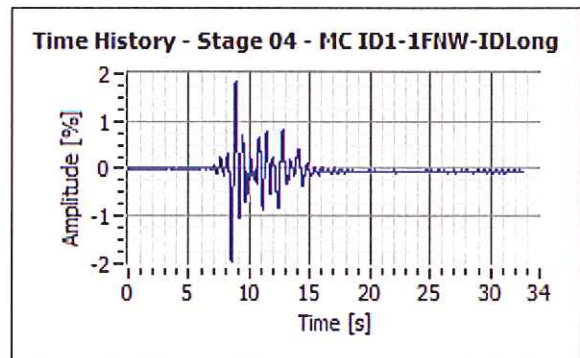


Figure K-82: Time History – Interstorey Drift ID1 – Stage 04 – Longitudinal direction

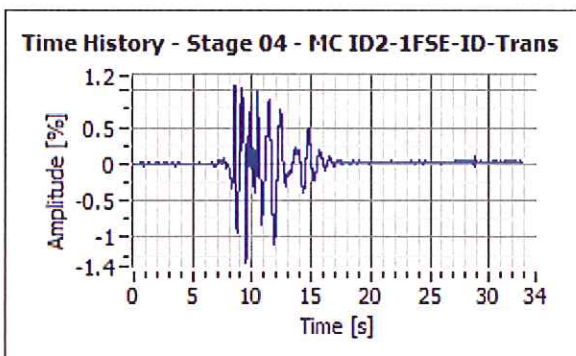


Figure K-83: Time History – Interstorey Drift ID2 – Stage 04 – Transversal direction

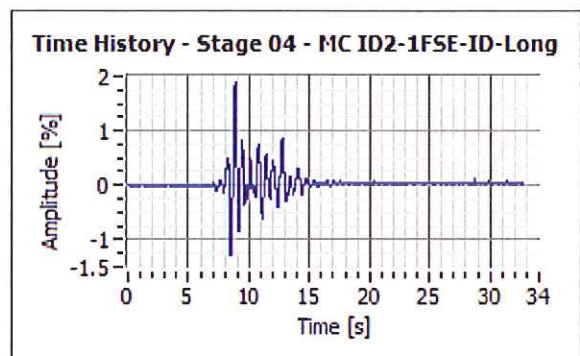


Figure K-84: Time History – Interstorey Drift ID2 – Stage 04 – Longitudinal direction

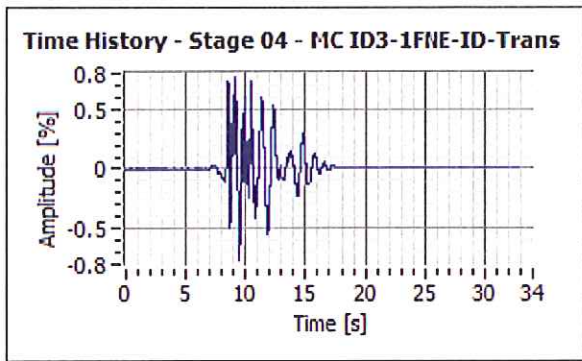


Figure K-85: Time History – Interstorey Drift ID3 – Stage 04 – Transversal direction

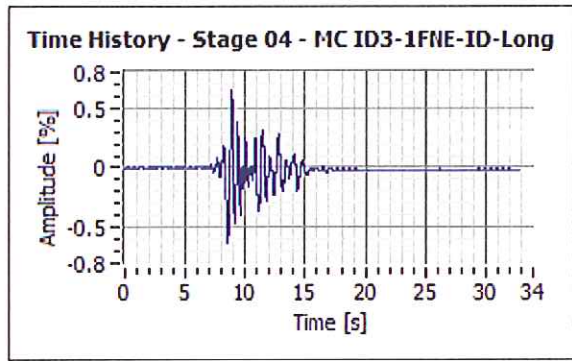


Figure K-86: Time History – Interstorey Drift ID3 – Stage 04 – Transversal direction

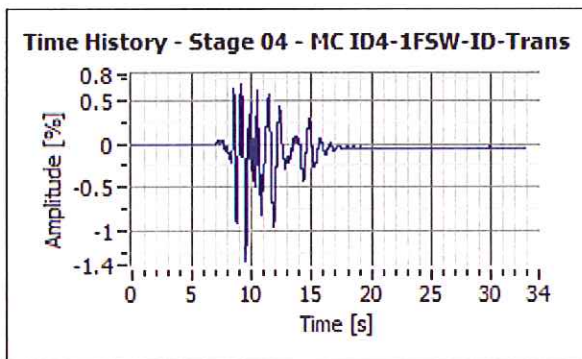


Figure K-87: Time History – Interstorey Drift ID4 – Stage 04 – Transversal direction

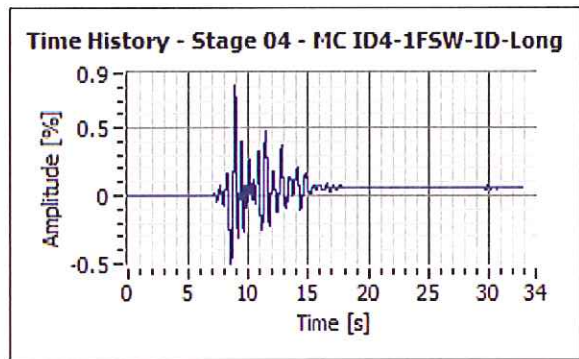


Figure K-88: Time History – Interstorey Drift ID4 – Stage 04 – Longitudinal direction

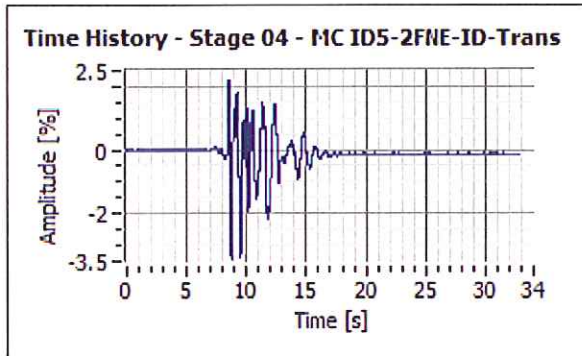


Figure K-89: Time History – Interstorey Drift ID5 – Stage 04 – Transversal direction

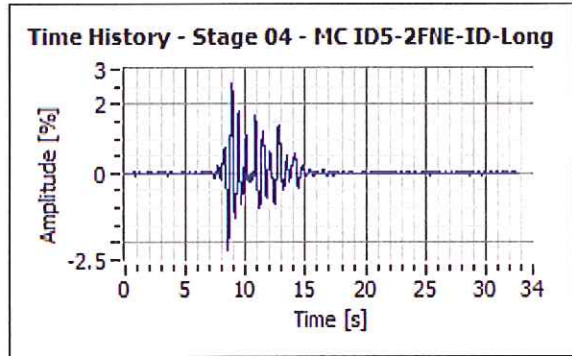


Figure K-90: Time History – Interstorey Drift ID5 – Stage 04 – Longitudinal direction

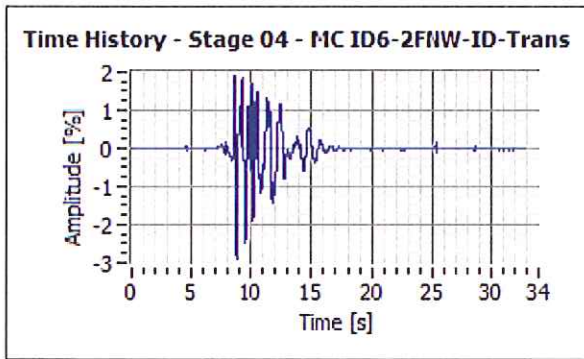


Figure K-91: Time History – Interstorey Drift ID6 – Stage 04 – Transversal direction

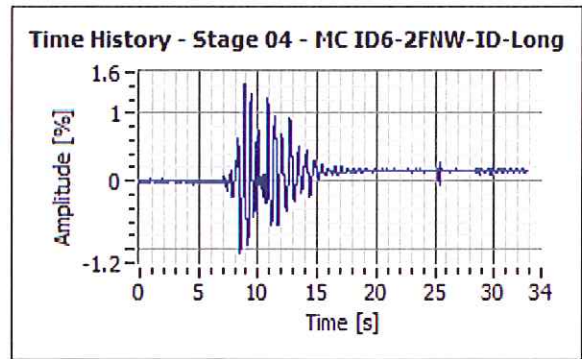


Figure K-92: Time History – Interstorey Drift ID6 – Stage 04 – Longitudinal direction

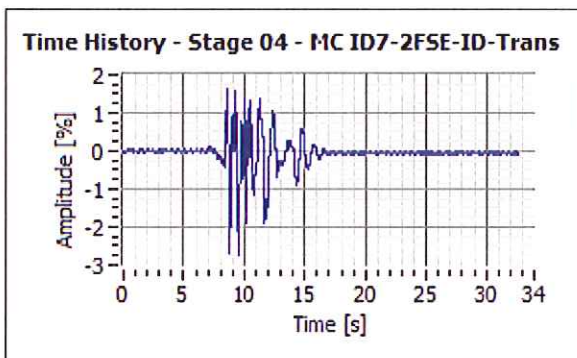


Figure K-93: Time History – Interstorey Drift ID7 – Stage 04 – Transversal direction

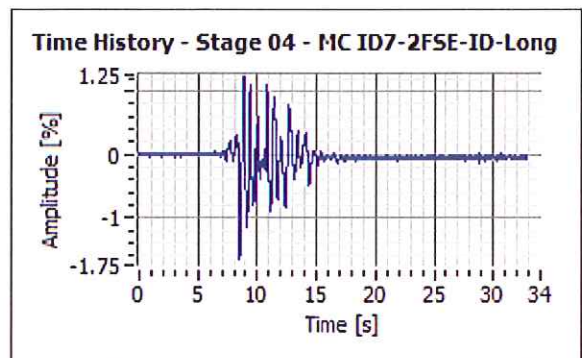


Figure K-94: Time History – Interstorey Drift ID7 – Stage 04 – Longitudinal direction

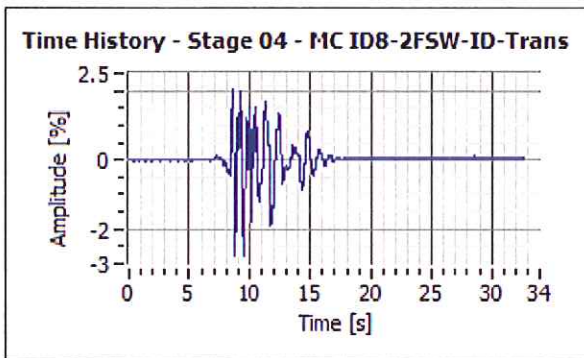


Figure K-95: Time History – Interstorey Drift ID8 – Stage 04 – Transversal direction

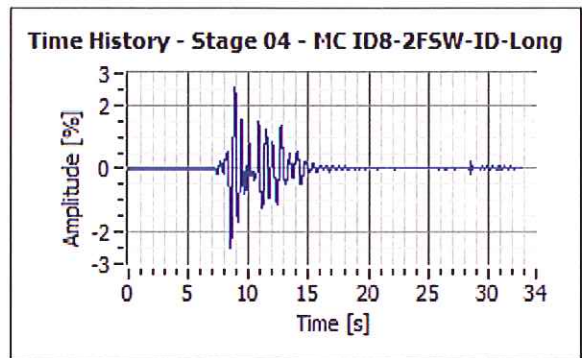


Figure K-96: Time History – Interstorey Drift ID8 – Stage 04 – Longitudinal direction

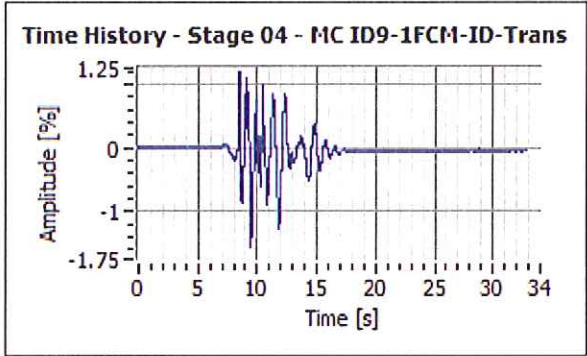


Figure K-97: Time History – Interstorey Drift ID9 – Stage 04 – Transversal direction

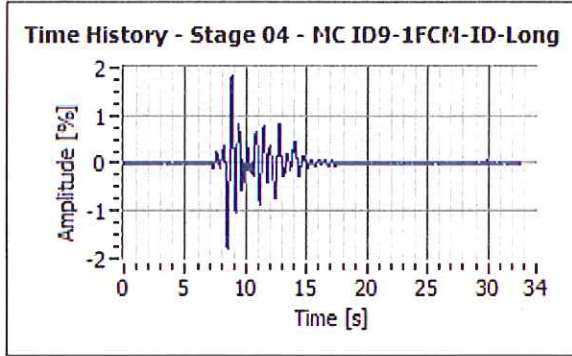


Figure K-98: Time History – Interstorey Drift ID9 – Stage 04 – Longitudinal direction

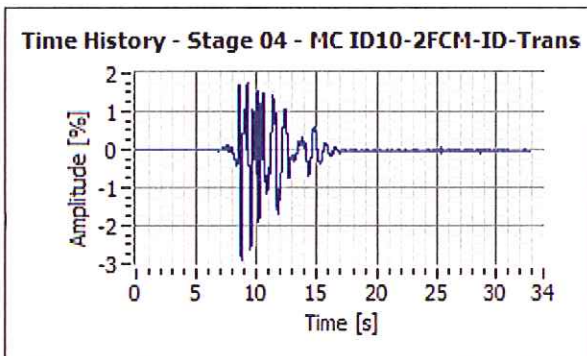


Figure K-99: Time History – Interstorey Drift ID10 – Stage 04 – Transversal direction

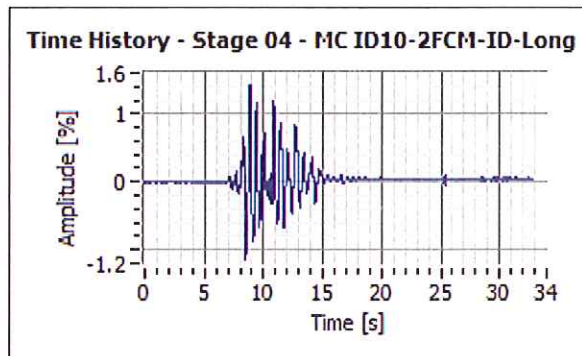
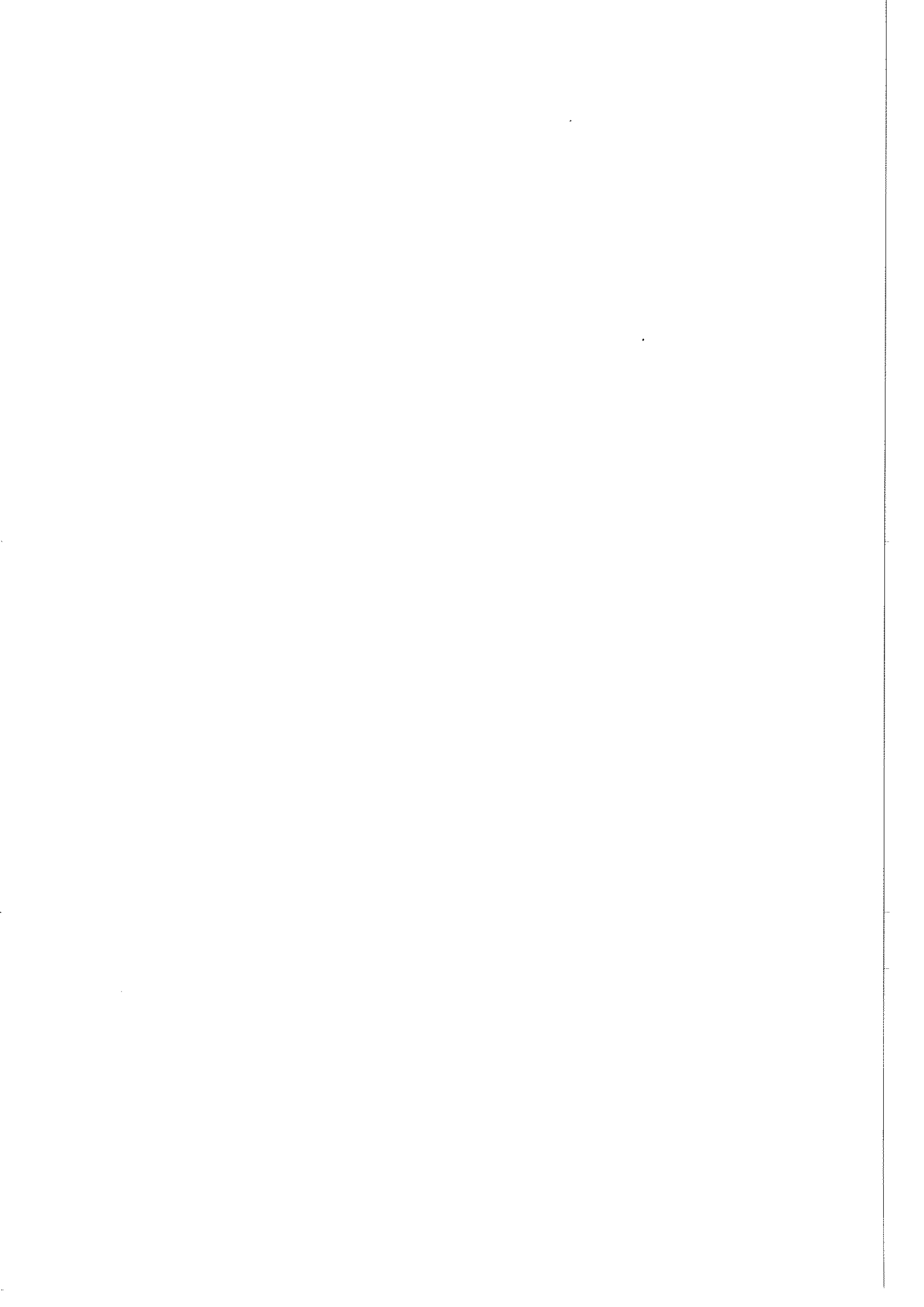


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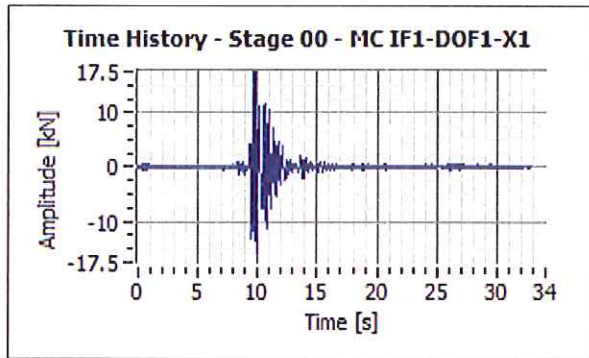


Figure L-1: Time History –Inertia Force DOF1 – Transversal Direction – Stage 00

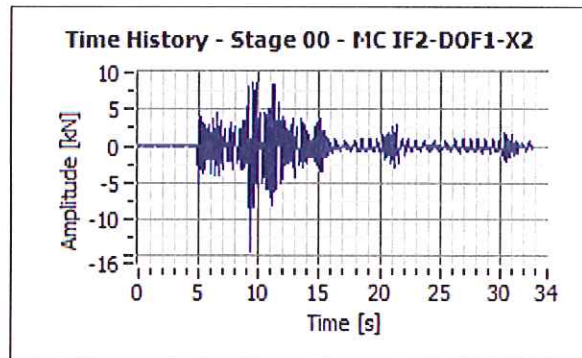


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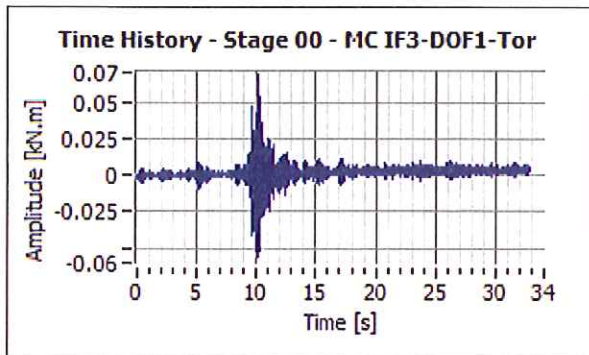


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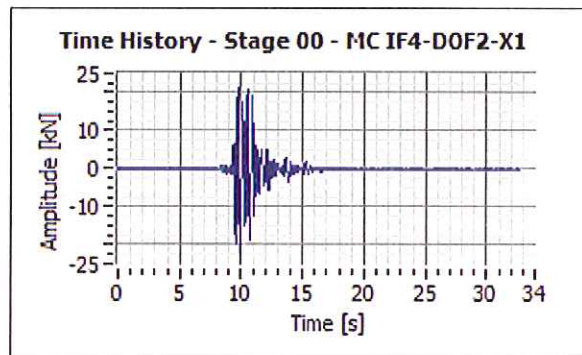


Figure L-4: Time History –Inertia Force DOF2 – Transversal Direction – Stage 00

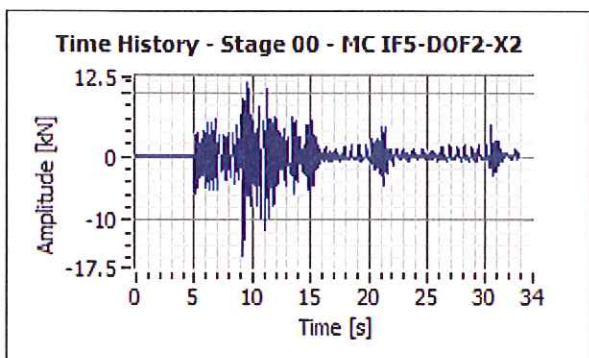


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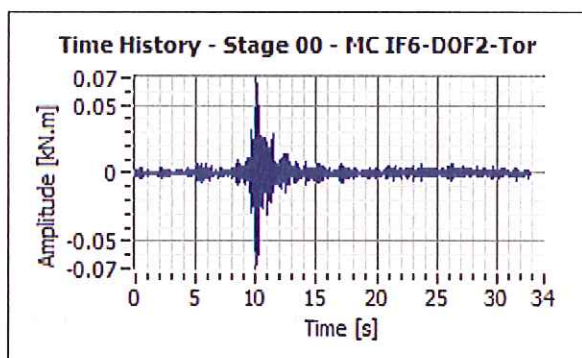


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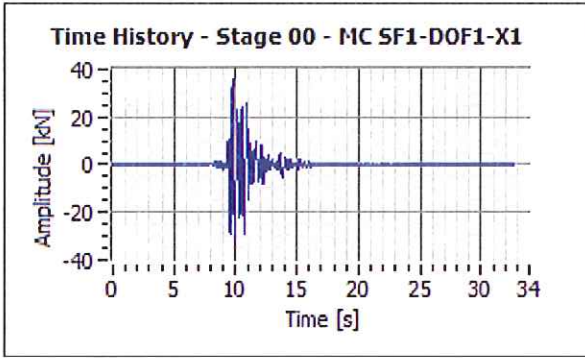


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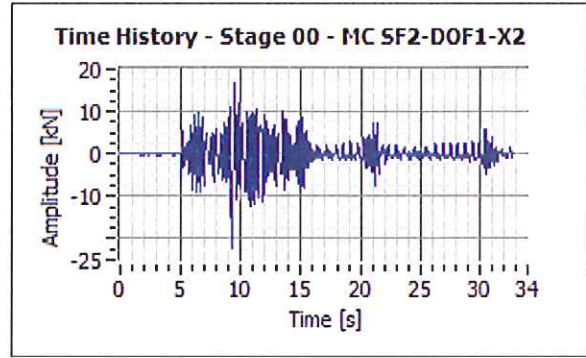


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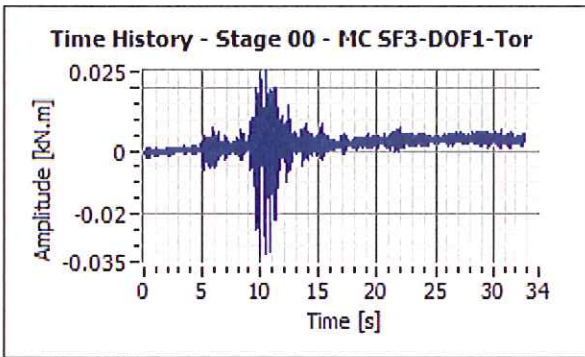


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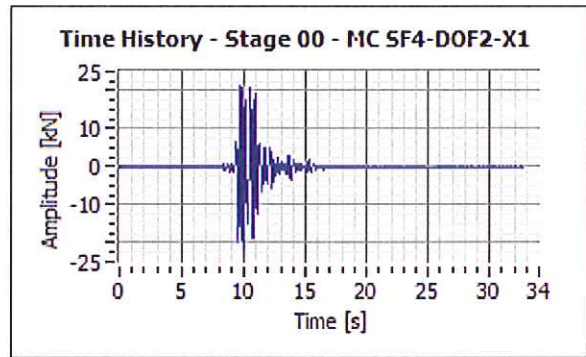


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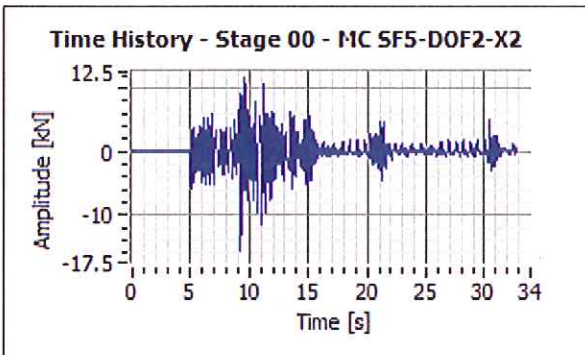


Figure L-11: Time History –Story Force DOF2 – Longitudinal Direction – Stage 00

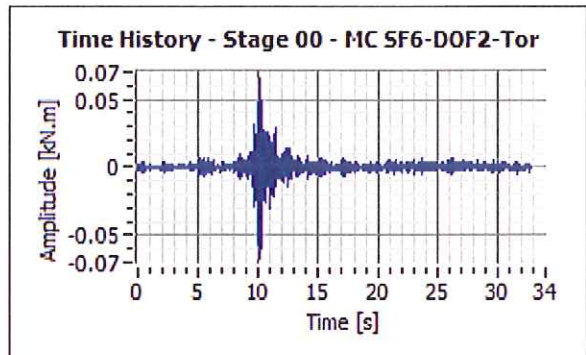


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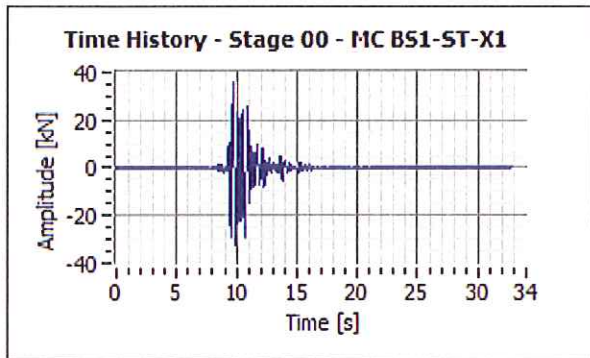


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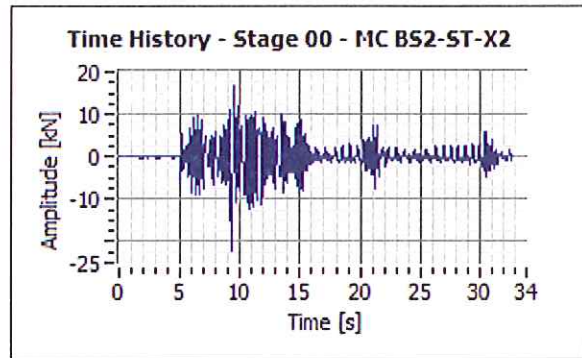


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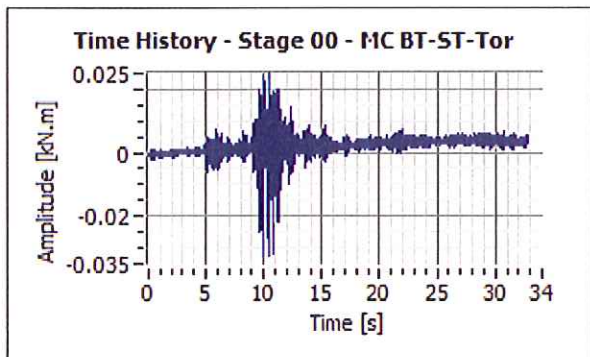


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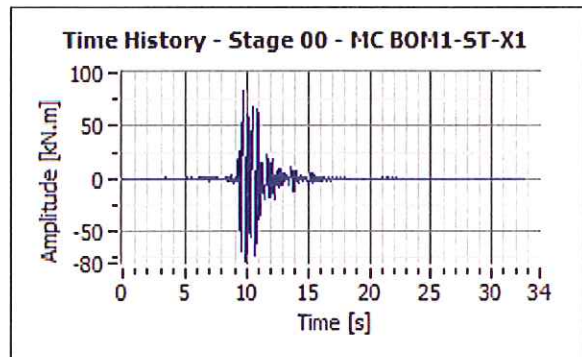


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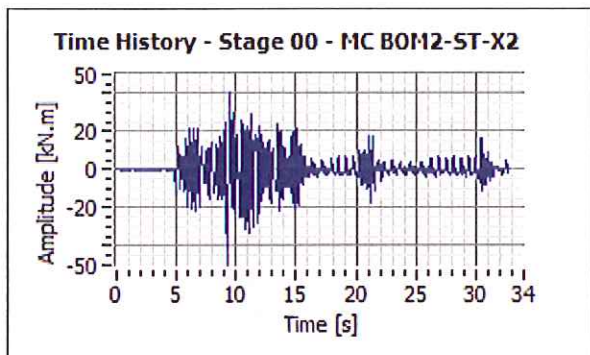


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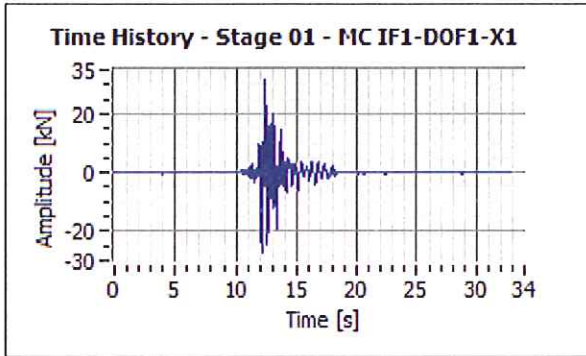


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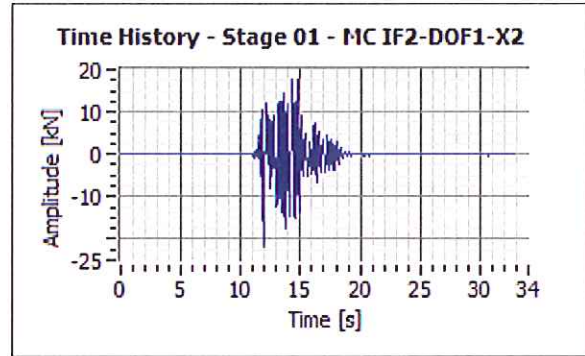


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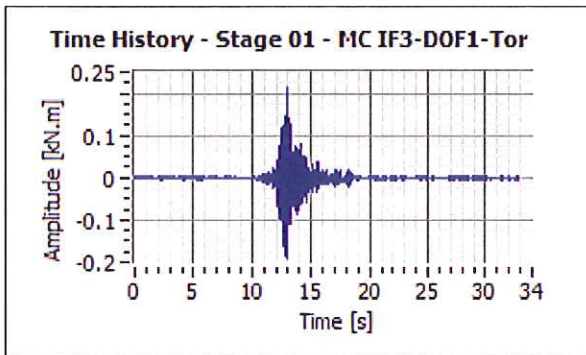


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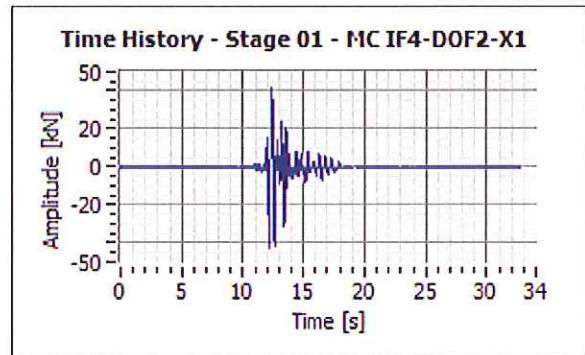


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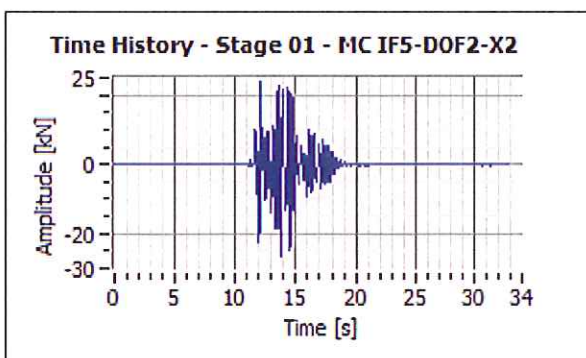


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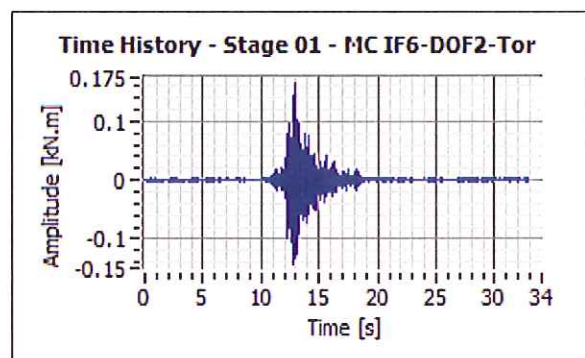


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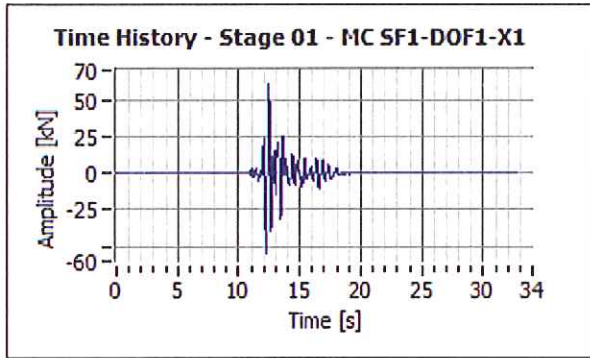


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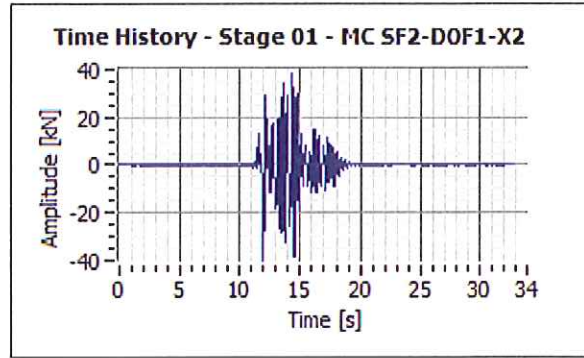


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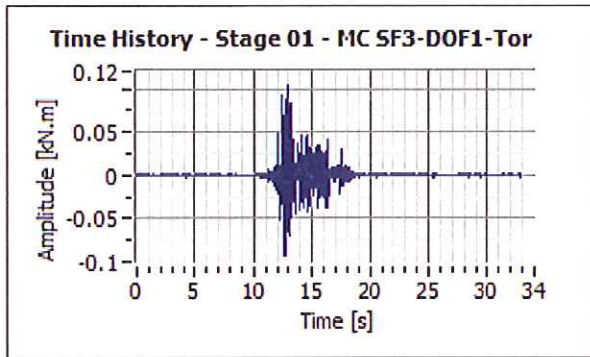


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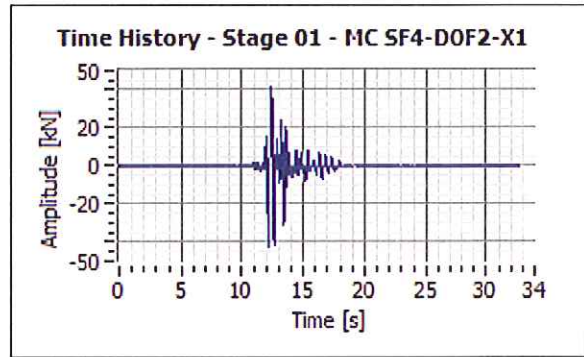


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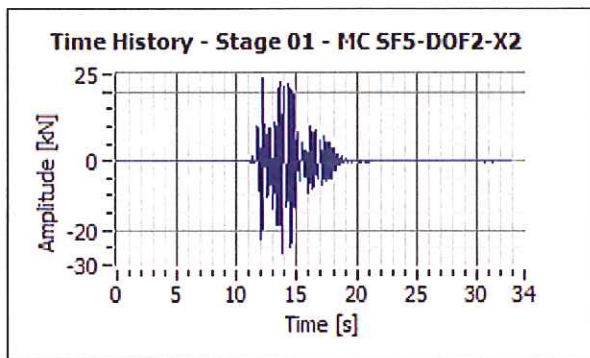


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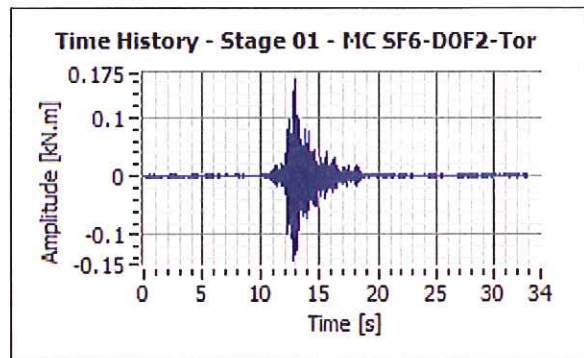


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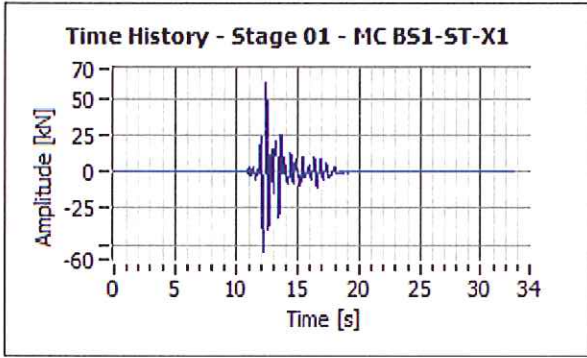


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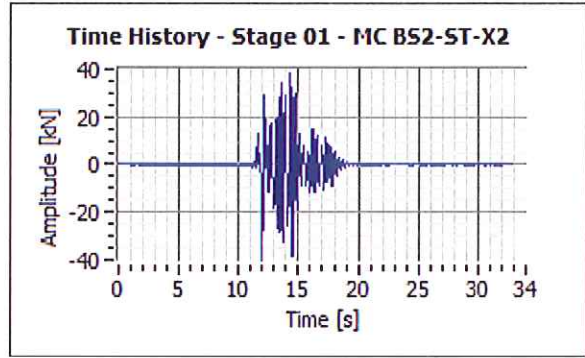


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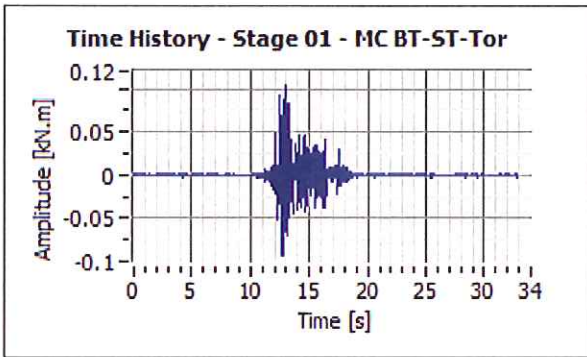


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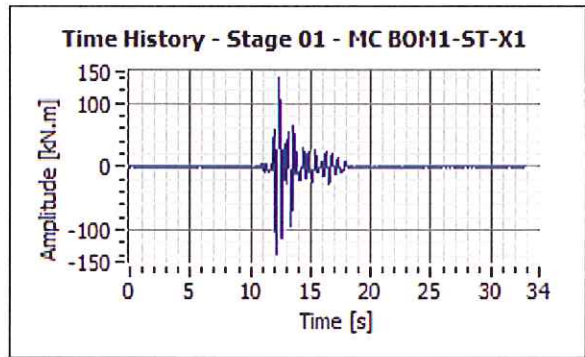


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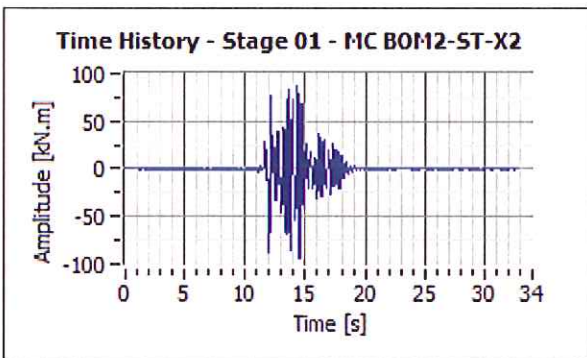


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L.3 Stage 02

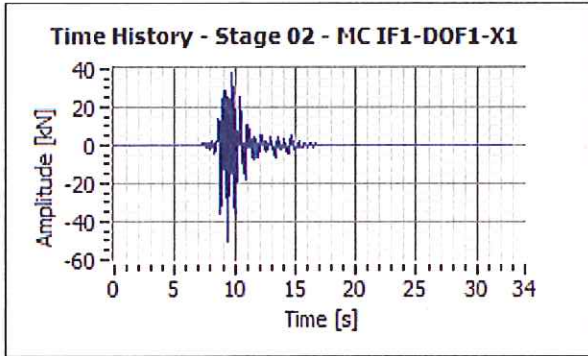


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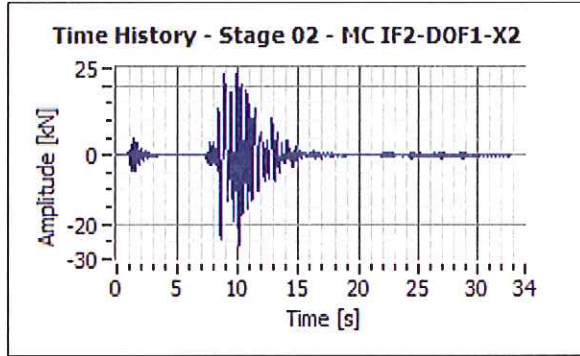


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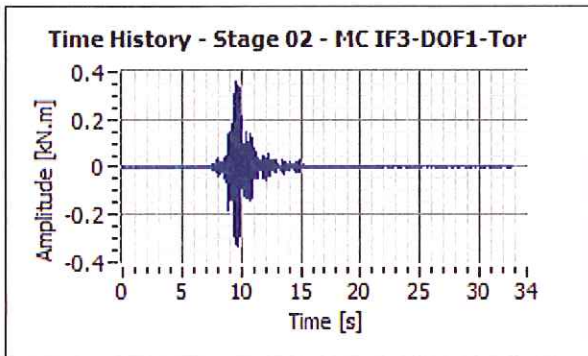


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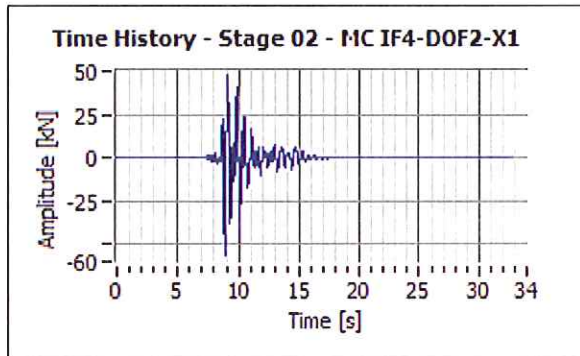


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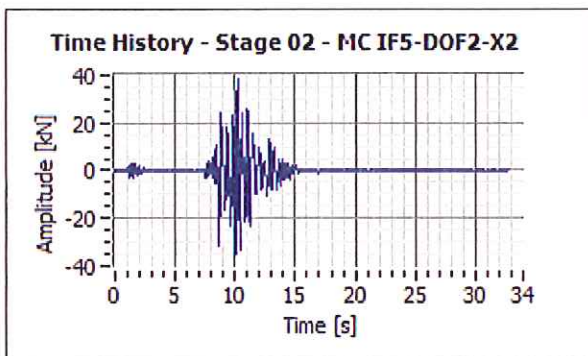


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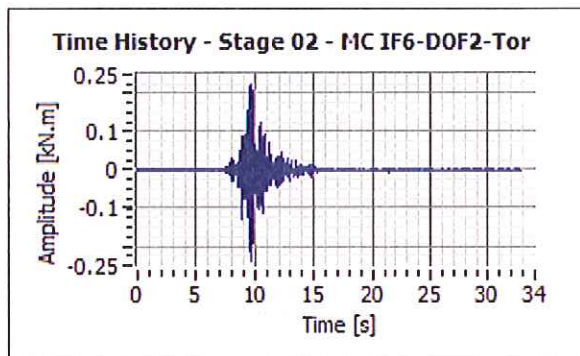


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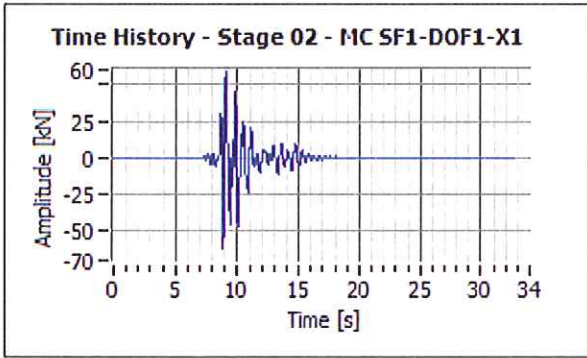


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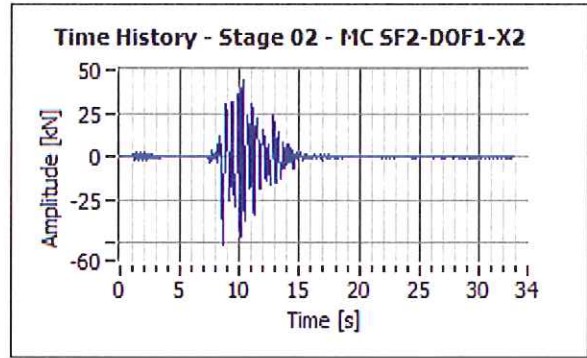


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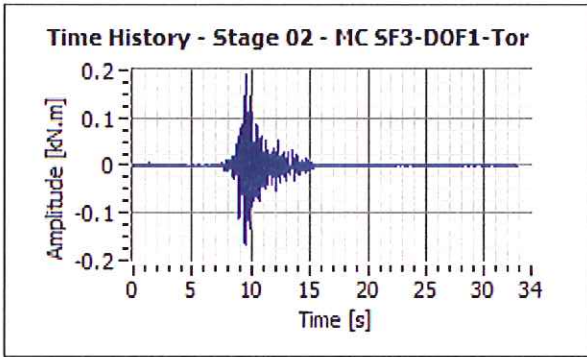


Figure L-43: Time History –Story Torque DOF1 – Stage 02

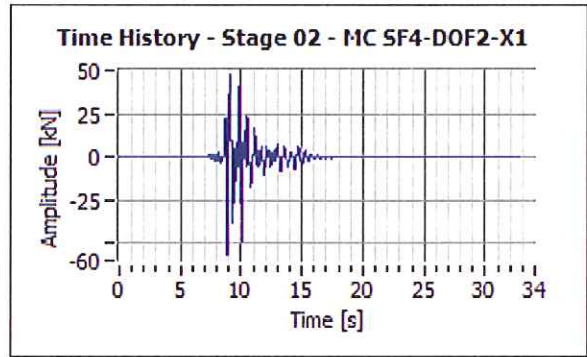


Figure L-44: Time History –Story Force DOF2 – Transversal Direction – Stage 02

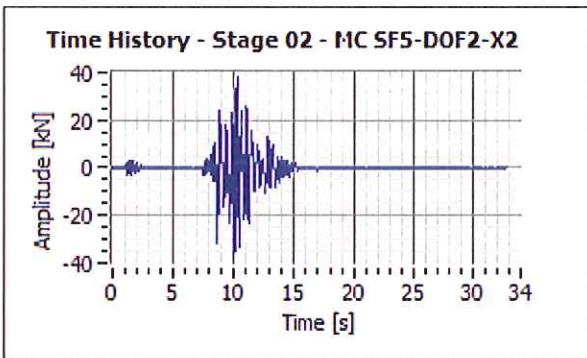


Figure L-45: Time History –Story Force DOF2 – Longitudinal Direction – Stage 02

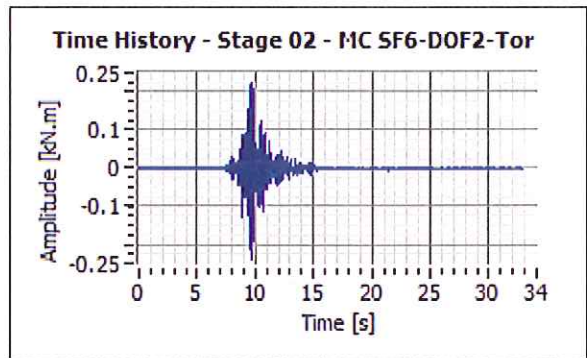


Figure L-46: Time History –Story Torque DOF1 – Stage 02

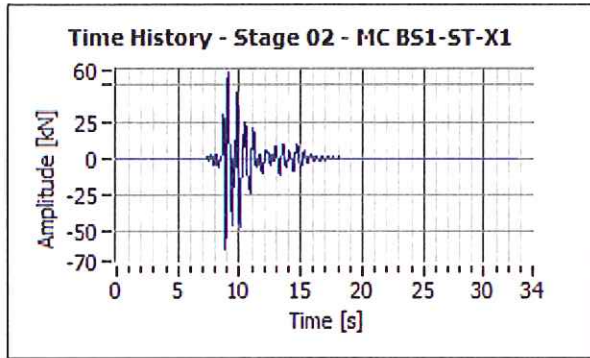


Figure L-47: Time History –Base Shear –
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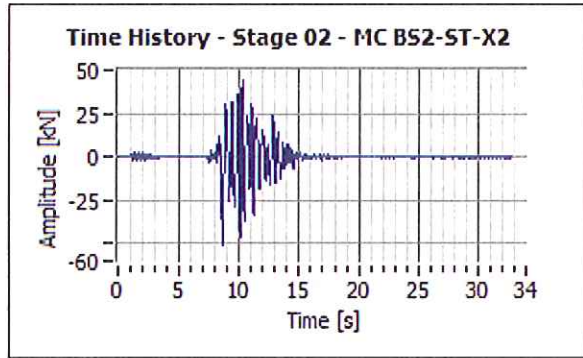


Figure L-48: Time History –Base Shear –
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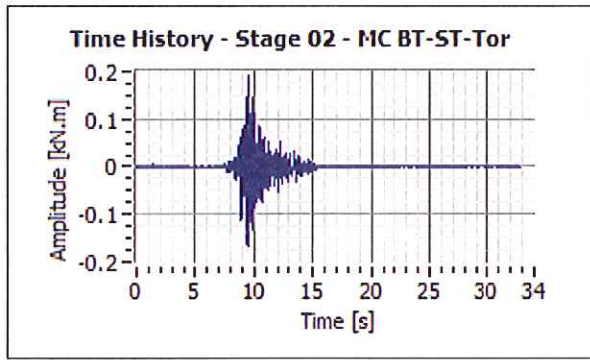


Figure L-49: Time History –Base Torque – Stage 02

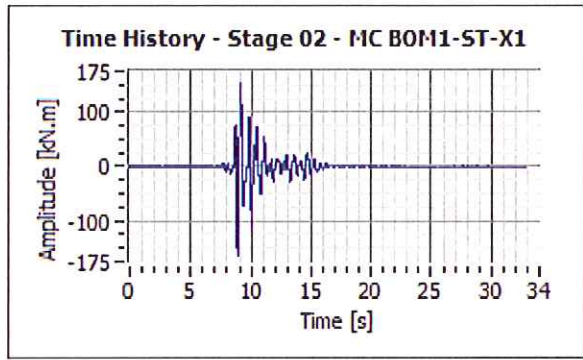


Figure L-50: Time History –Base Overturning
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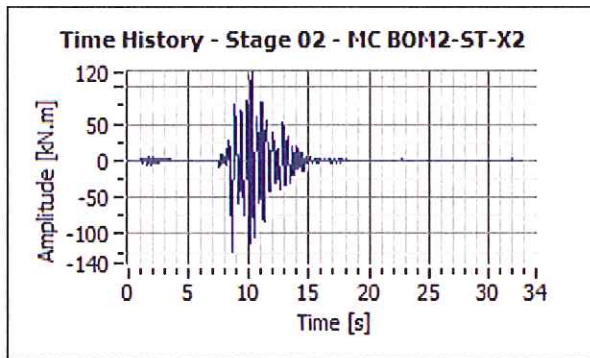


Figure L-51: Time History –Base Overturning
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L.4 Stage 03

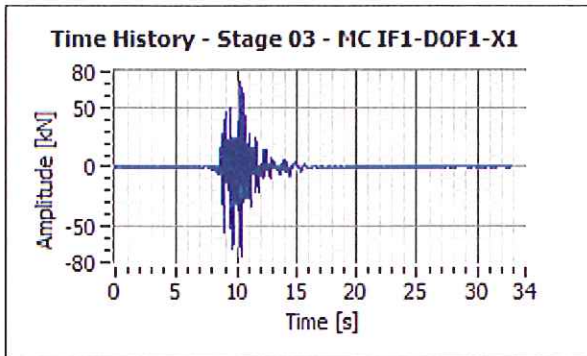


Figure L-52: Time History –Inertia Force DOF1 –
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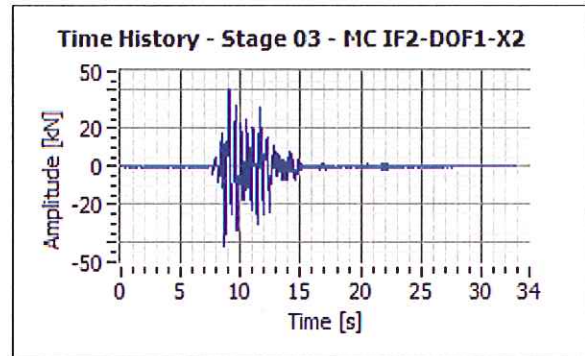


Figure L-53: Time History –Inertia Force DOF1 –
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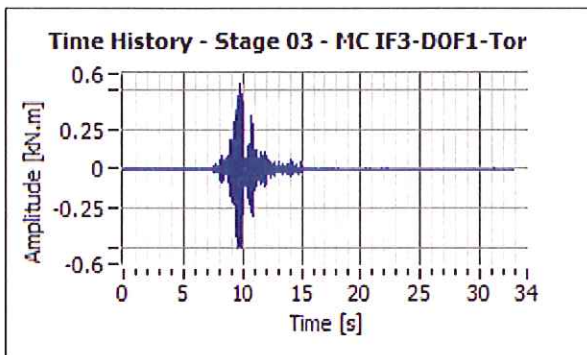


Figure L-54: Time History –Inertia Torque DOF1 –
Stage 03

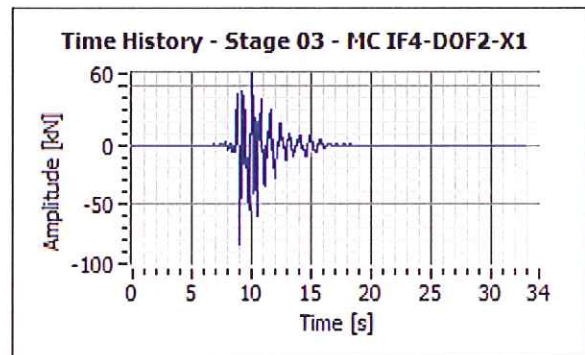


Figure L-55: Time History –Inertia Force DOF2 –
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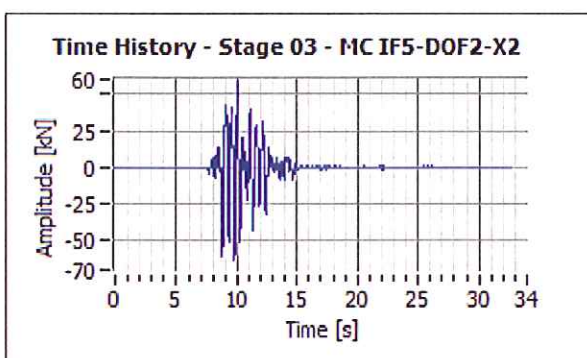


Figure L-56: Time History –Inertia Force DOF2 –
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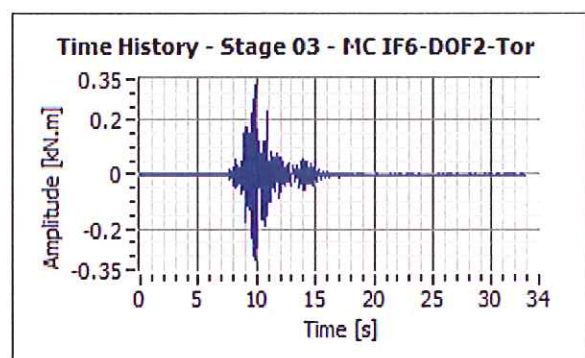


Figure L-57: Time History –Inertia Torque DOF2 –
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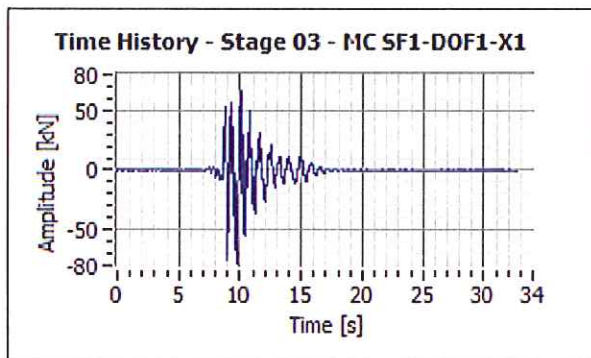


Figure L-58: Time History –Story Force DOF1 – Transversal Direction – Stage 03

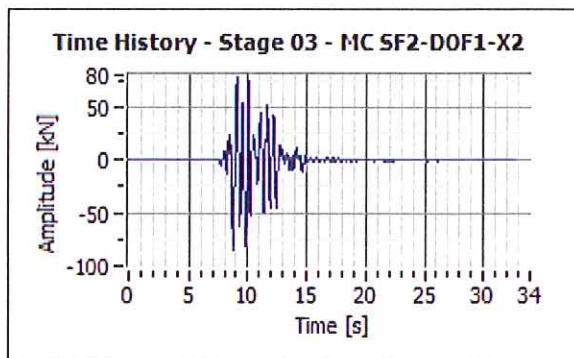


Figure L-59: Time History –Story Force DOF1 – Longitudinal Direction – Stage 03

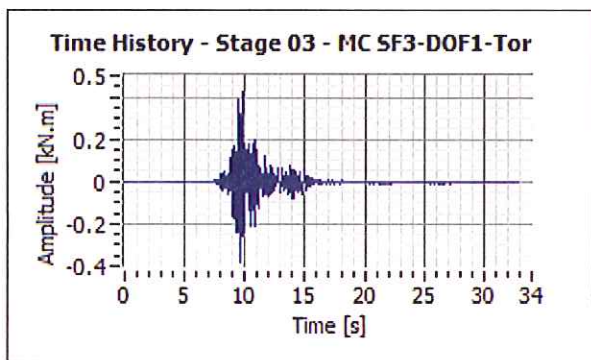


Figure L-60: Time History –Story Torque DOF1 – Stage 03

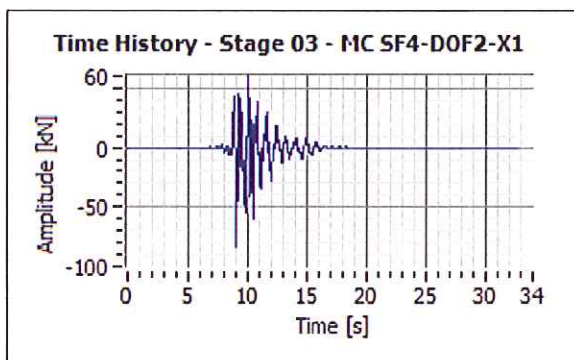


Figure L-61: Time History –Story Force DOF2 – Transversal Direction – Stage 03

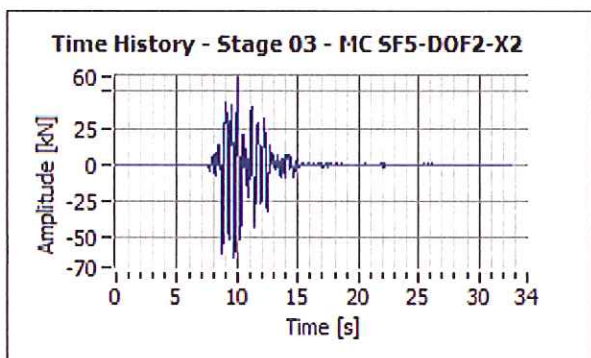


Figure L-62: Time History –Story Force DOF2 – Longitudinal Direction – Stage 03

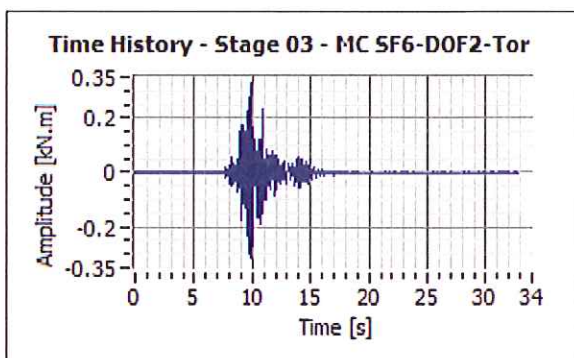


Figure L-63: Time History –Story Torque DOF1 – Stage 03

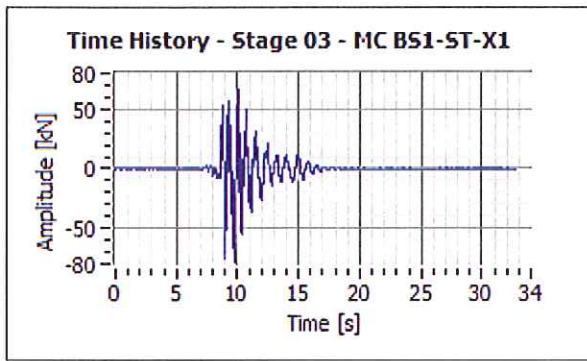


Figure L-64: Time History –Base Shear –
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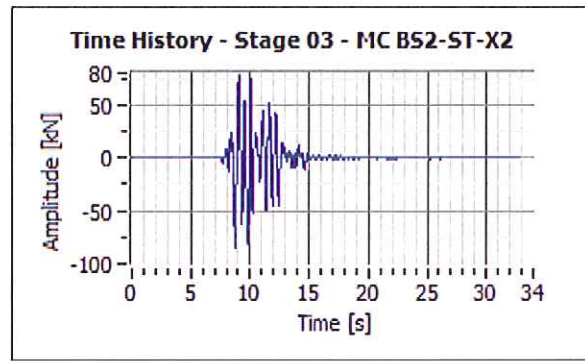


Figure L-65: Time History –Base Shear –
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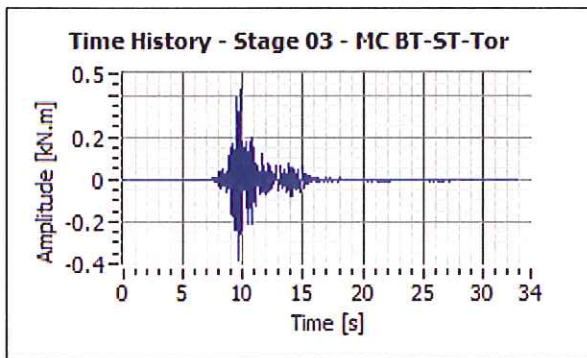


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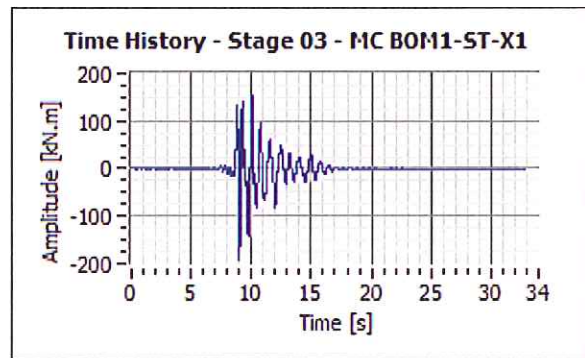


Figure L-67: Time History –Base Overturning
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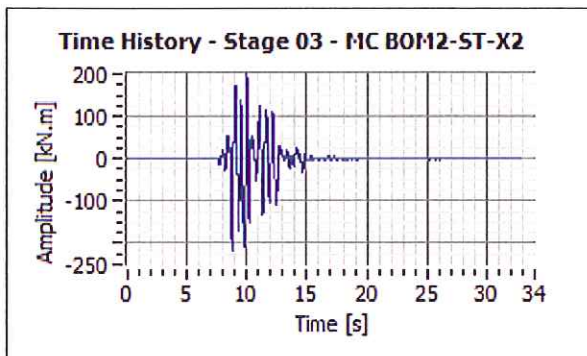


Figure L-68: Time History –Base Overturning
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L.5 Stage 04

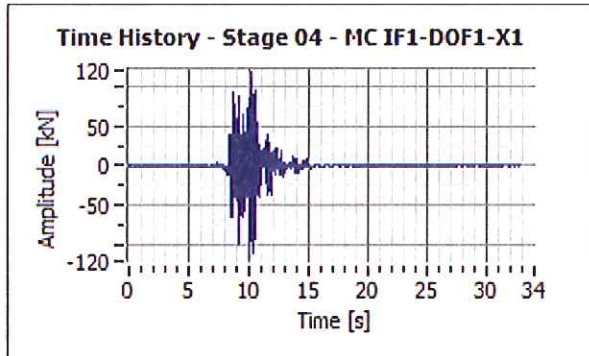


Figure L-69: Time History –Inertia Force DOF1 – Transversal Direction – Stage 04

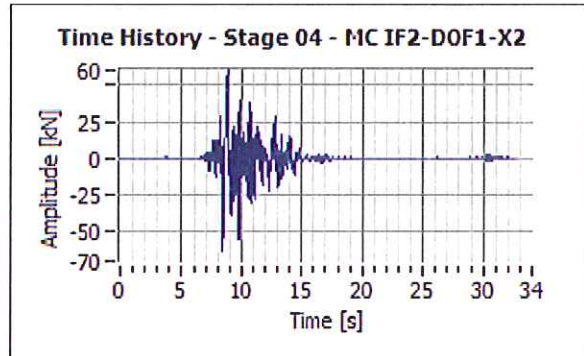


Figure L-70: Time History –Inertia Force DOF1 – Longitudinal Direction – Stage 04

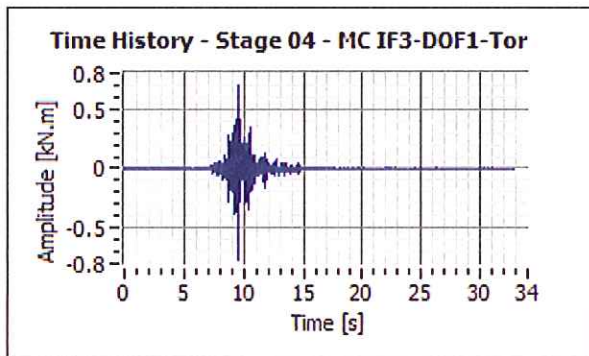


Figure L-71: Time History –Inertia Torque DOF1 – Stage 04

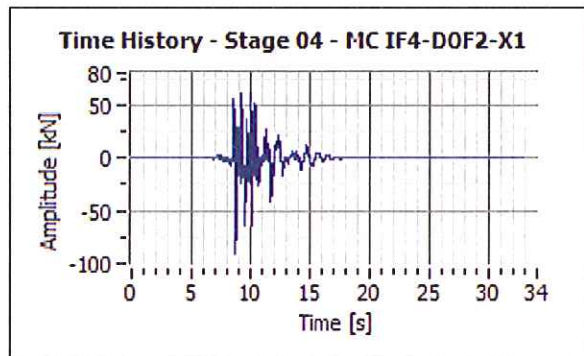


Figure L-72: Time History –Inertia Force DOF2 – Transversal Direction – Stage 04

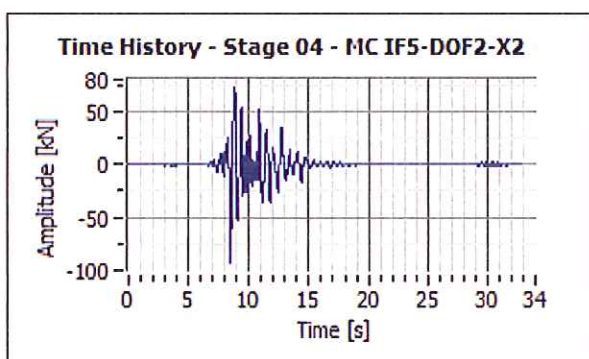


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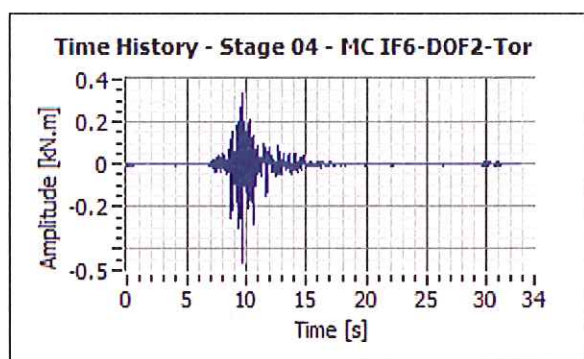


Figure L-74: Time History –Inertia Torque DOF2 – Stage 04

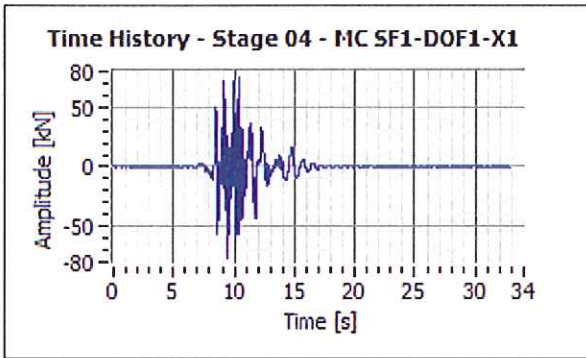


Figure L-75: Time History –Story Force DOF1 – Transversal Direction – Stage 04

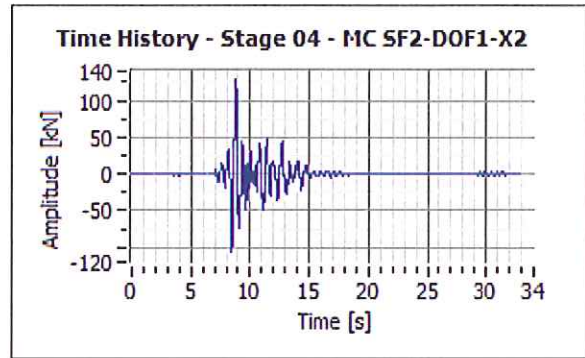


Figure L-76: Time History –Story Force DOF1 – Longitudinal Direction – Stage 04

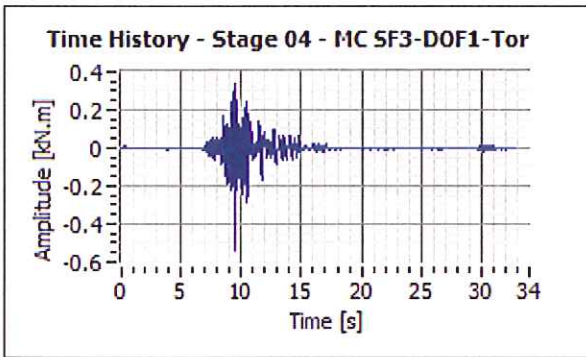


Figure L-77: Time History –Story Torque DOF1 – Stage 04

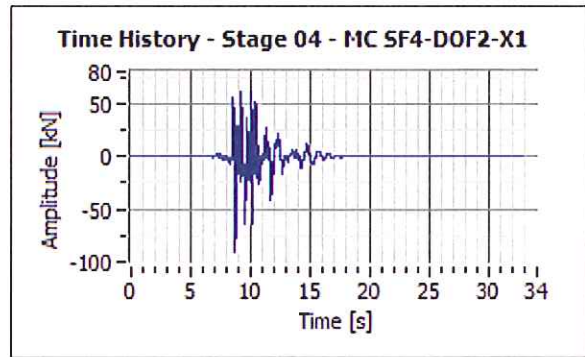


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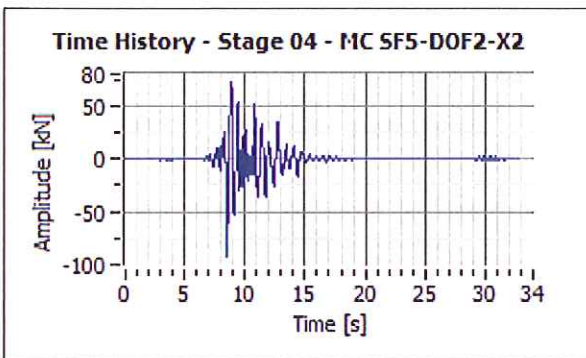


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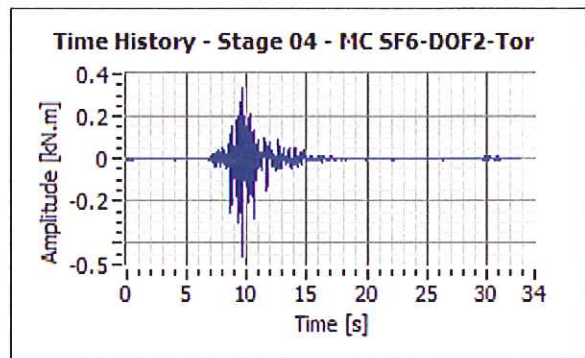


Figure L-80: Time History –Story Torque DOF1 – Stage 04

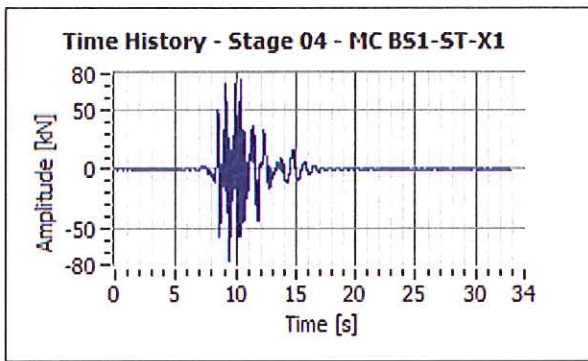


Figure L-81: Time History –Base Shear –
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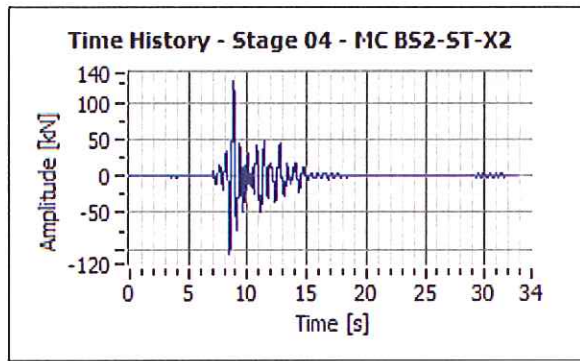


Figure L-82: Time History –Base Shear –
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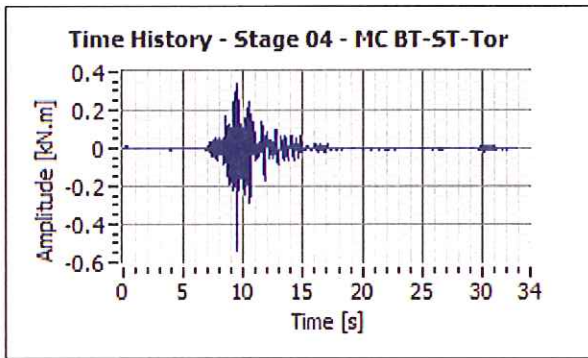


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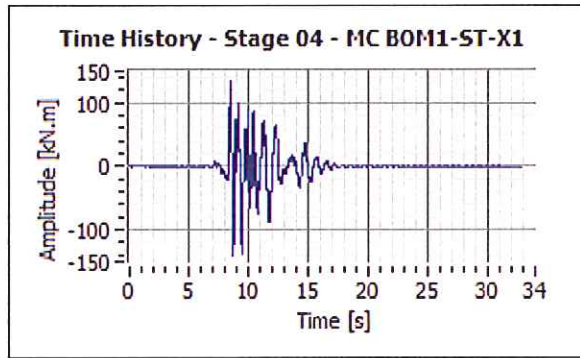


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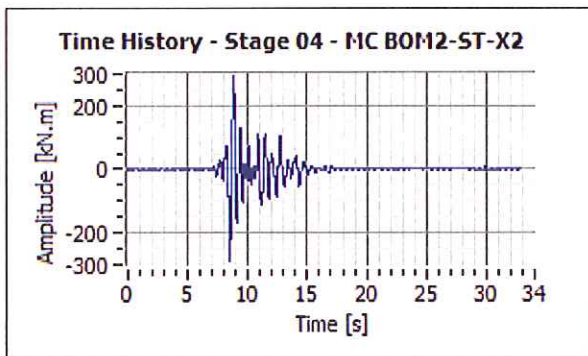


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ANNEX M HISTERESIS LOOPS

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M.1 Global Hysteresis Loops

M.1.1 Stage 00

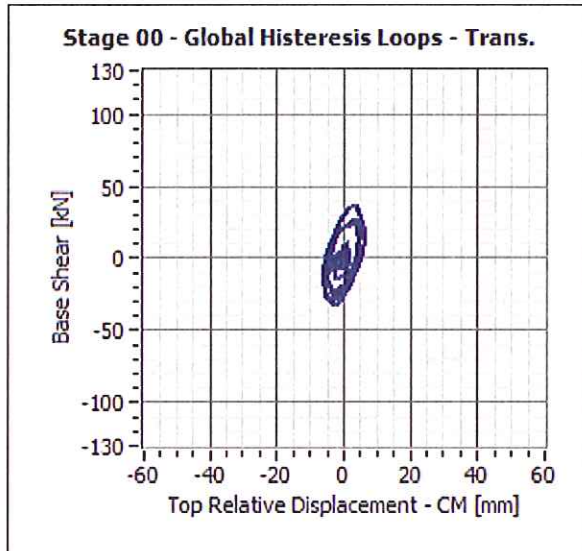


Figure M-1: Global Hysteresis Loops – Transversal Direction – Stage 00

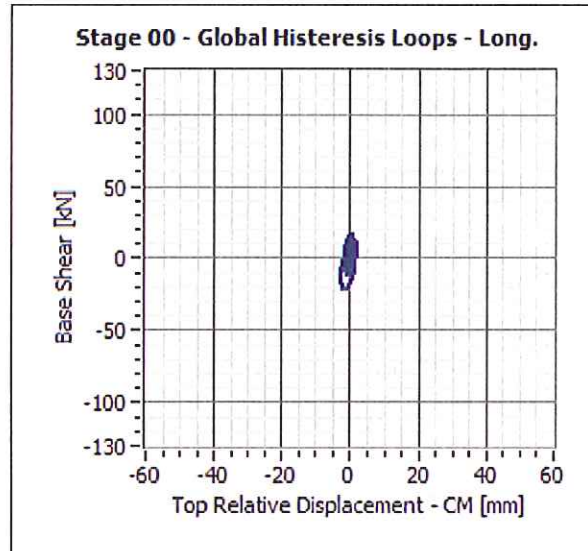


Figure M-2: Global Hysteresis Loops – Longitudinal Direction – Stage 00

M.1.2 Stage 01

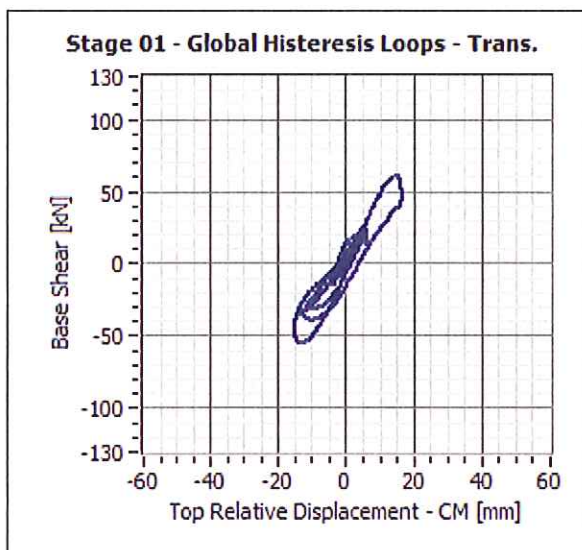


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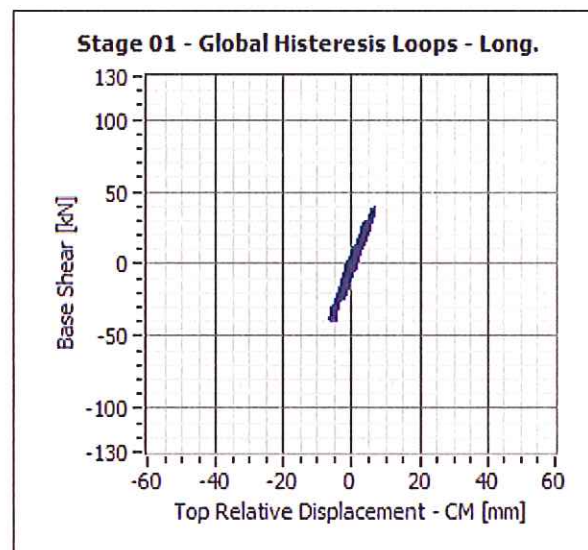


Figure M-4: Global Hysteresis Loops – Longitudinal Direction – Stage 01

M.1.3 Stage 02

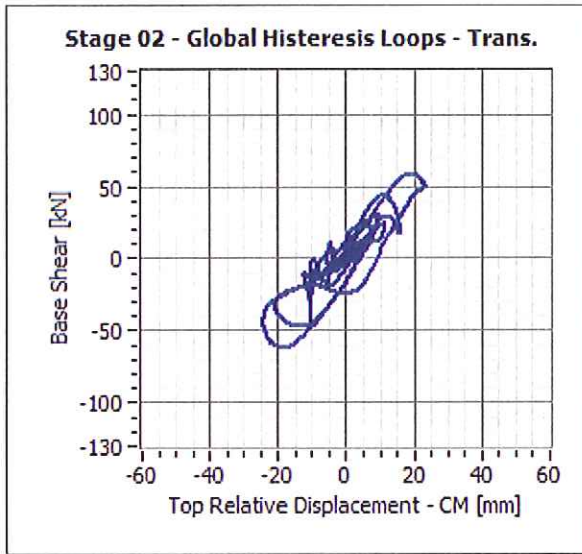


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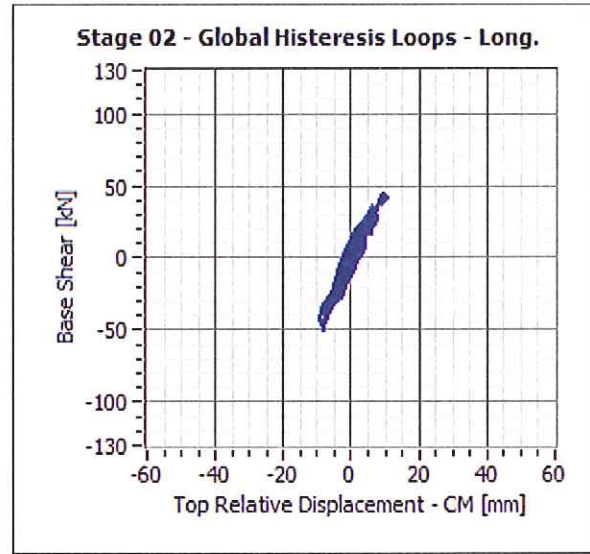


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M.1.4 Stage 03

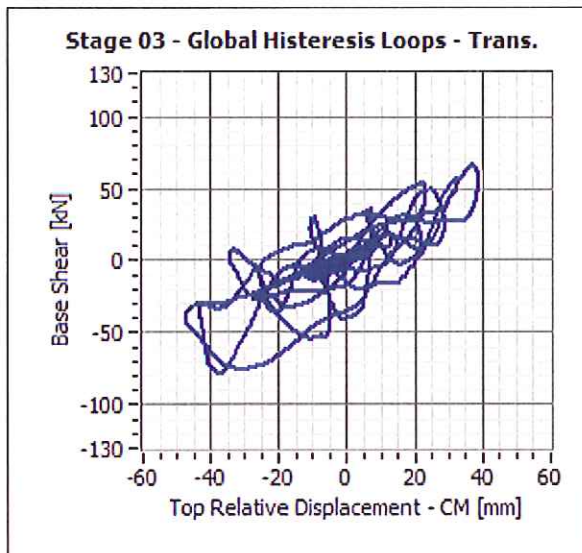


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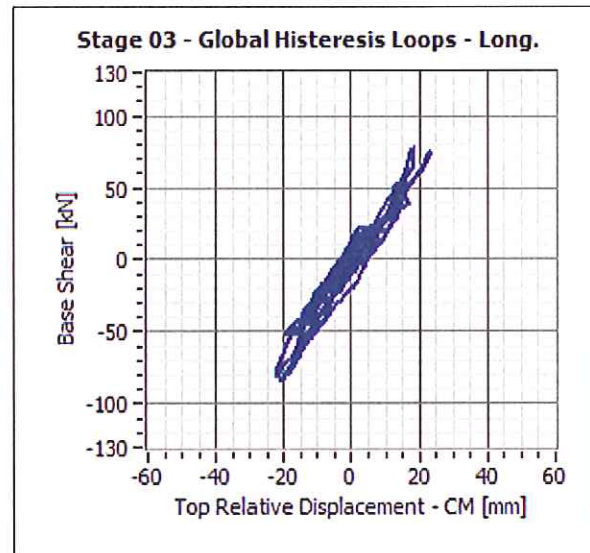


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M.1.5 Stage 04

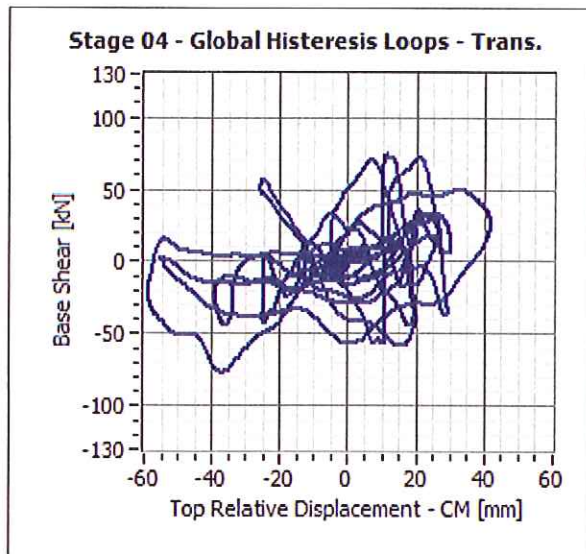


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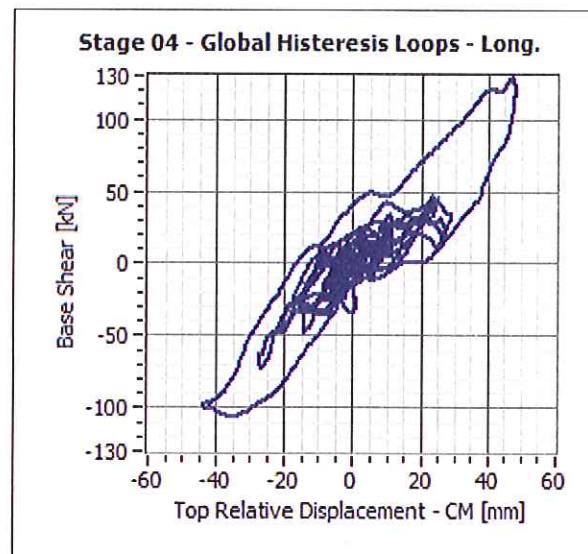


Figure M-10: Global Hysteresis Loops – Longitudinal Direction – Stage 04

M.2 Storey Hysteresis Loops

M.2.1 Stage 00

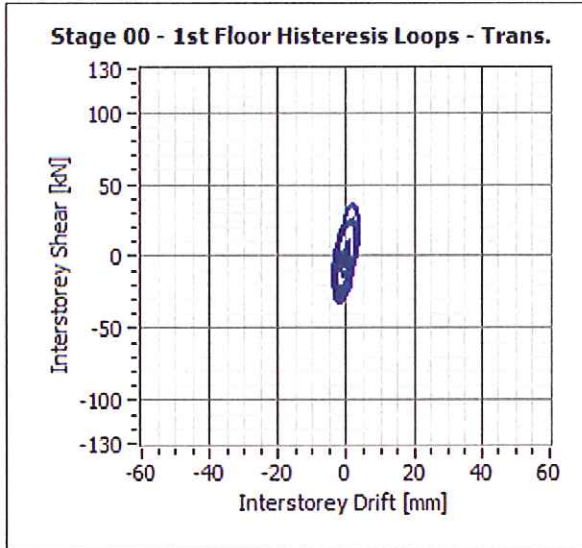


Figure M-11: Storey Hysteresis Loops – 1st Floor – Transversal Direction – Stage 00

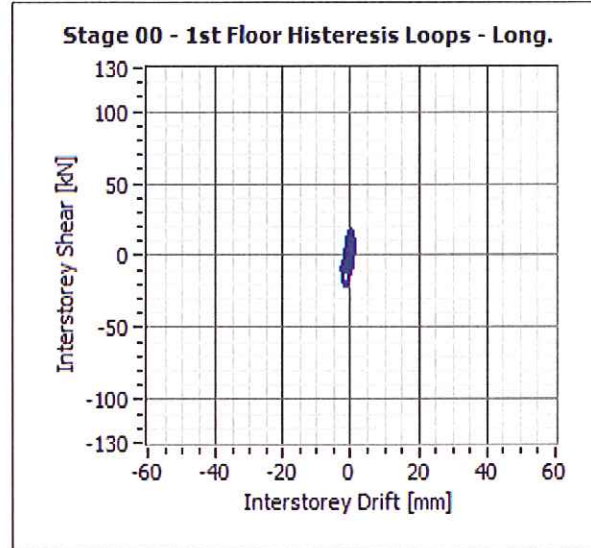


Figure M-12: Storey Hysteresis Loops – 1st Floor – Longitudinal Direction – Stage 00

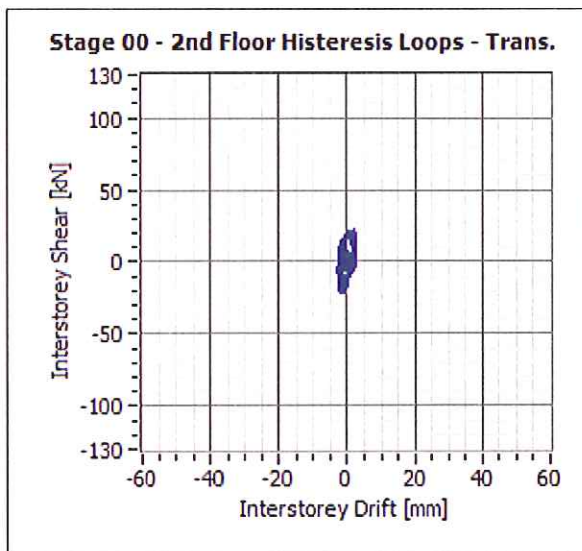


Figure M-13: Storey Hysteresis Loops – 2nd Floor – Transversal Direction – Stage 00

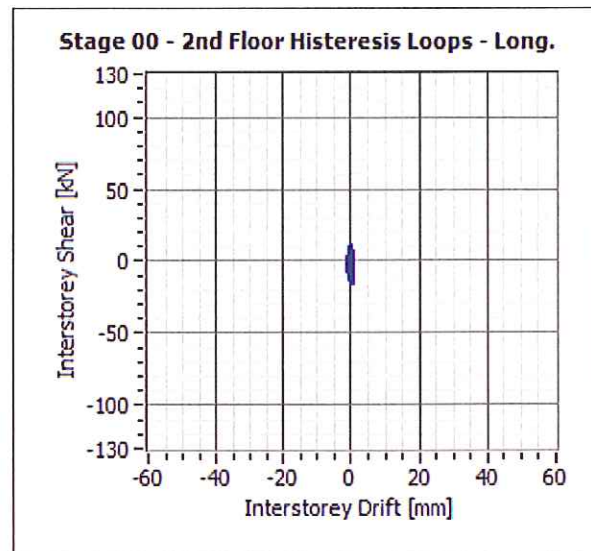


Figure M-14: Storey Hysteresis Loops – 2nd Floor – Longitudinal Direction – Stage 00

M.2.2 Stage 01

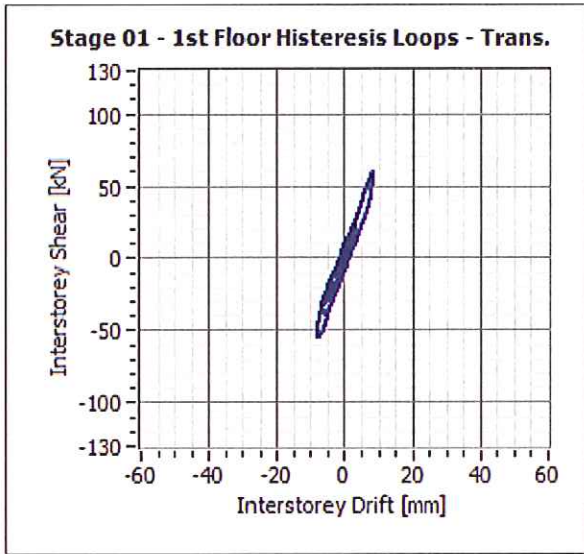


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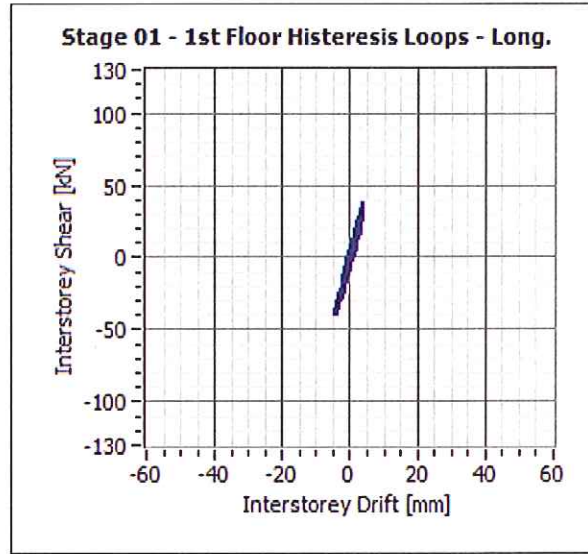


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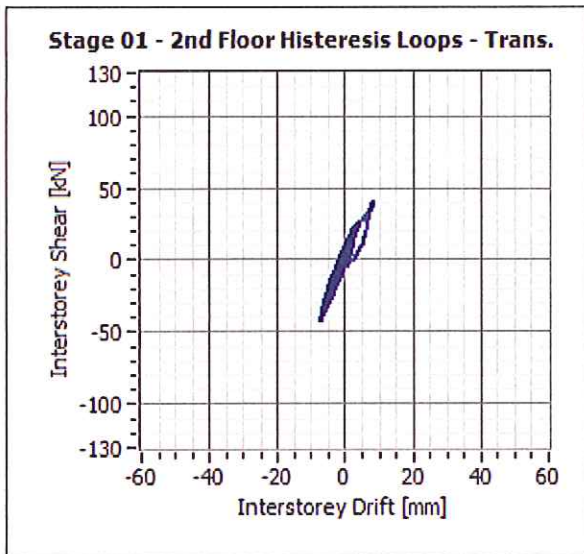


Figure M-17: Storey Hysteresis Loops – 2nd Floor – Transversal Direction – Stage 01

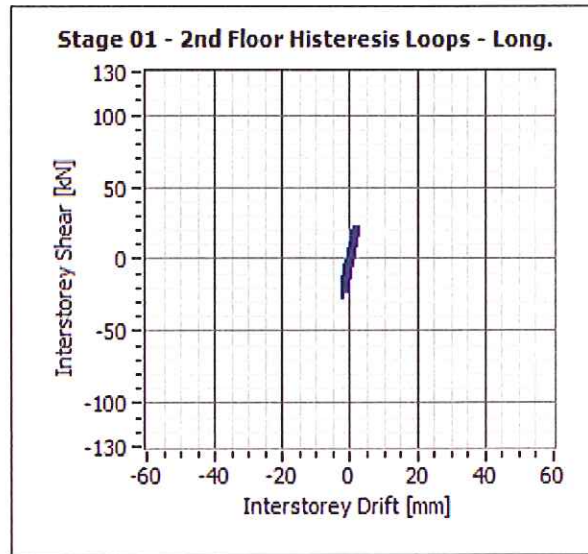


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M.2.3 Stage 02

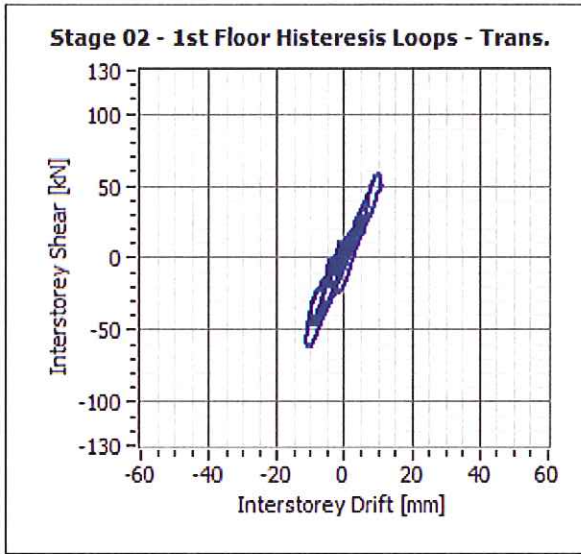


Figure M-19: Storey Hysteresis Loops – 1st Floor – Transversal Direction – Stage 02

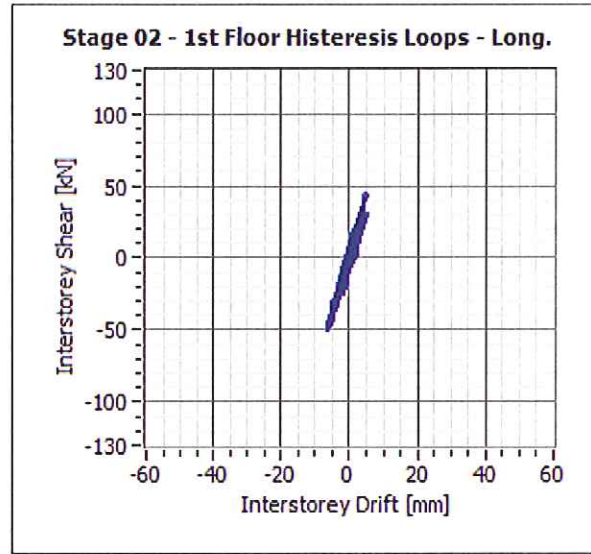


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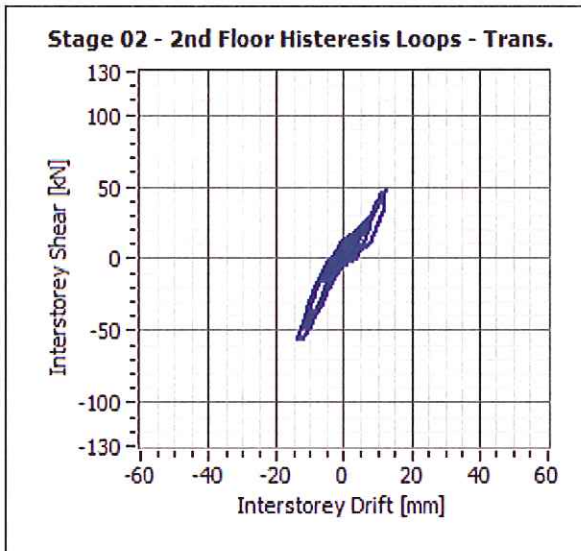


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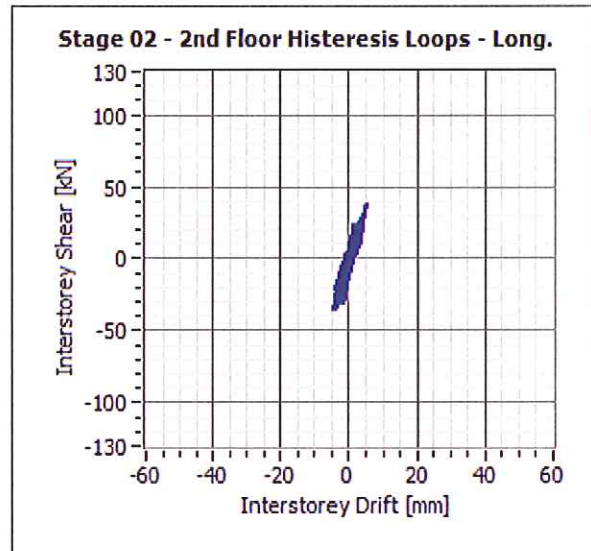


Figure M-22: Storey Hysteresis Loops – 2nd Floor – Longitudinal Direction – Stage 02

M.2.4 Stage 03

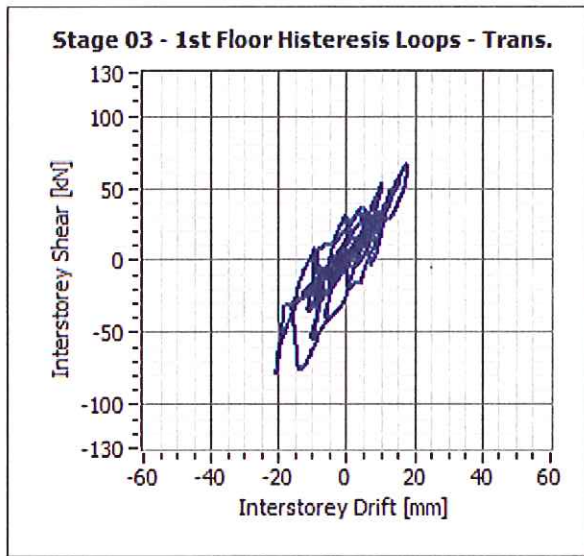


Figure M-23: Storey Hysteresis Loops – 1st Floor – Transversal Direction – Stage 03

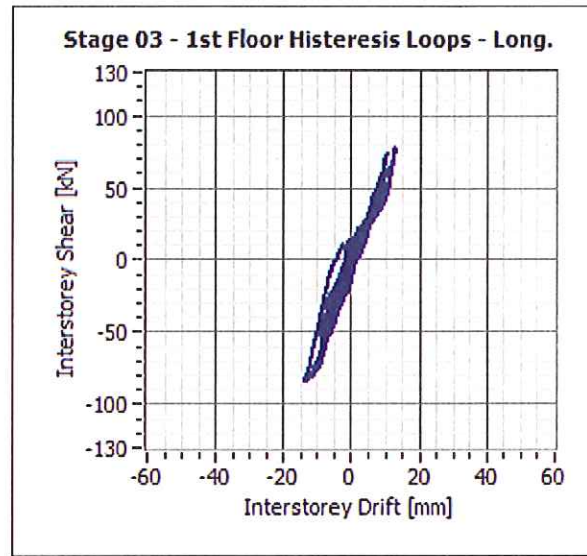


Figure M-24: Storey Hysteresis Loops – 1st Floor – Longitudinal Direction – Stage 03

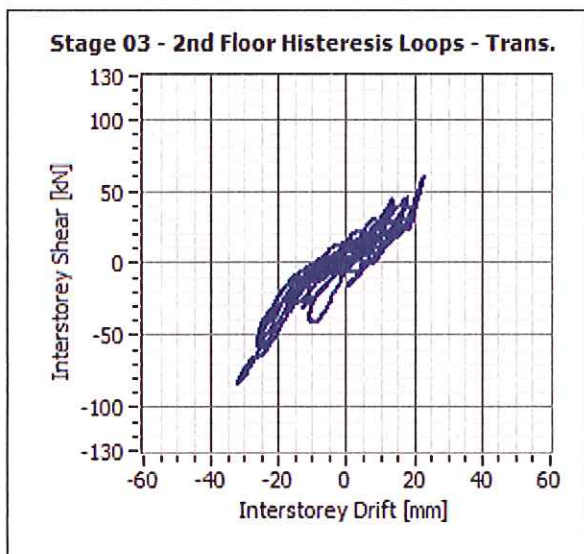


Figure M-25: Storey Hysteresis Loops – 2nd Floor – Transversal Direction – Stage 03

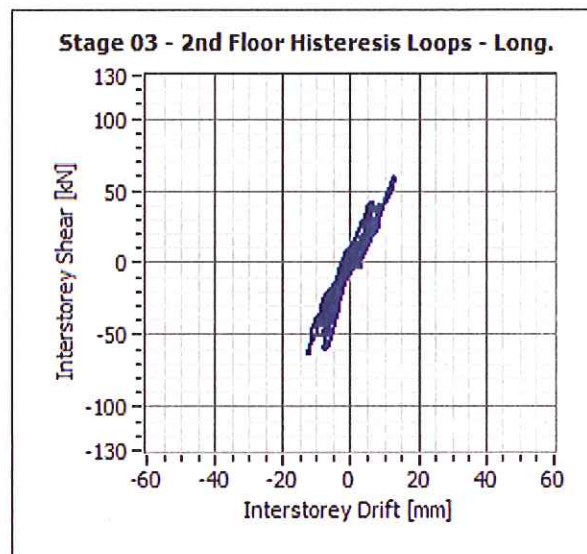


Figure M-26: Storey Hysteresis Loops – 2nd Floor – Longitudinal Direction – Stage 03

M.2.5 Stage 04

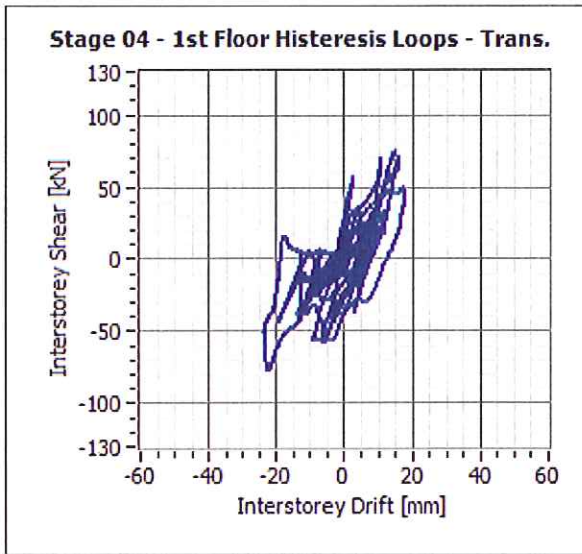


Figure M-27: Storey Hysteresis Loops – 1st Floor – Transversal Direction – Stage 04

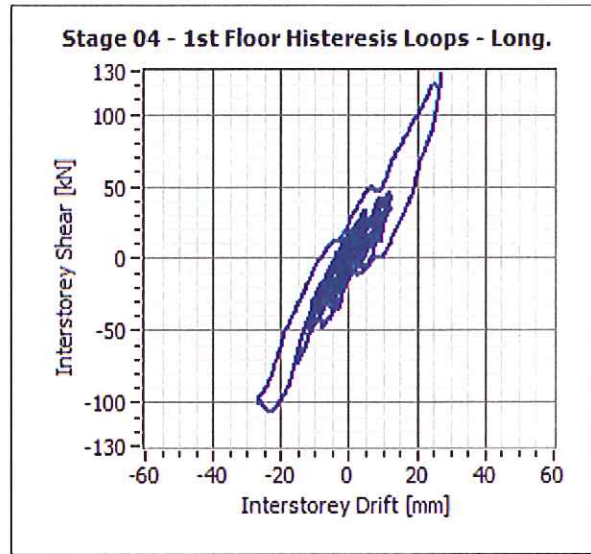


Figure M-28: Storey Hysteresis Loops – 1st Floor – Longitudinal Direction – Stage 04

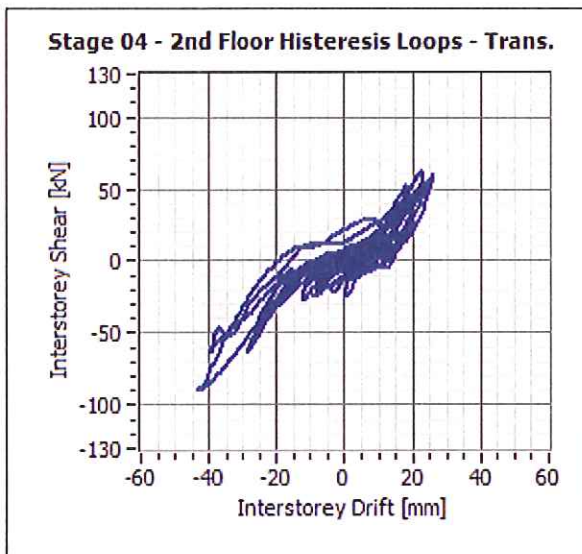


Figure M-29: Storey Hysteresis Loops – 2nd Floor – Transversal Direction – Stage 04

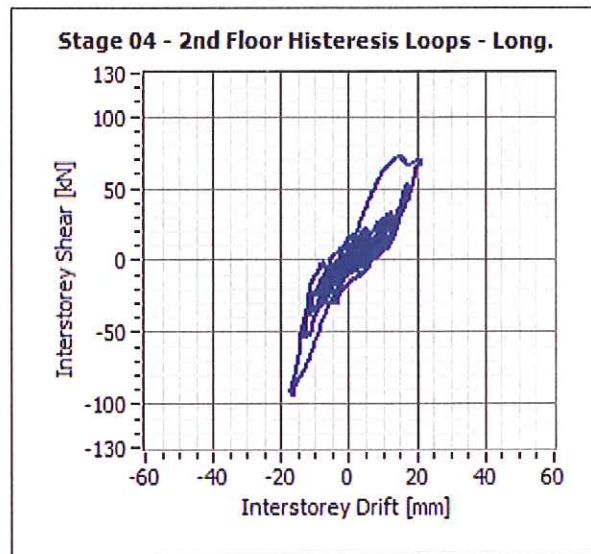


Figure M-30: Storey Hysteresis Loops – 2nd Floor – Longitudinal Direction – Stage 04

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N.1 Math channels

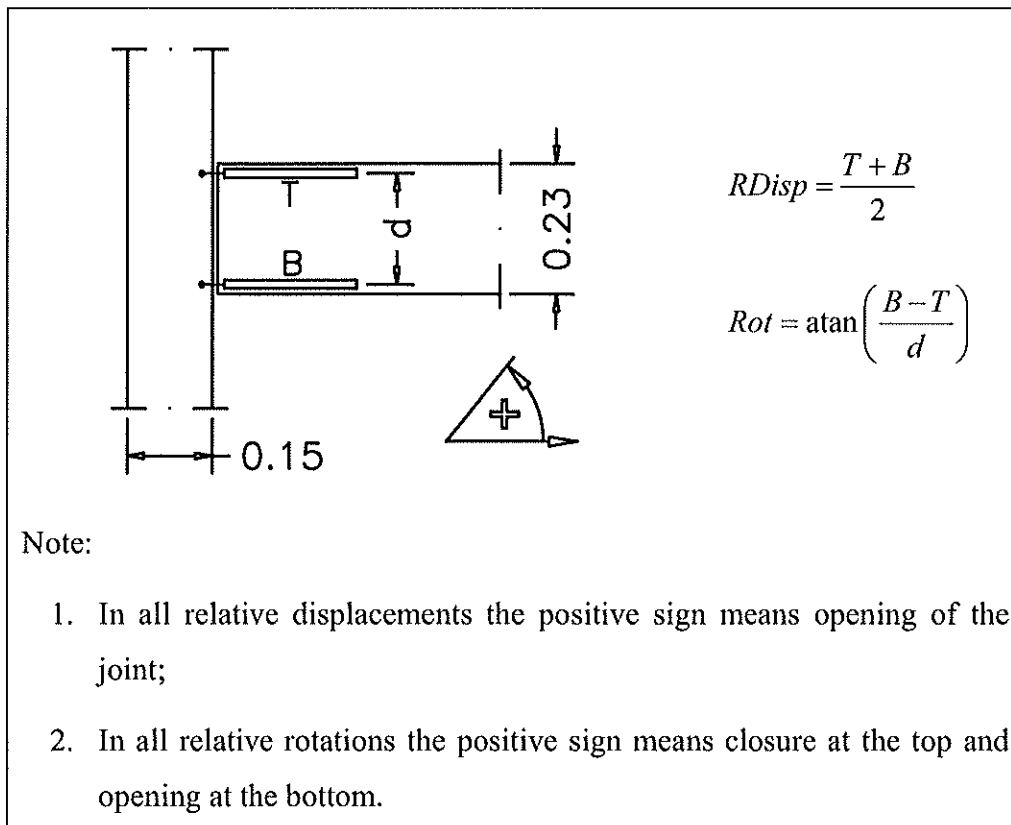


Figure N-1: Math channels calculation scheme

N.2 Joint 3 – Transversal Direction

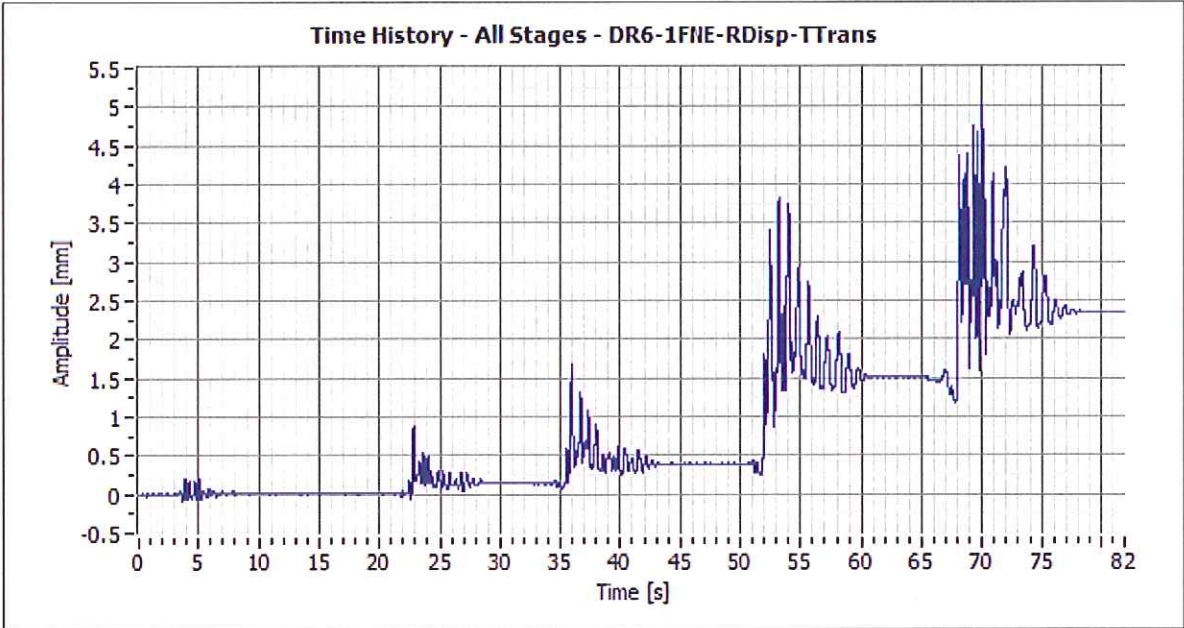


Figure N-2: Time History – Top Relative Displacement – Joint 3 Transversal

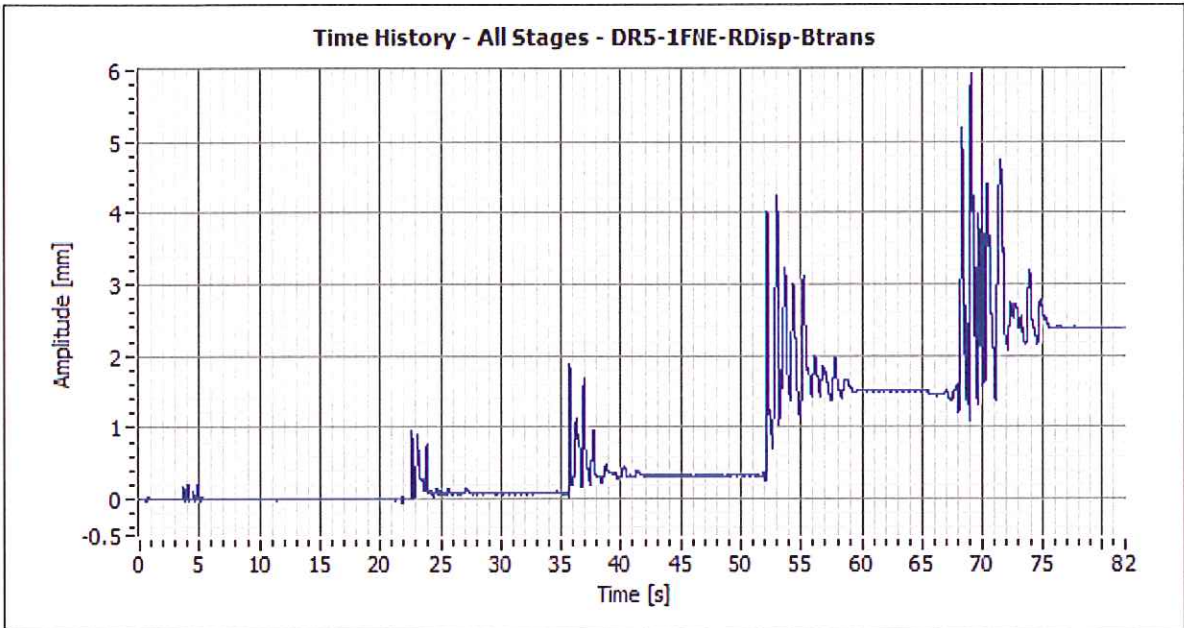


Figure N-3: Time History – Bottom Relative Displacement – Joint 3 Transversal

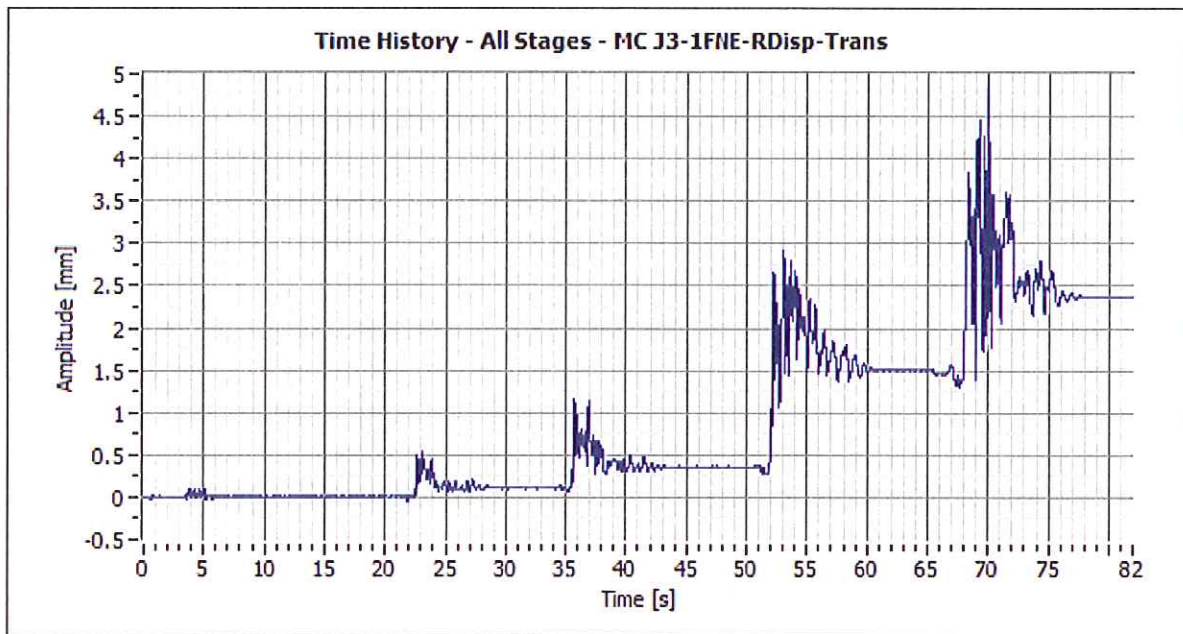


Figure N-4: Time History – Global Relative Displacement – Joint 3 Transversal

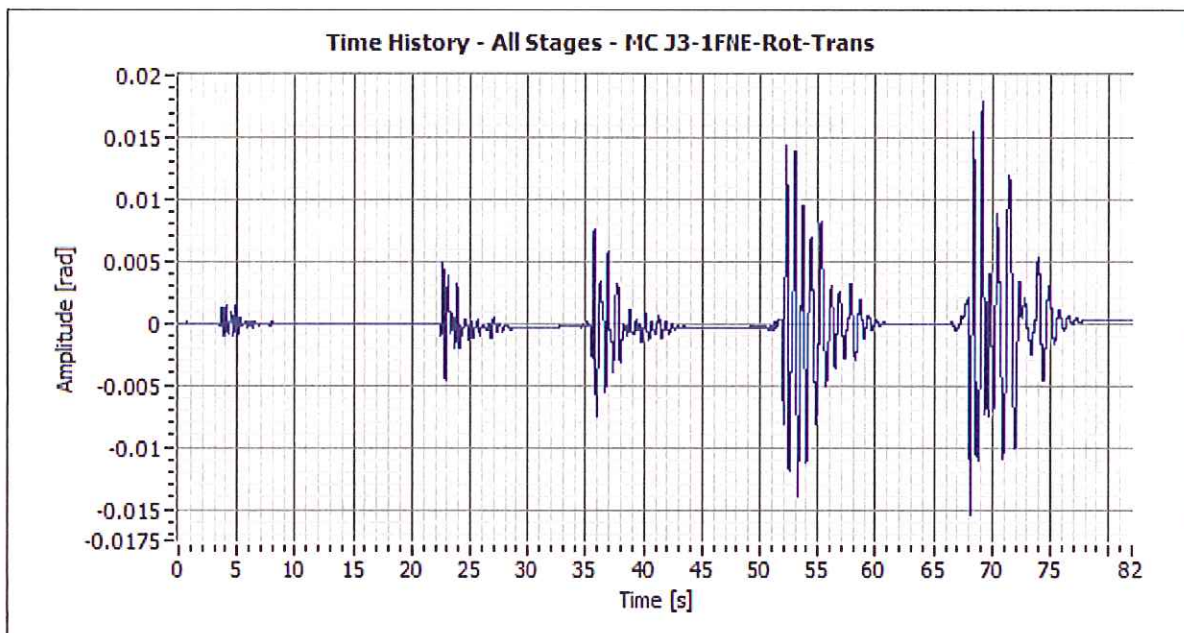


Figure N-5: Time History – Global Relative Rotation – Joint 3 Transversal

N.3 Joint 9 – Transversal Direction

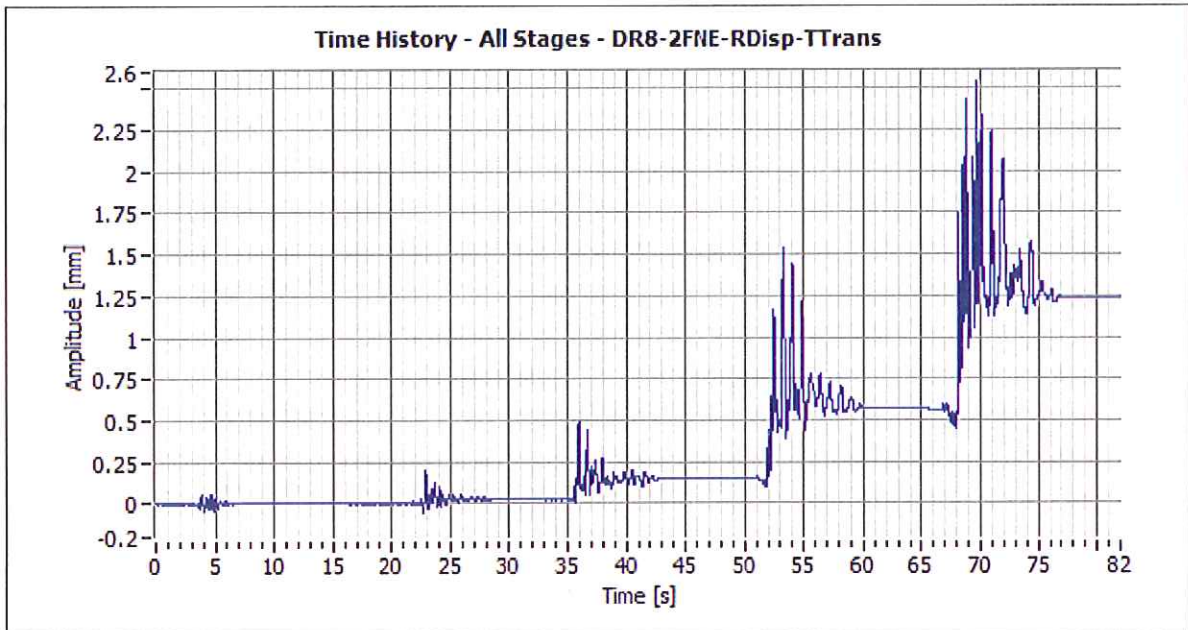


Figure N-6: Time History – Top Relative Displacement – Joint 8 Transversal

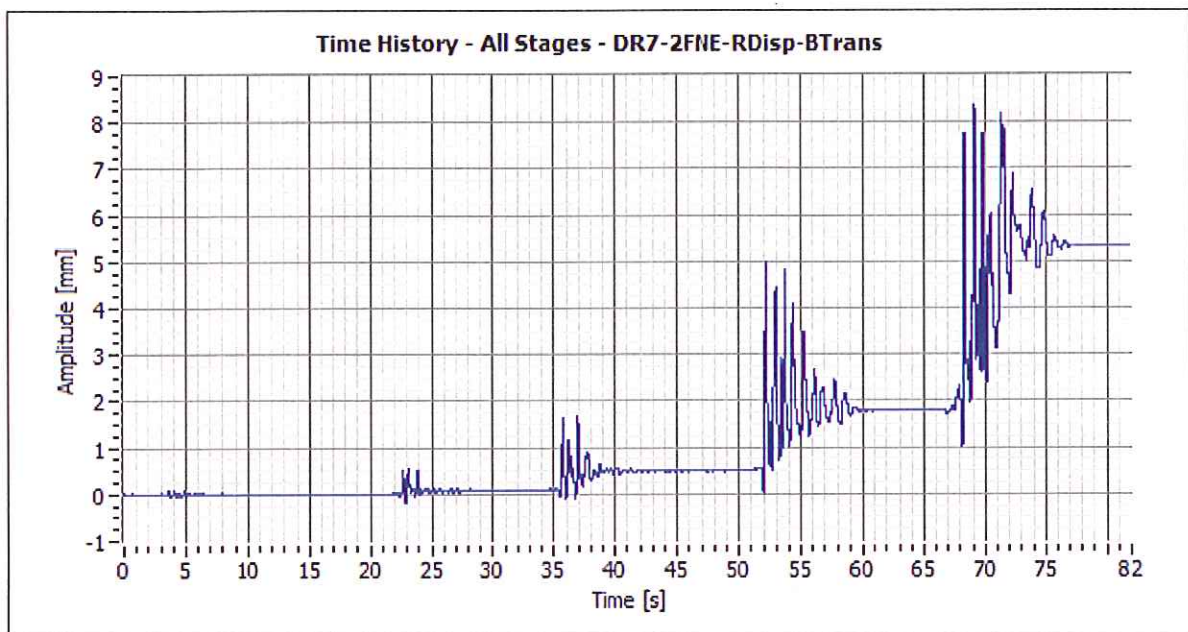


Figure N-7: Time History – Bottom Relative Displacement – Joint 3 Transversal

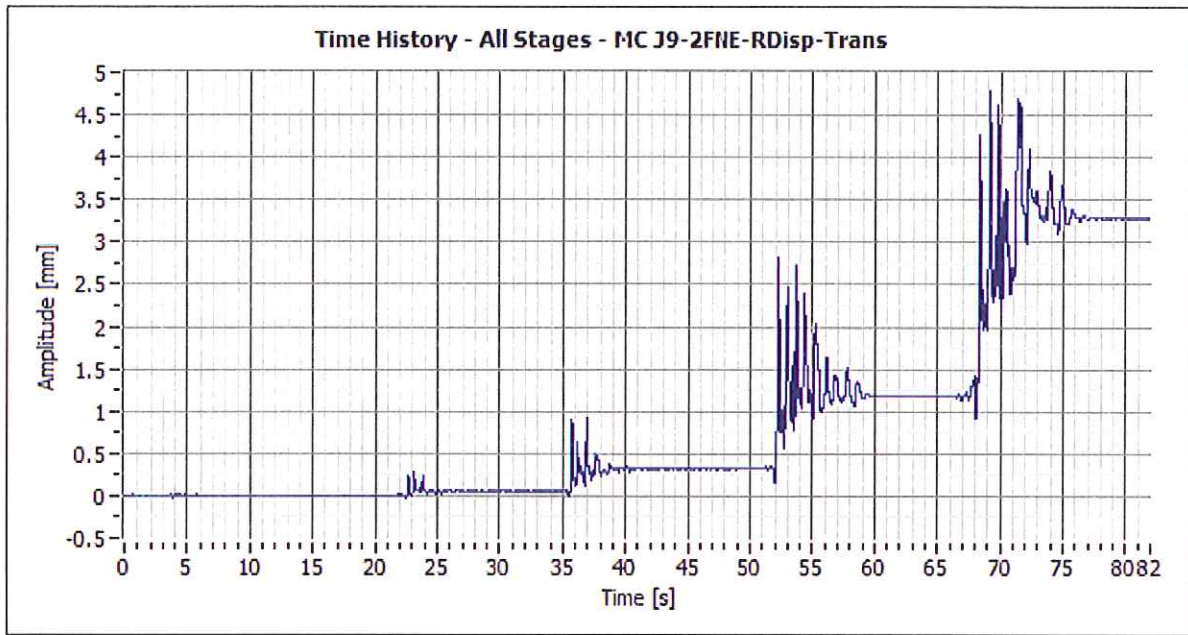


Figure N-8: Time History – Global Relative Displacement – Joint 9 Transversal

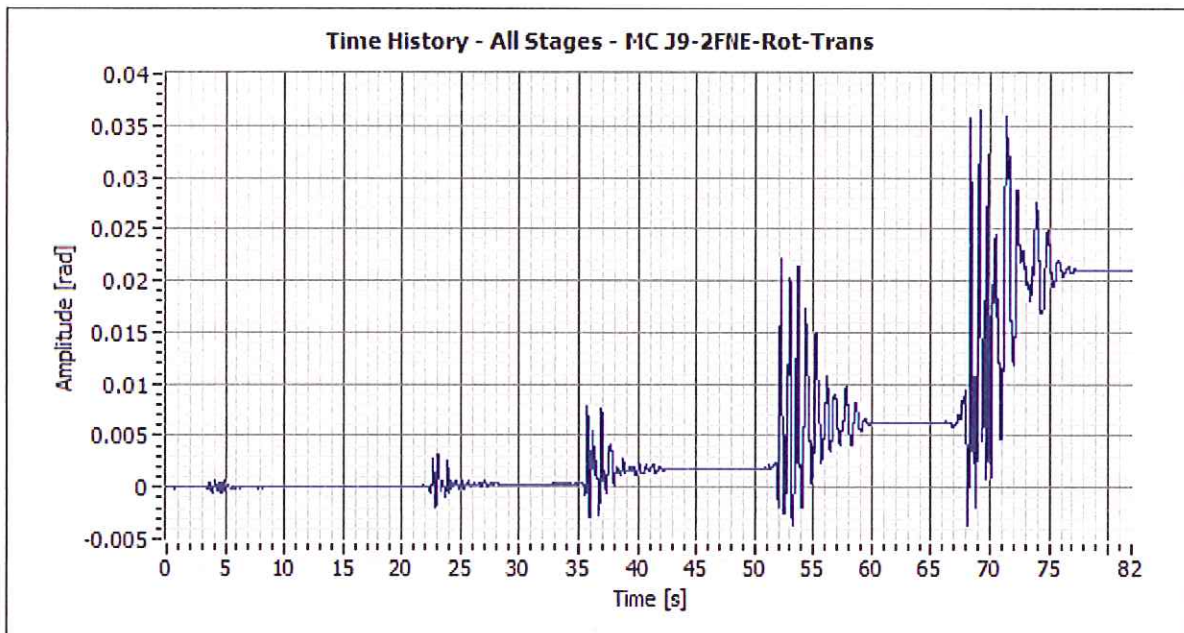


Figure N-9: Time History – Global Relative Rotation – Joint 9 Transversal

N.4 Joint 3 – Longitudinal Direction

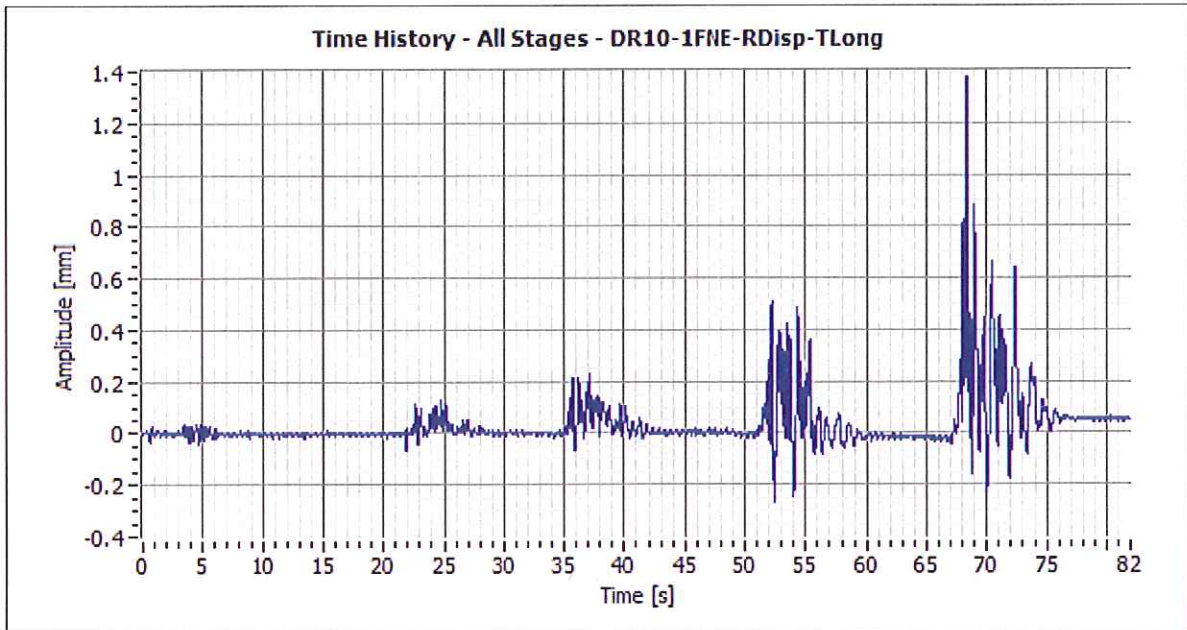


Figure N-10: Time History – Top Relative Displacement – Joint 3 Longitudinal

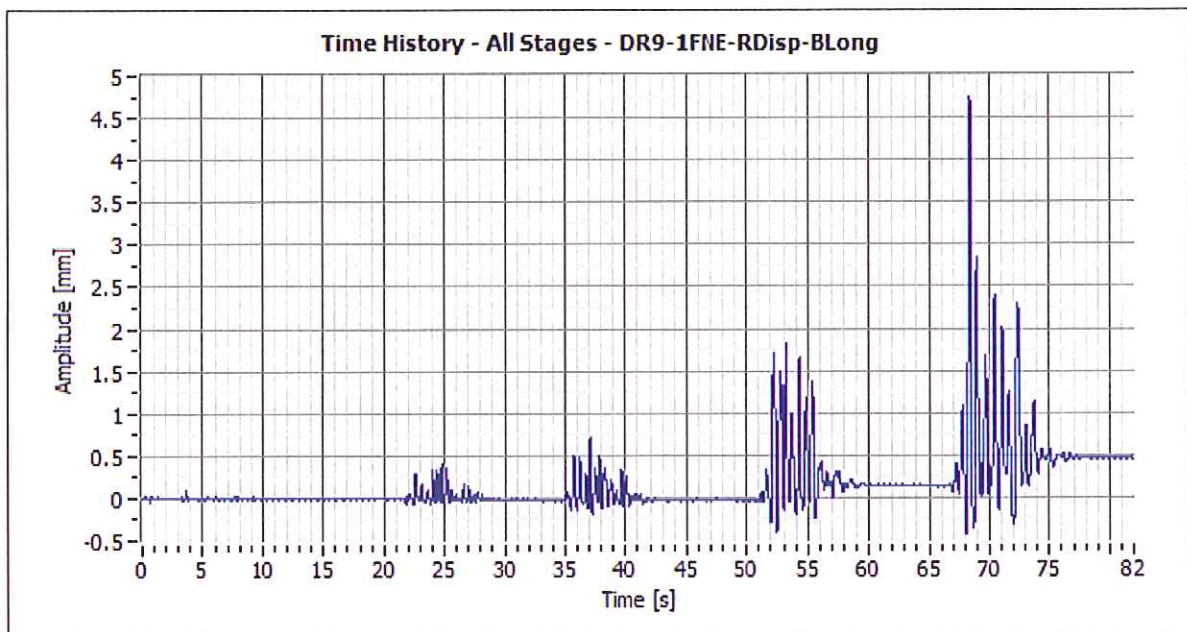


Figure N-11: Time History – Bottom Relative Displacement – Joint 3 Longitudinal

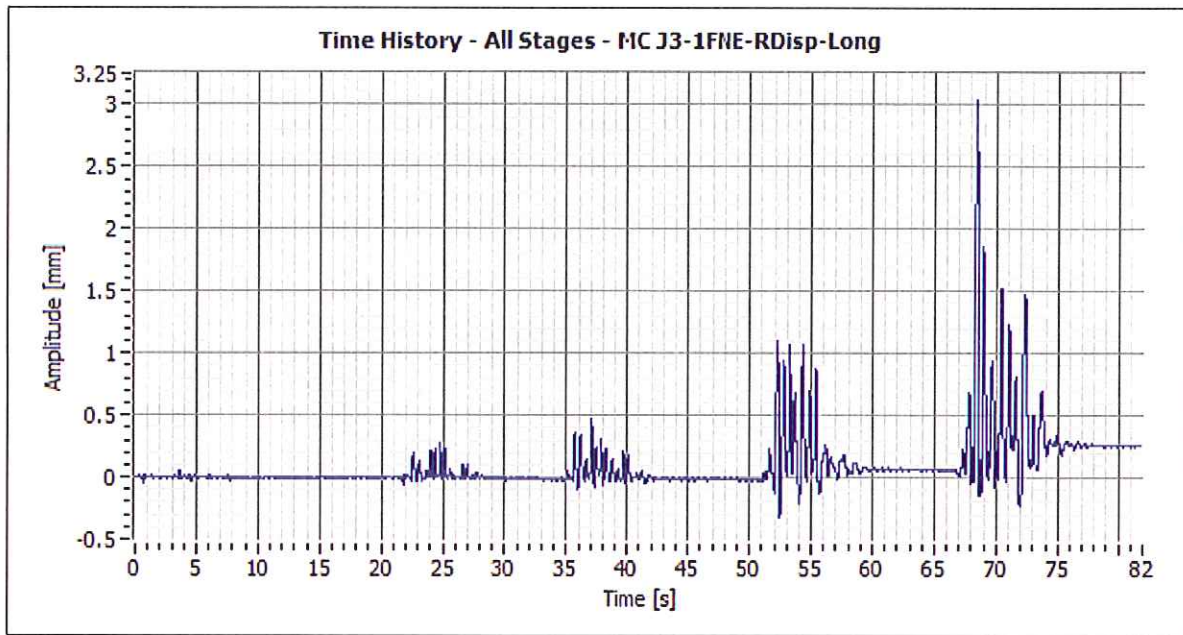


Figure N-12: Time History – Global Relative Displacement – Joint 3 Longitudinal

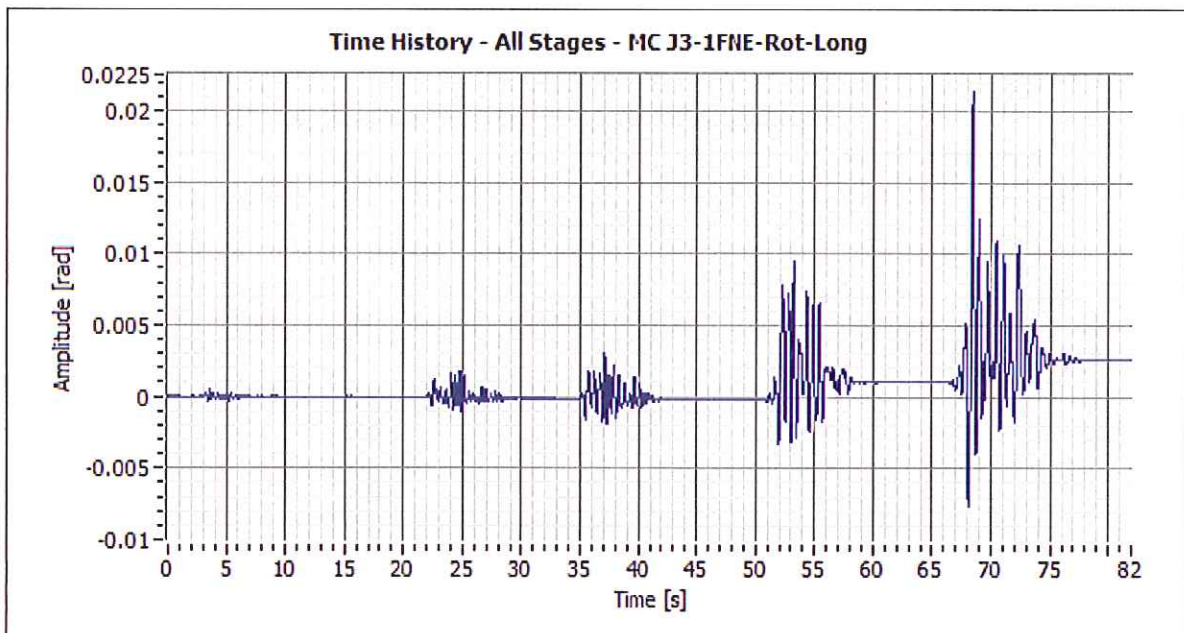


Figure N-13: Time History – Global Relative Rotation – Joint 3 Longitudinal

N.5 Joint 9 – Longitudinal Direction

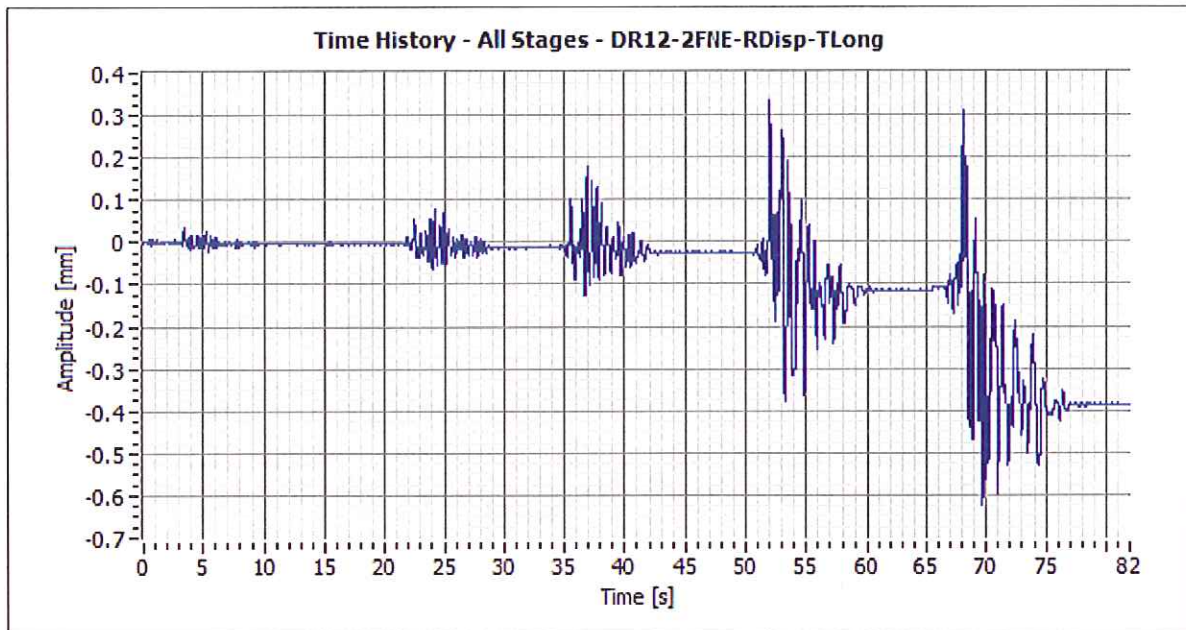


Figure N-14: Time History – Top Relative Displacement – Joint 9 Longitudinal

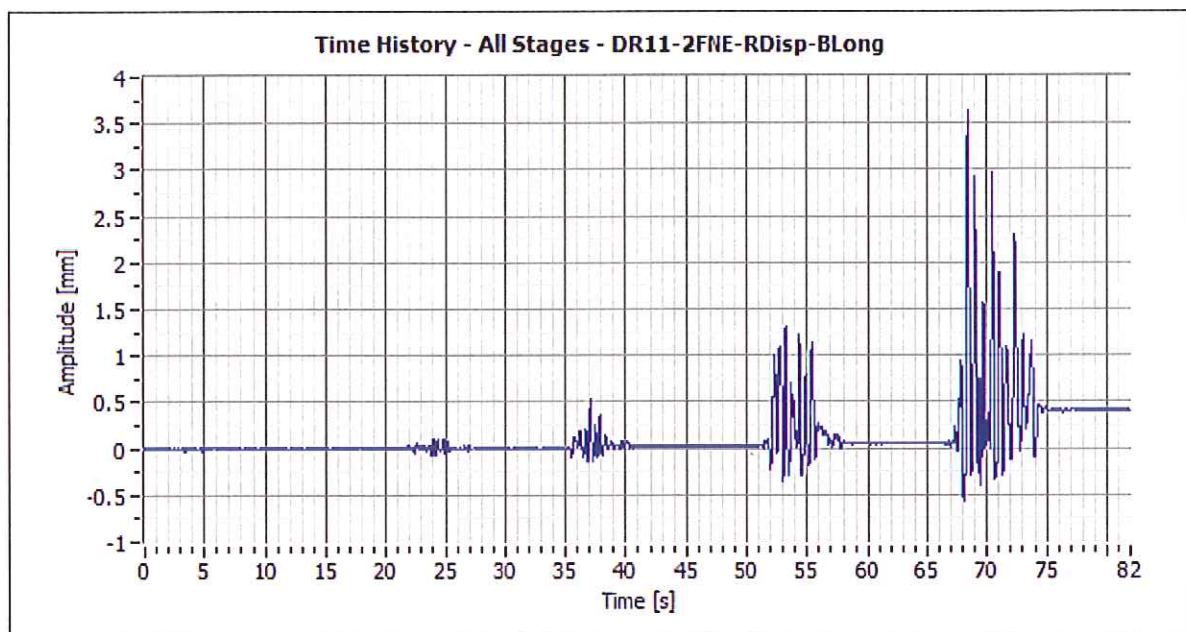


Figure N-15: Time History – Bottom Relative Displacement – Joint 9 Longitudinal

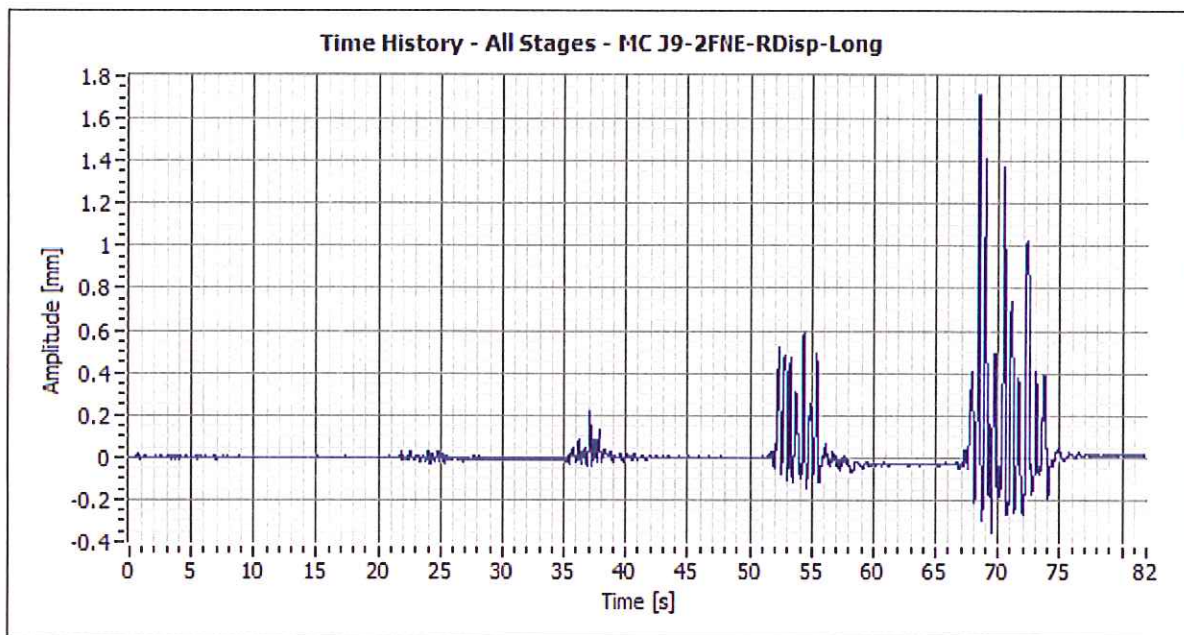


Figure N-16: Time History – Global Relative Displacement – Joint 9 Longitudinal

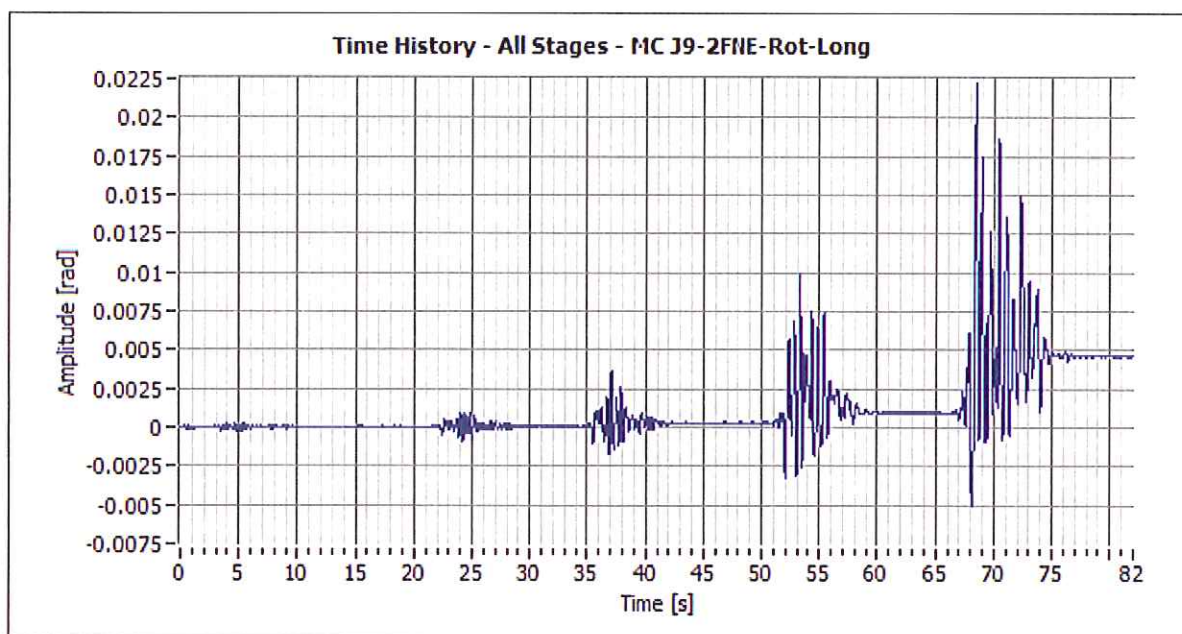


Figure N-17: Time History – Global Relative Rotation – Joint 9 Longitudinal

N.6 Joint 2

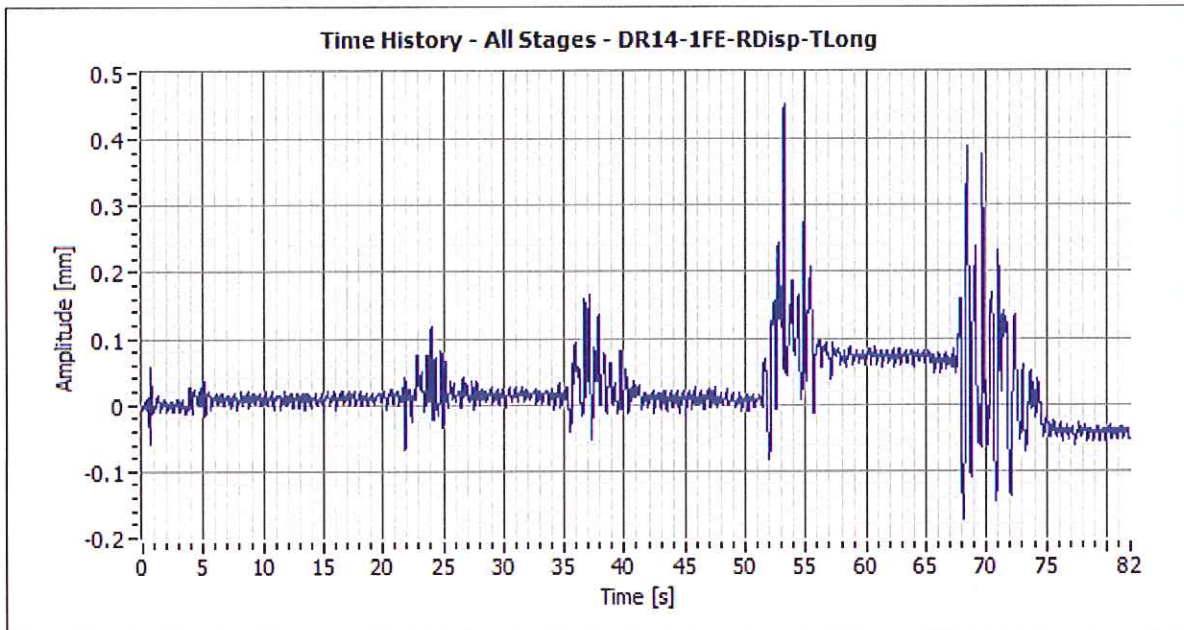


Figure N-18: Time History – Top Relative Displacement – Joint 2 NE

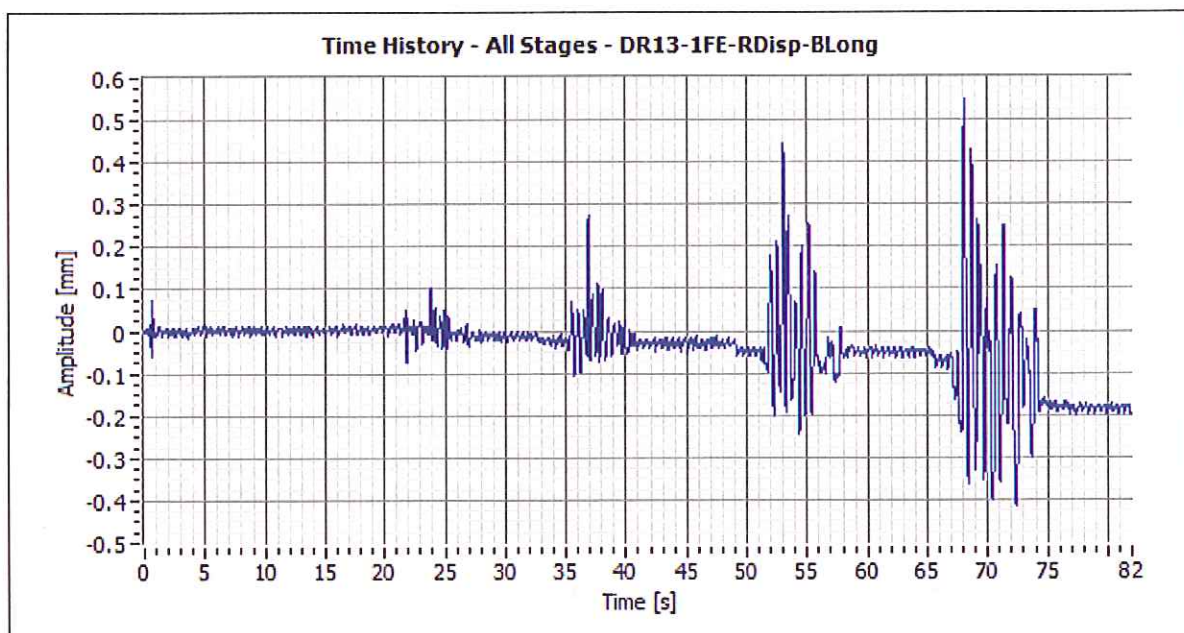


Figure N-19: Time History – Bottom Relative Displacement – Joint 2 NE

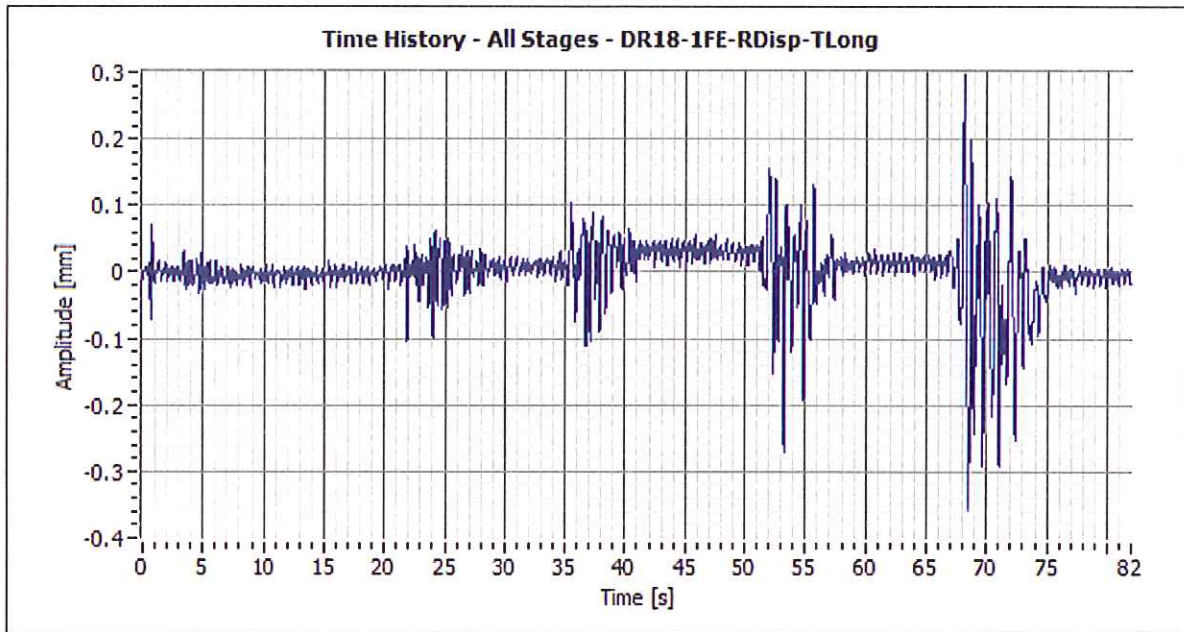


Figure N-20: Time History – Top Relative Displacement – Joint 2 SE

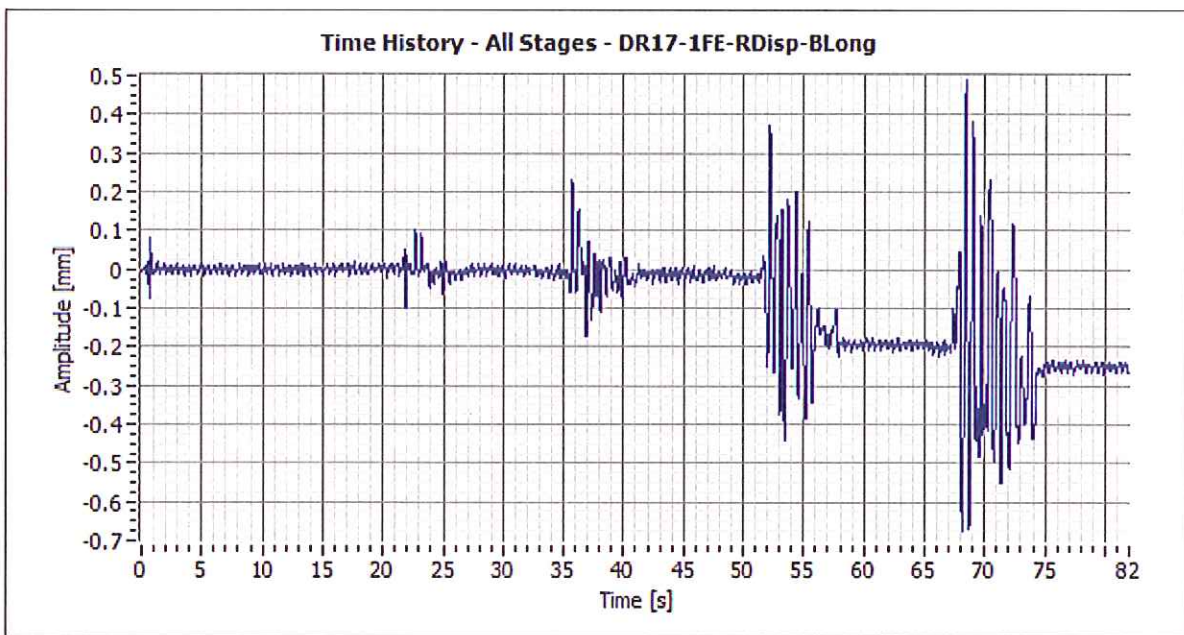


Figure N-21: Time History – Bottom Relative Displacement – Joint 2 SE

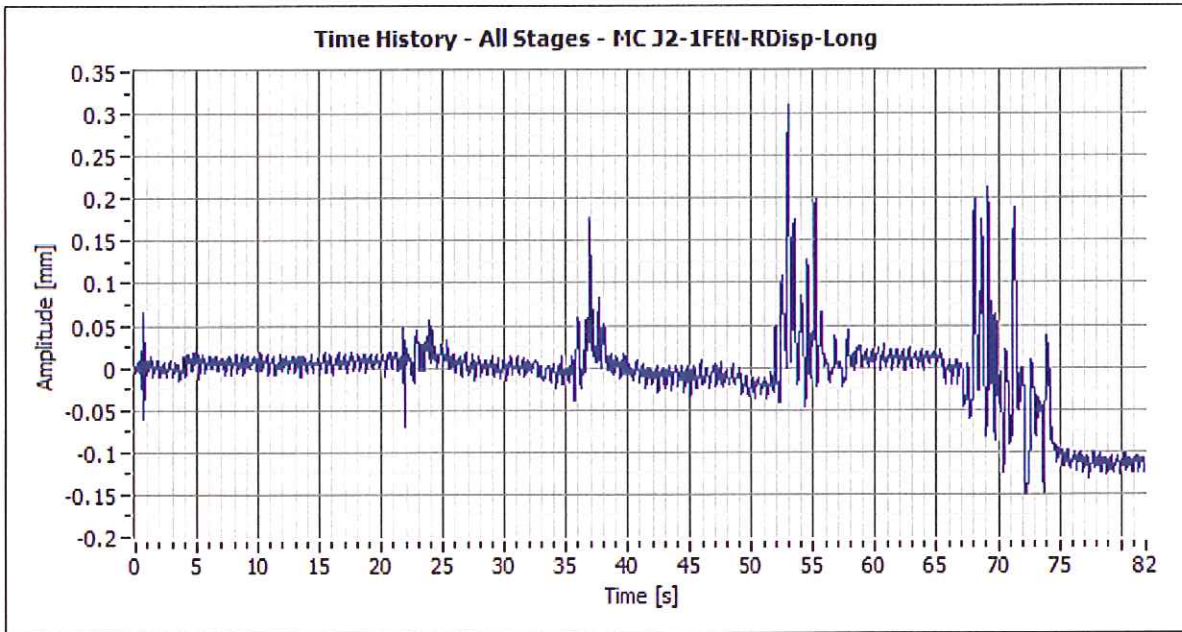


Figure N-22: Time History – Global Relative Displacement – Joint 2 NE

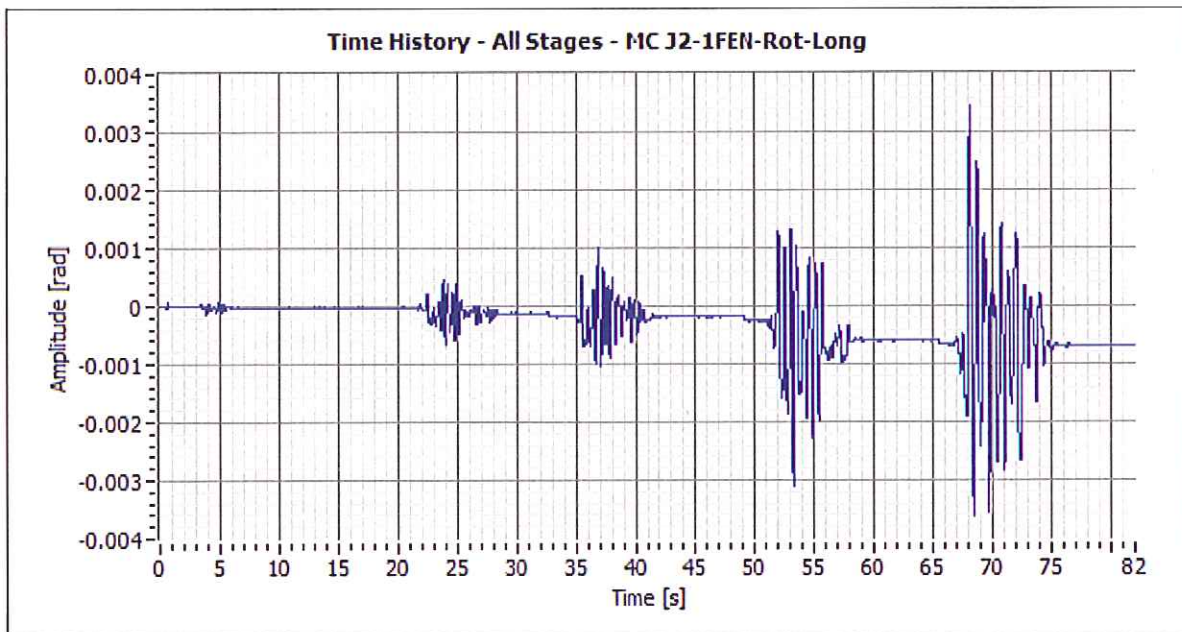


Figure N-23: Time History – Global Relative Rotation – Joint 2 NE

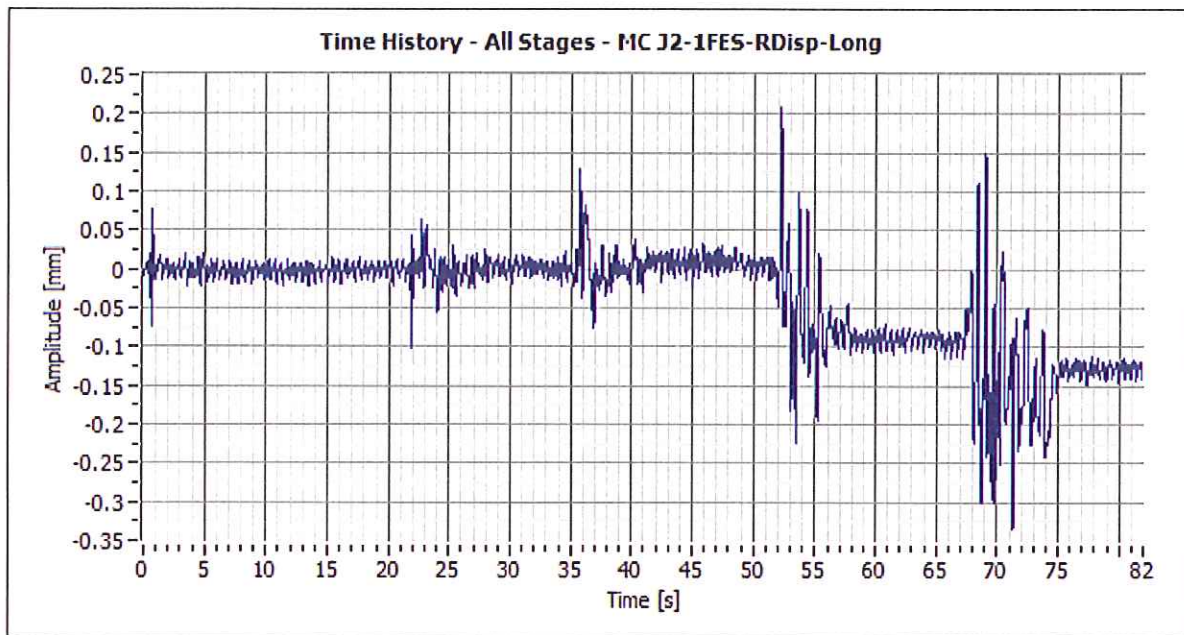


Figure N-24: Time History – Global Relative Displacement – Joint 2 SE

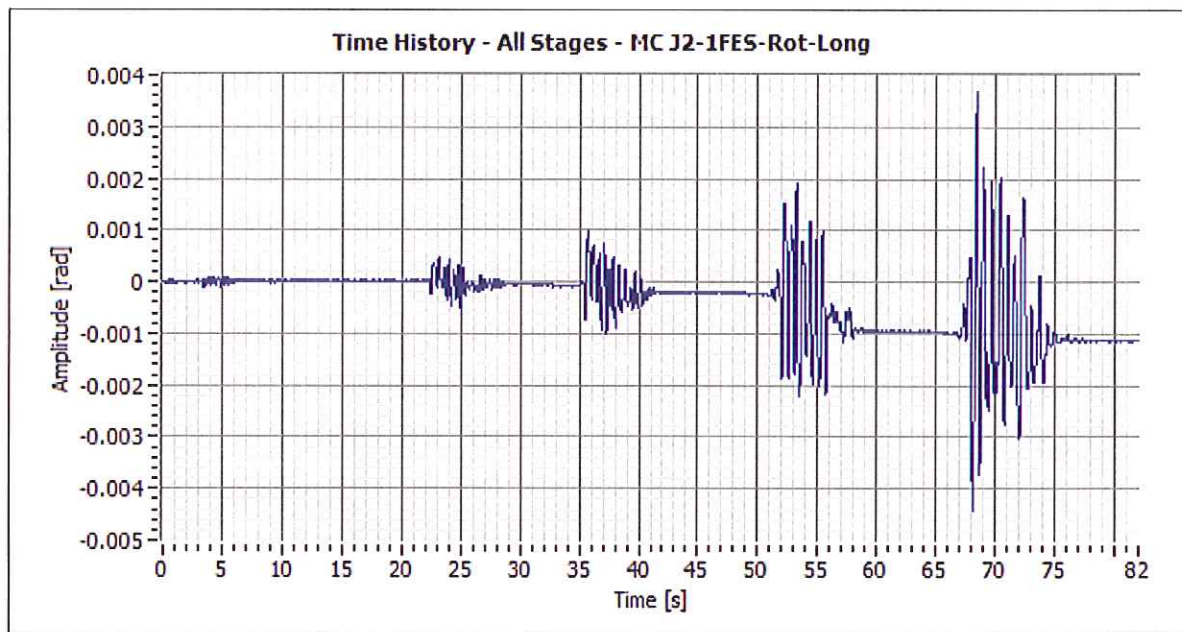


Figure N-25: Time History – Global Relative Rotation – Joint 2 SE

N.7 Joint 8

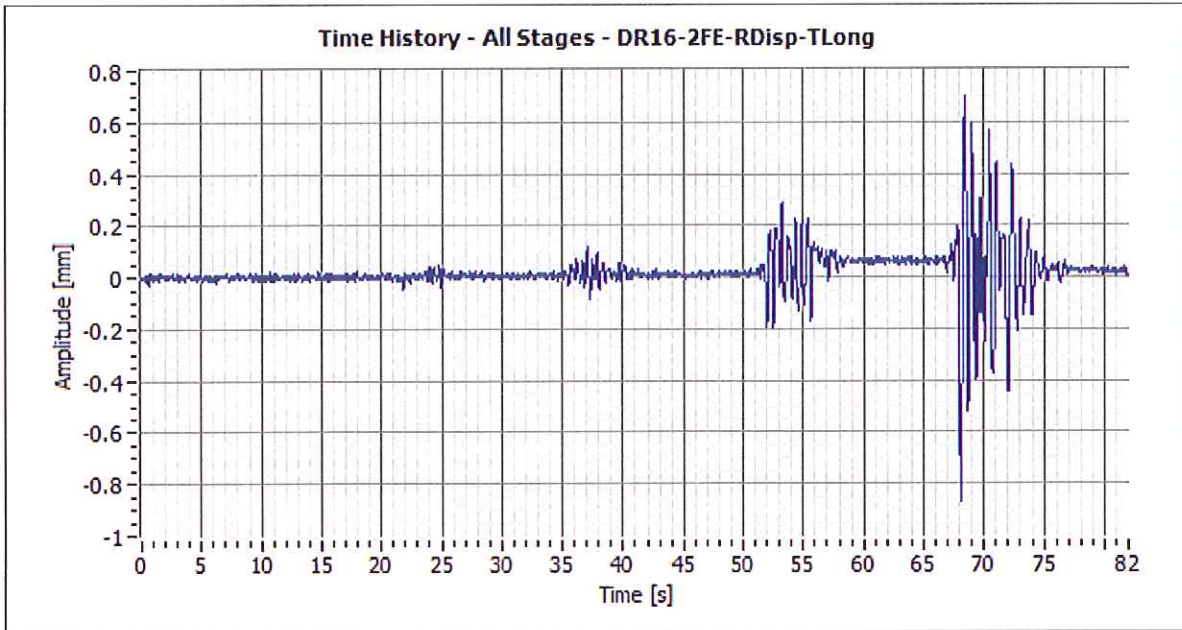


Figure N-26: Time History – Top Relative Displacement – Joint 8 NE

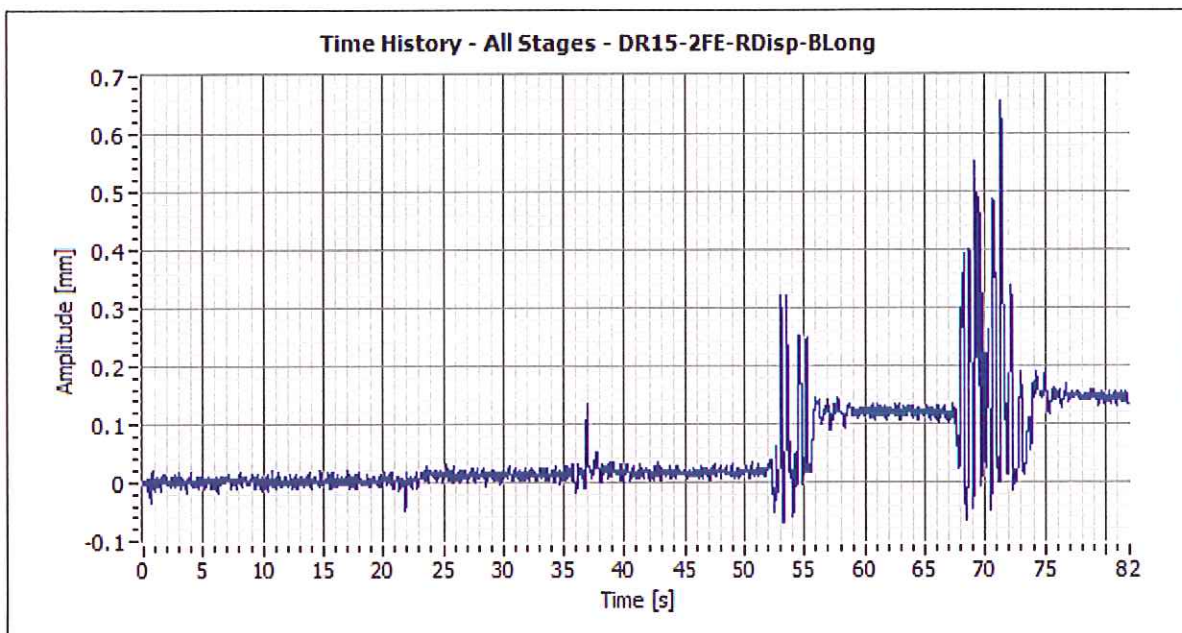


Figure N-27: Time History – Bottom Relative Displacement – Joint 8 NE

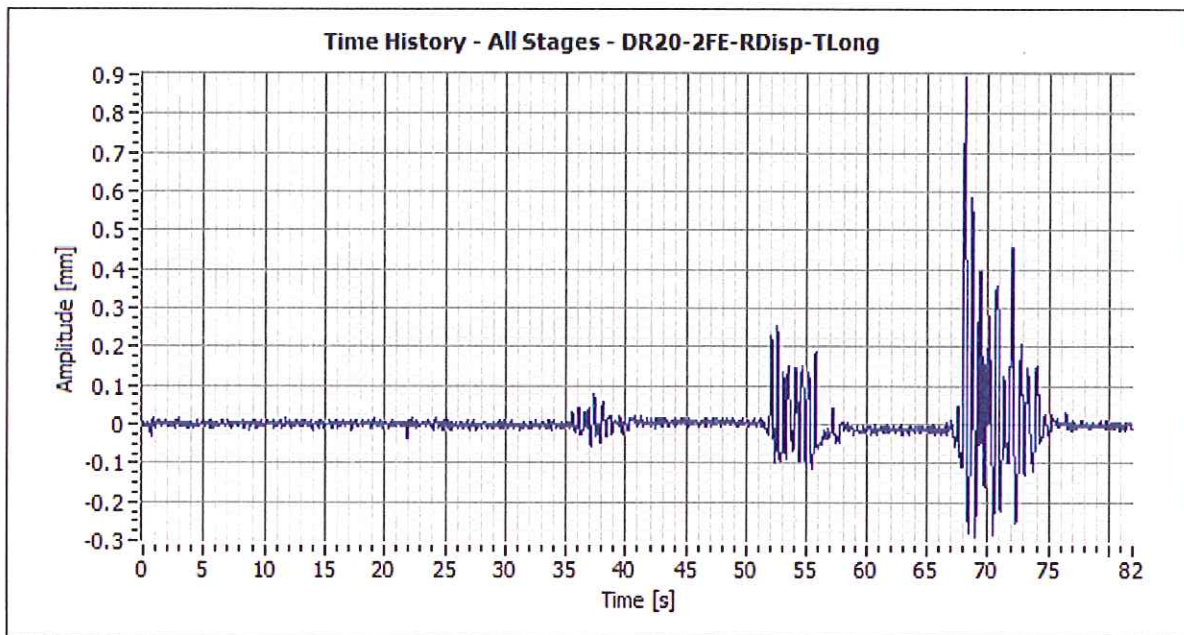


Figure N-28: Time History – Top Relative Displacement – Joint 8 SE

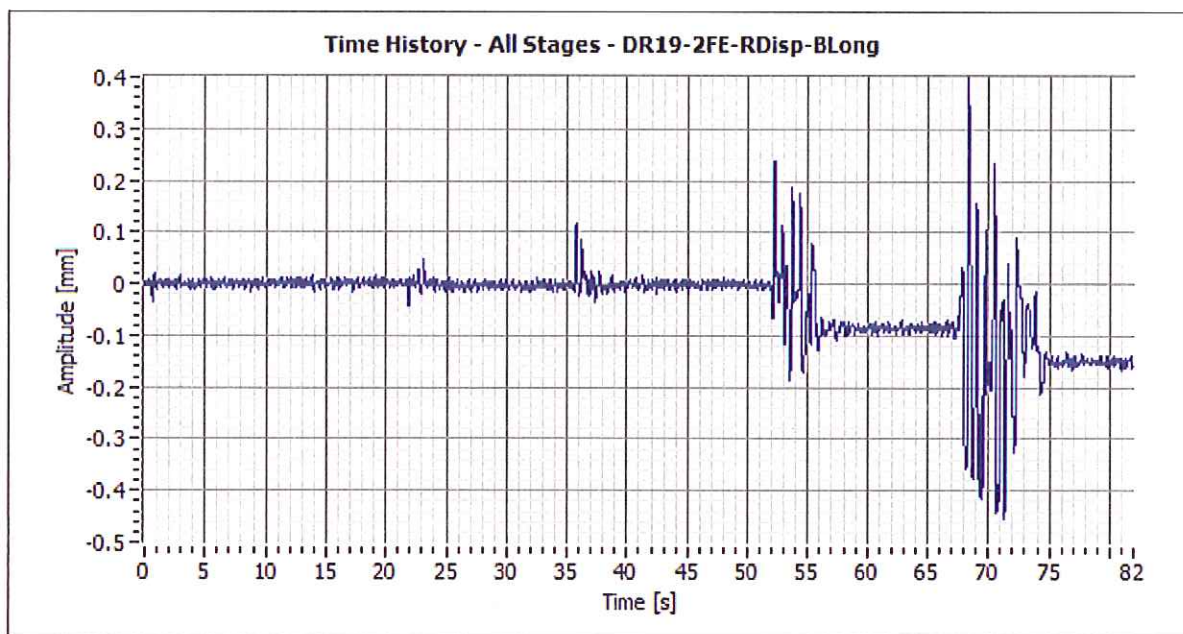


Figure N-29: Time History – Bottom Relative Displacement – Joint 8 SE

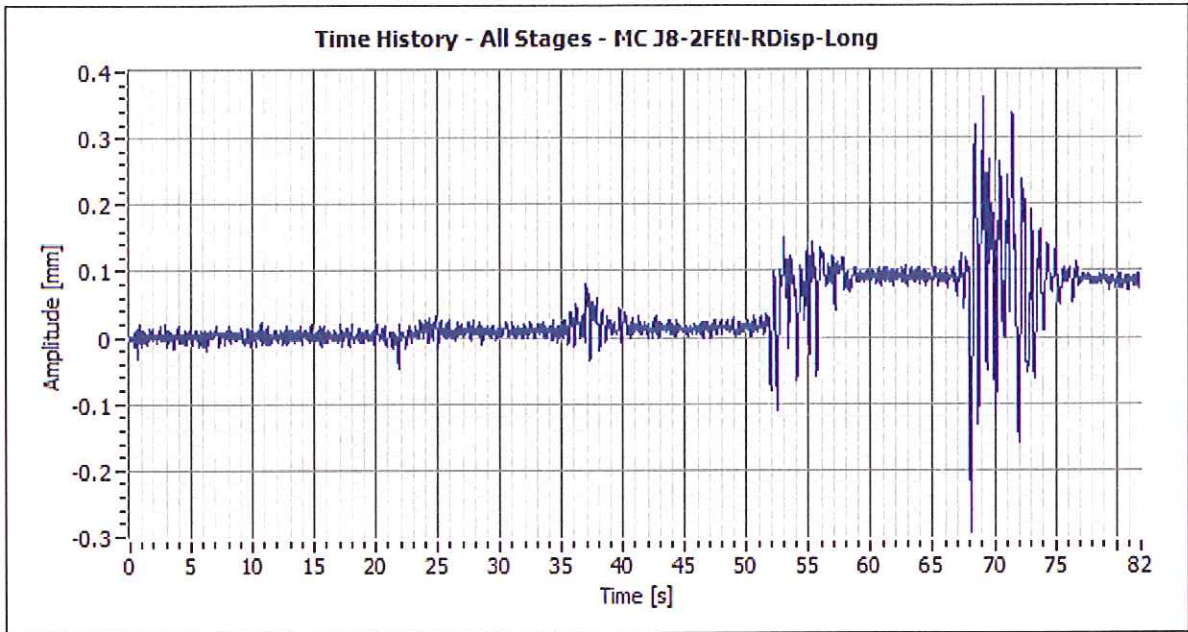


Figure N-30: Time History – Global Relative Displacement – Joint 8 NE

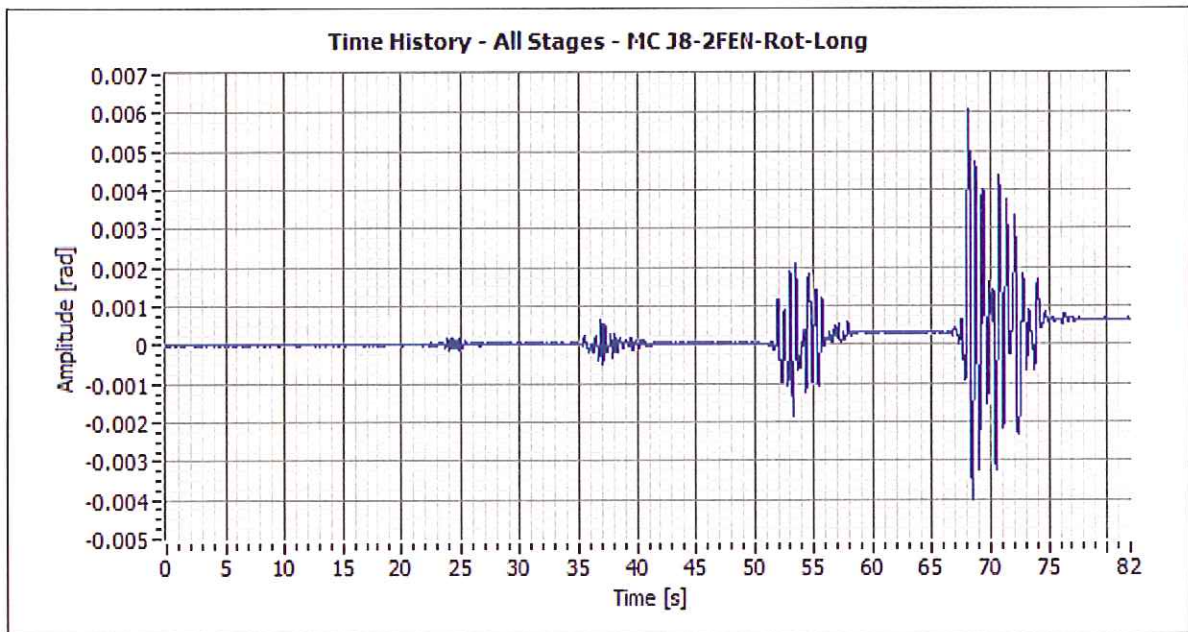


Figure N-31: Time History – Global Relative Rotation – Joint 8 NE

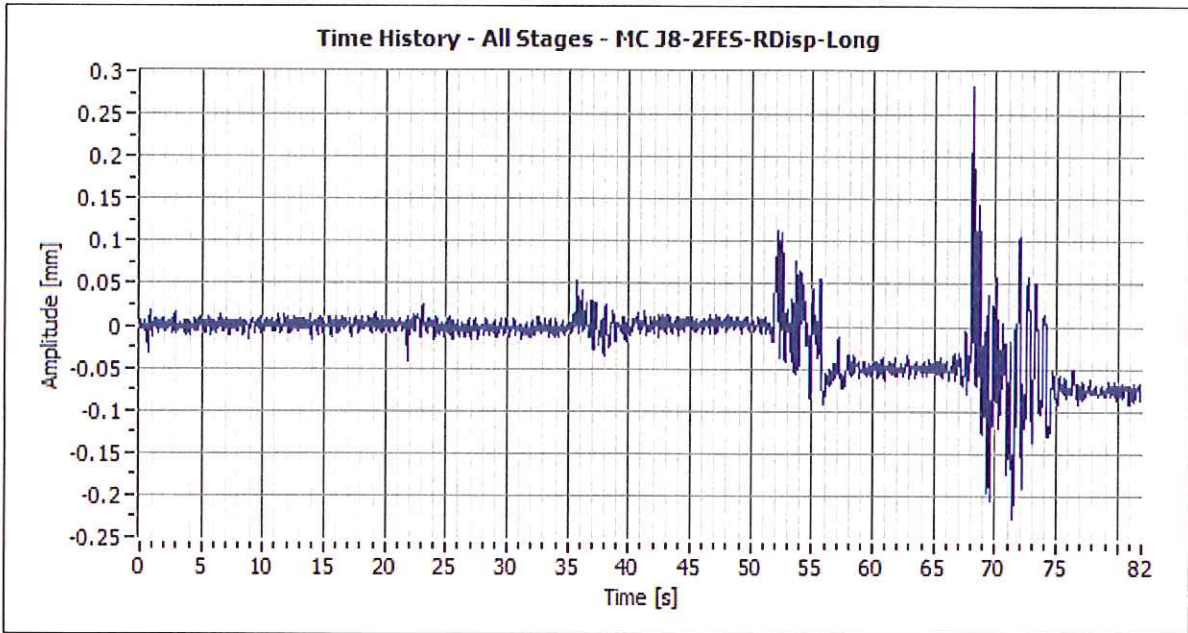


Figure N-32: Time History – Global Relative Displacement – Joint 8 SE

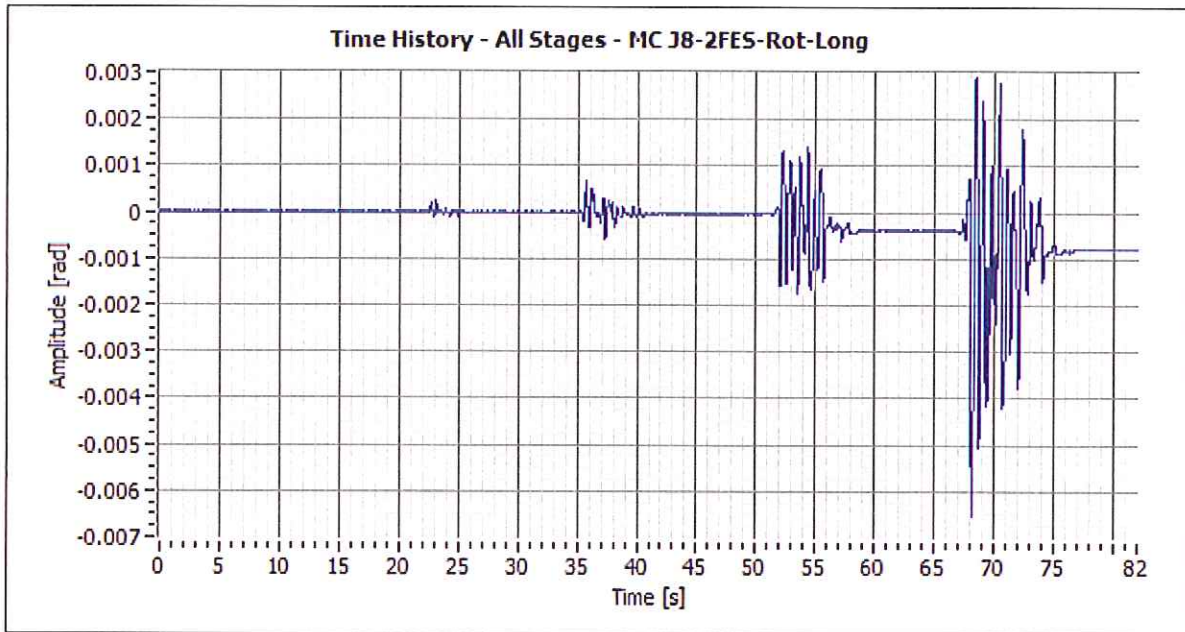


Figure N-33: Time History – Global Relative Displacement – Joint 8 SE

