

Movement in Masonry Walls Caused by Temperature and Moisture Changes

by

J.L. MIRANDA DIAS

LNEC (National Civil Engineering Laboratory - Portugal)

ABSTRACT

Movement in masonry walls of buildings is frequently associated with changes in temperature and moisture in the constituent materials and in the surrounding elements such as reinforced concrete beams, slabs and roofs. These changes also induce stresses in the walls that may lead to considerable damage in them and consequently affect the performance and durability of the building. The purpose of this paper is to discuss the factors that influence temperature and moisture movements in walls.

It is well known that masonry units (concrete units and clay bricks) usually expand when wet and shrink when dry. So, experimental tests were developed to assess the dimensional and weight variations of masonry unit specimens (with or without a mortar joint) subjected to moisture and temperature changes and to evaluate the behaviour of masonry walls when exposed to weathering which results in cyclic moisture content and dimensional changes to the wall.

The results of these tests reveal the significant contribution of the dimensional variation of masonry constituent materials to the overall movement of the wall.

1. INTRODUCTION

Changes in environment and in loading can cause significant movements in buildings. Usually the main causes of these movements are related to temperature and moisture changes, settlement of foundations, excessive deflection of supporting beams, elastic and time-dependent deformations (creep), vibration and chemical reactions within the materials [1] to [7] (see in Table 1, estimated variation of dimensional stability variables for two different types of masonry, [5]). These movements cannot be computed as a simple summation of each individual contributory effect, but are rather the result of a complex behaviour that is difficult to analyse.

With the rise or fall of temperature, the construction elements expand or contract, and due to the fact that these elements are wholly or partially restrained, stresses are consequently induced and can be considerable, even more than those from external loading. These changes of temperature may act uniformly on these elements; but under normal climatic conditions, the temperature gradient set up within the elements is usually non-linear and the induced internal stresses can be significant.

As mentioned above, another cause of movement in walls is associated with moisture change (so called "moisture movement") because masonry is a porous material. Moisture may enter in different ways such as vapour diffusion, air movement, rain penetration, and seepage. At low humidity, the migration of moisture in brickwork and block masonry is outwards, whereas at high humidity both types of masonry will absorb moisture. In practice, a wall is not totally saturated and usually stands in a relative humidity of between 50% and 80%. Besides causing movement in walls, the presence of moisture in the building envelope also affects the structural behaviour of building components through mechanical, chemical and possibly biological degradation. Damage induced by moisture is often related to efflorescence of masonry walls and corrosion of wall fixings. Excessive moisture in the envelope may also affect the health of occupants (by leading to an unhealthy indoor environment) and

can reduce the effectiveness of thermal insulation and damage aesthetic appearance. Therefore, moisture control in the building envelope is important for several reasons and implies both minimizing moisture entry into the building envelope and maximizing the exit of moisture that is present inside, so that the time for which the construction elements in the building remain wet can be limited.

Frequently, movement in masonry walls of buildings is associated with changes, both in temperature and in moisture, that occur in the constituent materials and in the surrounding elements such as reinforced concrete beams, slabs and roofs. Just as masonry materials are subject to expansion due to an increase in temperature, they are also subject to shrinkage as moisture is lost.

Moisture expansion or shrinkage presents a non-linear behaviour, whereas temperature contraction or expansion is directly proportional to the change of temperature (see Table 2 for typical values of moisture and linear thermal expansion, [5]).

These combined changes induce stresses in the walls that may considerably damage them (particularly in terms of cracking), and consequently affect the performance and durability of the building. According to fracture mechanics applied to masonry [6], it is tensile stress which is associated with the formation of visible cracks, whether the load is compressive (which induces tension transverse to the axial force) or shear (which induces a diagonal tensile stress). Excessive shrinkage in masonry units and mortar when drying after initial construction can also result in cracks. If masonry constituent materials are made with Portland cement, the shrinkage is related to water loss (reversible shrinkage) and carbonation (irreversible shrinkage). Initial shrinkage is an intrinsic property of construction materials such as concrete and cement-based masonry units.

The purpose of this paper is to discuss factors that can influence temperature and moisture movement in masonry walls, particularly those related to the essential properties of the constituent materials.

2. EXPERIMENTAL TESTS ON MASONRY MATERIALS

2.1 General

Concrete and brick expand when wet and shrink when dry. When exposed to weathering, cyclic changes may occur in moisture content and consequently dimensional changes may also happen. It has been shown by other work [6] that concrete masonry units, which had been previously dried to equilibrium with low humidity, and were subsequently sprayed on one face for two hours to simulate rain, re-expand by approximately one third of their original shrinkage from a saturated condition.

ASTM (1985) specifies for the maximum moisture content of concrete masonry units (CMU) values ranging from 25% to 45% depending on CMU shrinkage and annual relative humidity at the construction site [6]. Generally, unrestrained concrete wall shrinkage is most probably between the value of unrestrained shrinkage of the concrete masonry units and that of the mortar (usually this latter is greater than the CMU shrinkage). Total concrete masonry wall shrinkage may vary between 0.1mm/m and 10mm/m.

Experimental tests were developed at LNEC to assess the dimensional and mass variations of masonry block specimens and blockwork couplets with a mortar joint when subjected to moisture and temperature changes (temperature and moisture variation test). The main objective of these tests was to evaluate the significance of

dimensional variations of masonry constituent materials for the overall movement of the wall.

All of the figures for changes in mass and dimensions that follow in this section are mean values related to the properties of the units at the start of the test period.

2.2 Aerated autoclaved concrete blocks

The test aimed to evaluate the behaviour in terms of mass and dimensional variation of block material of autoclaved aerated concrete when subjected to a cyclic variation of moisture and temperature (two cycles - see Table 3). Six cubic specimens (approximate dimension - 100mm) of autoclaved aerated concrete (dry density - 560kg/m^3) were subjected to a period of drying (3 days in an oven at a constant temperature of 70°C and a relative humidity of about 20%) followed by a period of 3 days stored in laboratory conditions (closed ambient room - $20^\circ\text{C} \pm 5^\circ$ and relative humidity $45\% \pm 5\%$) and followed again by a period of immersion in water (7 days). The specimens were then stored in laboratory conditions for 1 day (the 14th day). The specimens were then subjected to a second cycle with a similar methodology to the first.

After the first three day period in the oven, there was a slight decrease in mass of the specimens (1.4%) but they almost returned to their initial mass by the end of the following period in laboratory conditions. In the same period, the specimens' dimensional change increased significantly, and on removal from the oven there was a 0.37mm/m contraction (Table 6). It then decreased significantly (i.e., it re-expanded) almost back to its original value. During the 7th day of the test (beginning of the immersion period), a strong increase of the mass (45%) and a dimensional deviation (0.28mm/m expansion) was detected. In the subsequent days up to the end of the 13th day (end of the immersion period), the mass variation showed a relatively moderate change (from a 45% to a 60% increase) and the dimensional variation almost stabilized. When there was a contraction of the specimens during their period in the oven (with peak value of 0.37mm/m after the 3rd day in the 1st cycle and 0.35mm/m after the 17th day in the 2nd cycle), the mass generally decreased (peak value of almost 1.5% after the 3rd day in the 1st cycle and a little over 1.5% after the 17th day in the 2nd cycle). The expansion of the specimens during their period of immersion in water (with a peak value of 0.28mm/m after the 13th day in the 1st cycle and 0.17mm/m after the 27th day in the 2nd cycle) was accompanied by a significant increase of mass (with a peak value of 59% after the 13th day in the 1st cycle and 58% after the 27th day in 2nd cycle).

With exception of the period of immersion in the 1st cycle (especially between the 8th and 13th days when the dimensional variation almost stabilized), the variation of the mean value of mass was generally accompanied by a significant dimensional variation of the specimens.

2.3 Bricks

The same methodology of testing was used to evaluate the behaviour in terms of mass and dimensional variation of solid brick material when subjected to a cyclic variation of moisture and temperature (Table 4). Six prismatic specimens of bricks ($100\text{mm} \times 100\text{mm} \times 65\text{mm}$) were tested in same way as described previously in 2.2.

In the first period (period in an oven), there was a slight increase in the mass of the specimens (0.2%) but this almost returned to the initial value after the period in laboratory conditions. In the same period, the specimens' dimensional change (measured in one direction, l_1 , for Z1-Z6, and in two directions, l_1 , l_2 , only for Z1-Z3) increased moderately, and after the 1st day was about 0.12mm/m (expansion - l_1), and then it decreased a little. During the 7th day of the test (beginning of the immersion period), an increase of the mass (12.4%) and dimensional change (0.15mm/m contraction) was detected. Up to the end of the immersion period (13th day), the mass variation showed a relatively small variation (13.0%) and the dimensional variation reduced a little (0.10mm/m after the 13th day).

2.4 Lightweight concrete blocks with expanded clay aggregate

The same test as described in 2.2 was carried out on six cubic specimens (approximate dimension - 100mm) aiming to assess the behaviour in terms of mass and dimensional variation of blocks made from lightweight concrete with expanded clay aggregate when there is a cyclic variation of conditions of moisture and temperature (Table 5). The blocks of lightweight concrete with expanded clay aggregate had a dry density of 1040kg/m^3 .

All the specimens were subjected to successive periods of drying in an oven at a constant temperature (70°C) followed by a period during which they remained in laboratory conditions and finally a period of immersion in water. After that period, the specimens were again placed in an oven and put through a second similar cycle.

During the first period (in the oven), a steady increase in the mass of the specimens (2.0% after the 1st day and 2.9% after the 3rd day) was detected. At the same time, the specimens' dimensional change increased very sharply in the first 24 hours (reaching 0.58mm/m expansion) and then decreased until end of the 6th day, when it reached a value of about 0.20mm/m (contraction). The mass of the specimens during the period in laboratory conditions decreased a little (2.3%) by the end of the 6th day. During the immersion period between the 6th and 13th day of the test, there was a moderate increase of mass (reaching 8.8% after the 13th day) and a small dimensional deviation (0.17mm/m expansion). During the immersion period, the mass variation showed a relative moderate variation after the 8th day (8.2% - 8.8%).

2.5 Lightweight concrete with expanded clay aggregates blocks with mortar joints (block-block and block-concrete)

The same characteristics were also investigated through tests using the same methodology as above on blockwork specimens. These were couplets built with a cement mortar joint (cube specimens of cement/sand mortar, 1:5, compressive strength = 5.16 N/mm^2). These tests aimed to investigate the behaviour of the mortar joint between blocks (Yj1-Yj2) or, to simulate joint contact between masonry and beams, floors or columns, between blocks and concrete elements (Yb1-Yb2). A total of four cubic specimens (approximate dimension of 100mm) were tested. Two of these were of lightweight concrete containing a mortar joint (Yj1, Yj2 - both halves of lightweight concrete, see Figure 5 and Table 5): the other two were composites of a normal concrete section joined to a lightweight concrete section (Yb1, Yb2 - see Figure 6 and Table 5). The deformation was measured in each specimen in two parallel directions, one in first half of the specimen, l_1 (in masonry block for Yj1 and Yj2, and concrete for Yb1 and Yb2), the other in the second half of the specimen, l_2 (masonry block).

In the first period (period in an oven), there was a slight increase in mass of specimens Yj1, Yj2, Yb1 and Yb2 similar to the increase in mass of the Y1-Y6 series. In same period, the specimens' dimensional variation increased significantly. The difference for Yb2 ($\Delta l_1 - \Delta l_2$) was positive with the exception of the initial period (1st and 2nd days), (Figure 4) and the concrete material was contracting less than block material. The change for Yj2 was, compared with Yb1-Yb2, a little higher in absolute values (see Figure 7). Both the Yj2 and the Yb1-Yb2 changes generally a more stable evolution than the dimensional variation of the Y1-Y6 series (see Table 5 and Figure 4). Between the 6th and 7th day of the test, an increase in mass was detected. For Yj1-Yj2, this variation of mass was more pronounced than the corresponding values of the Y1-Y6 series. These latter in turn were greater than the Yb1-Yb2 values. In the subsequent days, up to the end of the 13th day, the mass variation showed a relatively modest variation.

It is important to stress that, compared with the unrestrained shrinkage of mortar specimens, the horizontal shrinkage of mortar in the bed joints of masonry is considerably reduced (it can be about 50% less, [6]) due the effect of restraint by shear with the masonry units. Mortar joint cracking is increased when the fineness of the mortar sand is increased. The effective free movement is likely to be greater than that of concrete units since initial moisture loss will not

totally take place due to reversible movement (typical values lie between 0.3 and 0.6mm/m) after initial drying shrinkage (typical values are between 0.4 and 1.0mm/m - [6]). The shrinkage values of mortar depend upon its constituents.

3. MAIN CONCLUSIONS OF THE TESTS RESULTS

The results of the temperature and moisture variation test on series X1-X6 (autoclaved aerated concrete), Y1-Y6 (lightweight concrete with expanded clay aggregates) and Z1-Z3 (bricks), showed some common features between them as well as important differences in their behaviour. The same comments can be applied to specimens Yj1 and Yj2 (lightweight concrete with expanded clay aggregate block with mortar joint), and specimens Yb1 and Yb2 (lightweight concrete with expanded clay aggregate block-concrete with mortar joint).

From the results of the tests it can be concluded that the Y1-Y6 series (mean values) during "dry" periods loss their mass more than the other series (0-2nd days; 14th-16th days). During the "immersion" periods (6-13th days; 20th-27th days), X1-X6 series (mean values) increased their mass clearly more than the other series, especially with regard to Y series, although the Z series was very similar in this respect to the Y series.

In all these series, the difference between the two relative peaks of negative mass variation (1st cycle -13th day and 2nd cycle - 27th day) was not so significant; but a trend was detected in the specimens of each series towards the reduction of the peak in the 2nd cycle. Apart from that, the type of mass variation of these three series (X, Y and Z) showed a similar behaviour in the 2nd cycle when compared with that in the 1st cycle. This suggests an approximation to reversible behaviour from the point of view of mass variation during these two cycles.

The results showed that, generally, Y series (mean values) shrink during "dry" periods (0-2nd days and 14th-16th days) while the X and Z series expand during the same periods, even though in the second cycle the Z series did not reach positive values as the X series did. In the first immersion period (6th-13th days), the dimensional variation of the Z series exhibited a similar pattern to that of the X series; but a different type of progression was detected in the second dry period (20th-27th days) with a trend to a strong contraction of the specimens. From the type of dimensional variation shown by these three series, a slight tendency to a similar behaviour of X and Y series in the 2nd cycle when compared with those obtained in the 1st cycle can be found. However, for the Z series, a different type of trend during the two cycles was evident.

The difference of dimensional variation ($\Delta l_1/\Delta l_2$) of Yj2 (with mortar joint- block-block) was a bit larger in absolute values than that of Yb1-Yb2 (with mortar joint - block-concrete), and both generally followed the trend of variation of Y1-Y6, although with higher absolute values (see Figure 7).

The results of these tests revealed the differences of dimensional variations, and the associated mass variations, of each type of masonry block material. That fact indicates a potential influence of the dimensional variations of masonry constituent materials in the overall movement of the different types of wall.

ACKNOWLEDGMENTS

LNEC Programmed Research (2001-2004) has funded the present study. The assistance and help in the test programme of Mr. Deodato Sanches and Mr. Torcato Duarte is gratefully acknowledged.

REFERENCES

1. COMMISSION OF THE EUROPEAN COMMUNITIES (CEC) - *Eurocode n°6: Common unified rules for masonry structures*. CEN TC/250/SC6, June 1995.
2. ORGANIZATION INTERNATIONALE DE NORMALIZATION (ISO) - *Bases du calcul des constructions - Déformations des bâtiments à l'état limite d'utilisation*. Norme ISO 4356, Genève, ISO, 1977.
3. ROBERTS, J.; TOVEY, A.; FRIED, A. - *Concrete masonry, Designers handbook*. Dec. 1999.
4. MIRANDA DIAS, J.L. - *Composite action between lightweight concrete masonry blocks and their supporting beams - PhD Thesis, IST*. LNEC, July, 1997.
5. GRIM, C.T. - *Differential movement in composite load-bearing masonry walls*. Journal of Structural Division Engineer, ASCE, No ST7, Proc. Paper 14666, July, 1977, pp.1277-1288.
6. GRIM - *Masonry Cracks: A review of the Literature*. Symposium of ASTM - Masonry: Materials, Design, Construction and Maintenance, STP 992 Dec. 1986.
7. LENCZNER, D - *Movements in loadbearing masonry walls - Third International Seminar on Structural masonry for developing countries, Mauritius, July 1990*.

Table 1
Estimated variation of dimensional stability variables [5]

	Brick Masonry						Hollow concrete masonry					
	Modulus of elasticity (MPa x 10 ³)	Elastic strain (mm/m)	Creep (mm/m)	Moisture expansion (mm/m)	Temperature change (°C)	Thermal dimensional change (mm/m)	Modulus of elasticity (MPa x 10 ³)	Elastic strain (mm/m)	Creep (mm/m)	Moisture expansion (mm/m)	Temperature change (°C)	Thermal dimensional change (mm/m)
Minimum	2.57	0	0	0	0	0	2.07	0	0	0	0	0
Maximum	29.67	-0.472	-0.471	+0.486	62.2°	0.773	8.28	-0.80	-2.40	0	42.7°	+0.578
Mean	11.73	-0.176	-0.088	+0.186	37.2°	0.228	17.25	-0.25	-0.438	0	17.8°	+0.138

Table 2
Typical values of moisture and linear thermal expansion [5]

Materials				
Dense concrete (gravel aggregate)	Moisture expansion- dry to saturated (mm/m)		Linear thermal expansion	
	minimum reference value	maximum reference value	Coefficient of expansion per °C x 10 ⁻⁶	Expansion mm/m (T=100°C)
Light aggregate concrete	0.2	0.6	11.7	1.17
Ultra-Light aggregate concrete	0.3	0.6	8.3	0.83
Limestone (average)	1	2	-	-
Sandstone	0.1	0.1	2.5-9.0	0.25-0.90
Brick (clay)	0.5	0.8	7.0-16.0	0.70-1.60
Brick (sand-lime)	0.03	0.2	5.0	0.5
Mortar	0.1	0.5	-	-
Glass reinforced cement			3-11	
Timber (perp. to the grain)	1.5	2.5	7.0-12.0	0.70-1.20
Timber (along the grain)	20	80	50-60	5.0-6.0
Aluminium	0.5	1	3.8-6.5	0.38-0.65
Stainless steel	-	-	23.5	2.35
Structural steel	-	-	18.0	1.80
Granite	-	-	12.1	1.21
Marble	-	-	8.5-11.0	0.85-1.10
Glass	-	-	13.2	1.32

Table 3
Temperature and moisture variation test (specimens of autoclaved aerated concrete - X1 a X6)

Temp. (°C)	Mass value and mass and dimensional variation (relative to the initial value)																
	Period in oven			Lab. cond.	Immersion in water			Lab. cond.	Period in oven			Lab. Cond.	Immersion in water			Lab. conditions /end of test	
Time Days	0°D	1°D	2°D	3°D	6°D	7°D	8°D	13°D	14°D	15°D	16°D	17°D	20°D	21°D	22°D	27°D	28°D
X1-m	587	579	579	579	587	872	903	962	937	639	594	578	586	876	904	947	919
Δm/m	0	0.0136	0.0136	0.0136	0	-0.4855	-0.5383	0.6388	0.5963	-0.08859	-0.01193	0.01533	0.00170	0.49233	0.54003	0.61329	-0.56559
Δl/l	0	-0.2	-0.3	-0.35	0	0.2	0.2	0.2	0.15	-0.2	-0.2	-0.6	-0.55	-0.3	-0.35	0	0.15
X2-m	618	609	609	609	617	900	928	979	957	676	628	609	616	908	931	966	943
Δm/m	0	0.0146	0.0146	0.0146	0.0016	-0.4563	-0.5016	0.5841	0.5485	-0.09385	-0.01618	0.01456	0.00324	0.46926	0.50647	0.56311	-0.52589
Δl/l	0	-0.3	-0.4	-0.45	-0.05	0.2	0.2	0.25	0.2	-0.1	-0.1	-0.35	-0.4	-0.2	-0.25	0.1	0.2
X3-m	619	610	610	610	618	894	924	967	938	669	625	609	617	902	933	959	933
Δm/m	0	0.01454	0.01454	0.01454	0.0016	-0.4443	-0.4927	0.5622	0.5153	-0.08078	-0.00969	0.01616	0.00323	0.45719	0.50727	0.54927	-0.50727
Δl/l	0	-0.5	-0.25	-0.3	0.05	0.25	0.25	0.25	0.2	0.05	0.15	-0.15	-0.2	0.05	0	0.3	0.2
X4-m	592	583	583	583	591	865	895	947	923	649	601	583	590	882	911	944	916
Δm/m	0	0.01520	0.01520	0.01520	0.0017	-0.4611	-0.5118	0.5997	0.5591	-0.09628	-0.01520	0.01520	0.00338	0.48987	0.53885	0.59460	0.547297
Δl/l	0	-0.3	-0.3	-0.35	0.1	0.35	0.35	0.35	0.3	-0.05	-0.05	-0.4	-0.35	-0.1	-0.15	0.2	0.3
X5	612	603	603	603	610	872	909	957	936	669	621	603	609	888	919	948	928
Δm/m	0	0.01471	0.01471	0.01471	0.0033	-0.4248	-0.4853	0.5637	0.5294	-0.09314	-0.01471	0.01471	0.00490	0.45098	0.50163	0.54902	-0.51634
Δl/l	0	-0.3	-0.35	-0.4	0.1	0.35	0.35	0.35	0.3	0	0	-0.3	-0.25	-0.1	-0.15	0.2	0.3
X6	598	590	589	589	597	870	913	978	950	652	604	588	596	893	919	957	933
Δm/m	0	0.01338	0.01505	0.01505	0.0017	-0.4548	-0.5268	0.6355	0.5887	-0.09030	-0.01003	0.01672	0.00335	0.49331	0.53679	0.60033	-0.56020
Δl/l	0	-0.25	-0.3	-0.35	0.05	0.3	0.3	0.3	0.2	0.05	0.1	-0.3	-0.2	0	-0.05	0.2	0.2

Values of m (mass) in grams; Δm/m (mass relative variation) in % ; Δl/l (dimensional variation) in mm/m

Table 4
Temperature and moisture variation test (specimens of bricks - Z1 to Z6)

Temp (°C)	Mass value and mass and dimensional variation (relative to the initial value)																	
	Period in oven			Lab. cond.	Immersion in water				Lab. Cond.	Period in oven			Lab. cond.	Immersion in water			Lab. conditions /end of test	
	0°D	1°D	2° D	3°D	6°D	7°D	8°D	13°D	14° D	15°D	16°D	17°D	20°D	21° D	22°D	27°D	28°D	
Z1-m	1435	1432	1432	1432	1434	1607	1609	1616	1577	1436	1432	1432	1434	1601	1610	1614	1553	
$\Delta m/m$	0	0.0021	0.0021	0.0021	0.0007	-0.1199	-0.1213	-0.1261	-0.0990	-0.0007	0.0021	0.0021	0.0007	-0.1157	-0.1220	-0.1247	-0.0822	
$\Delta l/l_1$	0	0	0	-0.1	-0.4	-0.3	-0.2	-0.2	-0.4	-0.1	-0.3	-0.4	-0.7	-0.8	-0.9	-0.6	-0.6	
$\Delta l/l_2$	0	0	0	0	-0.3	-0.1	0	0	-0.3	-0.3	-0.1	-0.2	-0.5	-0.7	-0.7	-0.5	0	
Z2-m	1211	1209	1209	1209	1211	1363	1365	1370	1344	1214	1209	1209	1211	1364	1366	1368	1325	
$\Delta m/m$	0	0.0017	0.0017	0.0017	0	-0.1255	-0.1272	-0.1313	-0.1098	-0.0025	0.0017	0.0017	0	-0.1263	-0.1280	-0.1296	-0.0942	
$\Delta l/l_1$	0	0.1	0.1	0.1	-0.2	*	-	-	-	-	-	-	-	-	-	-	-	
$\Delta l/l_2$	0	0.2	0.1	0.1	-0.3	-0.1	-0.1	-0.1	-0.3	0.2	0.2	0	-0.3	-0.4	-0.4	-0.2	0	
Z3-m	1421	1418	1418	1418	1421	1602	1603	1609	1578	1427	1419	1419	1421	1602	1604	1609	1559	
$\Delta m/m$	0	0.0021	0.0021	0.0021	0	-0.1274	-0.1281	-0.1323	-0.1105	-0.0042	0.0014	0.0014	0	-0.1274	-0.1288	-0.1323	-0.0971	
$\Delta l/l_1$	0	0.1	0	0	*	-	-	-	-	-	-	-	-	-	-	-	-	
$\Delta l/l_2$	0	0.2	0.2	0.2	-0.2	0	0	0.1	-0.2	*	-	-	-	-	-	-	-	
Z4-m	1407	1404	1404	1404	1406	1578	1580	1585	1555	1407	1404	1404	1399	1579	1580	1586	1541	
$\Delta m/m$	0	0.0021	0.0021	0.0021	0.0007	-0.1215	-0.1230	-0.1265	-0.1052	0	0.0021	0.0021	0.0057	-0.1222	-0.1230	-0.1272	-0.0953	
$\Delta l/l_1$	0	0.1	0.1	0.1	-0.2	*	-	-	-	-	-	-	-	-	-	-	-	
Z5	1370	1367	1367	1367	1370	1547	1548	1553	1520	1376	1367	1367	1369	1547	1551	1553	1509	
$\Delta m/m$	0	0.0022	0.0022	0.0022	0	-0.1292	-0.1299	-0.1336	-0.1095	-0.0044	0.0022	0.0022	0.0007	-0.1292	-0.1321	-0.1336	-0.1015	
$\Delta l/l_1$	0	0.2	0.2	0.2	-0.2	0	0	0	-0.3	0	-0.1	-0.2	-0.5	-0.5	-0.5	-0.5	-0.4	
Z6	1402	1398	1398	1398	1401	1582	1584	1588	1556	1407	1399	1399	1394	1583	1584	1588	1534	
$\Delta m/m$	0	0.0029	0.0028	0.0029	0.0007	-0.1234	-0.1299	-0.1327	-0.1098	-0.0036	0.0022	0.0022	0.0057	-0.1291	-0.1298	-0.1327	-0.0942	
$\Delta l/l_1$	0	0.2	0.2	0.2	*	-	-	-	-	-	-	-	-	-	-	-	-	

*The measurement of dimensional variation of Z specimens was troubled by the fact that during the test some of the metal settings became unglued from the surface of the specimen; at that point, the measurement of the variable was discontinued

Table 5

Temperature and moisture variation test - Y1-Y6 series (lightweight concrete blocks with expanded clay aggregate) and Yj1,Yj2 (with mortar joint- block/block), Yb1,Yb2 (with mortar joint - block/concrete) specimens

Temp. (°C)	Mass value and mass and dimensional variation (relative to the initial value)																
	Period in oven			Lab. cond.	Immersion in water				Lab. cond.	Period in oven				Lab. Cond.	Immersion in water		
Time Days	0°D	1°D	2° D	3°D	6°D	7°D	8°D	13°D	14° D	15°D	16°D	17°D	20°D	21° D	22°D	27°D	28°D
Y1-m	1178	1153	1145	1141	1148	1272	1280	1286	1244	1178	1158	1150	1155	1271	1273	1280	1226
$\Delta m/m$	0	0.02122	0.02801	0.03140	0.02547	-0.07980	-0.08659	-0.091681	-0.05603	0	0.0170	0.0238	0.01953	-0.0790	-0.0807	-0.0866	-0.0408
$\Delta l/l$	0	0.3	0.3	0.3	-0.2	0	-0.2	0	0.15	0.5	0.6	0.3	-0.2	0	0	0.15	0.15
Y2-m	1128	1106	1100	1098	1105	1238	1240	1253	1193	1125	1108	1102	1108	1238	1239	1245	1183
$\Delta m/m$	0	0.01950	0.02482	0.02659	0.02039	-0.09751	-0.09929	-0.11082	-0.05762	0.0027	0.0177	0.0230	0.0177	-0.0975	-0.0984	-0.1037	-0.0488
$\Delta l/l$	0	0.5	0.4	0.4	-0.3	0.2	-0.3	0.1	0.2	0.6	0.6	0.4	-0.3	0.2	0.1	0.3	0.3
Y3-m	1186	1162	1154	1150	1157	1277	1288	1291	1248	1188	1168	1159	1164	1277	1281	1290	1239
$\Delta m/m$	0	0.02023	0.02698	0.03035	0.02445	-0.07672	-0.08600	-0.08853	-0.05227	-0.0017	0.0152	0.0228	0.0186	-0.0767	-0.0801	-0.0877	-0.0447
$\Delta l/l$	0	0.5	0.4	0.4	-0.1	0.1	-0.1	0.3	0.2	0.5	0.5	0.4	-0.1	0.1	0	0.1	0.1
Y4-m	1236	1214	1206	1201	1208	1308	1323	1330	1298	1243	1223	1214	1218	1311	1316	1329	1290
$\Delta m/m$	0	0.01779	0.02427	0.02831	0.02265	-0.05825	-0.07038	-0.07605	-0.05016	-0.0057	0.0105	0.0178	0.0146	-0.0607	-0.0647	-0.0752	-0.0437
$\Delta l/l$	0	0.8	0.7	0.6	-0.2	0.2	-0.2	0.2	0.3	0.8	0.8	0.6	-0.2	0.2	0.2	0.3	0.3
Y5	1281	1257		1246	1252	1345	1359	1365	1338	1284	1266	1258	1262	1349	1356	1366	1335
$\Delta m/m$	0	0.01873	0.02341	0.02732	0.02263	-0.04996	-0.06089	-0.06557	-0.04449	-0.0023	0.0117	0.0180	0.0148	-0.0531	-0.0586	-0.0664	-0.0422
$\Delta l/l$	0	0.6	0.4	0.4	-0.3	0	-0.3	0.2	0.2	0.8	0.9	0.6	-0.1	0.2	0.2	0.3	0.3
Y6	1172	1149	1141	1137	1145	1266	1273	1280	1243	1175	1155	1147	1152	1264	1269	1278	1228
$\Delta m/m$	0	0.01962	0.02645	0.02986	0.02303	-0.08020	-0.08617	-0.09215	-0.06058	-0.0026	0.0145	0.0213	0.0171	-0.0785	-0.0828	-0.0904	-0.0478
$\Delta l/l$	0	-0.25	-0.3	-0.35	0.05	0.3	0.3	0.3	0.2	0.6	0.6	0.4	-0.3	0	0.1	0.1	0.1
yj1-m	1316	1306	1305	1304	1310	1431	1435	1453	1402	1315	1305	1304	1309	1429	1433	1447	1374
$\Delta m/m$	0	0.00799	0.00835	0.00911	0.00455	-0.08738	-0.09042	-0.10410	-0.06535	0.00076	0.0084	0.0091	0.0053	-0.0859	-0.0889	-0.0995	-0.0441
$\Delta l/l_1$	*0	0.4	-0.2	-0.2	-0.5	-0.5	-0.4	-0.5	-0.5	-0.2	-0.4	-0.6	-0.9	-0.8	-0.7	-0.7	0
$\Delta l/l_2$	0	0	-0.1	-0.1	-0.6	-0.4	-0.3	-0.4	-0.5	-0.2	-0.4	-0.6	-0.9	-0.9	-0.8	-0.8	-0.8
$\Delta l/l_1-$	0	0	-0.1	-0.1	0.1	-0.1	-0.1	-0.1	0	0	0	0	0	0.1	0	0.1	0.1
yj2-m	1302	1292	1291	1291	1297	1427	1427	1450	1395	1298	1291	1290	1295	1425	1430	1442	1358
$\Delta m/m$	0	0.00845	0.00844	0.00844	0.00384	-0.0960	-0.09600	-0.11367	-0.07142	0.00307	0.0085	0.0092	0.0054	-0.0945	-0.0983	-0.1075	-0.0430
$\Delta l/l_1$	0	0.2	-0.5	-0.3	-1.0	-0.8	-0.7	-0.7	-0.8	-0.5	-0.6	-0.8	-1.2	-1.1	-1.1	-1.1	0
$\Delta l/l_2$	0	0.6	0.4	0.4	1.0	0.1	0.2	0.2	0.1	0.4	0.2	0	-0.3	-0.3	-0.3	-0.2	-0.2
$\Delta l/l_1-$	0	-0.4	-0.9	-0.7	-1.0	-0.9	-0.9	-0.9	-1.0	-0.9	-0.8	-0.8	-0.9	-0.8	-0.8	-0.9	-0.9
yb1-m	1871	1861	1858	1856	1861	1978	1977	1989	1943	1879	1866	1862	1865	1977	1978	1986	1925
$\Delta m/m$	0	0.00692	0.00694	0.00801	0.00534	-0.05718	-0.05665	-0.06306	-0.03848	-0.00427	0.0027	0.0048	0.0032	-0.0567	-0.0572	-0.0615	-0.0289
$\Delta l/l_1$	0	0	0	-0.3	-0.7	-0.5	-0.4	-0.3	-0.5	-0.2	-0.2	-0.4	-0.9	-0.8	-0.7	-0.8	0
$\Delta l/l_2$	0	0.2	-0.5	-1.0	-1.0	-0.9	-1.2	-1.2	-1.2	-0.9	-0.9	-1.2	-1.6	-1.6	-1.6	-1.5	-1.5
$\Delta l/l_1-$	0	-0.2	0.5	0.7	0.3	0.4	0.8	0.9	0.7	0.7	0.7	0.8	0.7	0.8	0.8	0.8	0.7
yb2-m	1858	1847	1844	1842	1847	1955	1959	1968	1918	1866	1853	1849	1852	1955	1959	1966	1905
$\Delta m/m$	0	0.00754	0.00753	0.00861	0.00592	-0.05220	-0.05436	-0.05920	-0.03229	-0.00430	0.0027	0.0048	0.0032	-0.0522	-0.0544	-0.0582	-0.0253
$\Delta l/l_1$	0	0	0.2	0	-0.5	-0.3	-0.3	-0.4	-0.4	-0.1	-0.1	-0.3	-0.8	-0.7	-0.6	-0.6	0
$\Delta l/l_2$	0	0.1	0.2	-0.3	-0.9	-0.4	-0.5	-0.7	-0.7	-0.4	-0.4	-0.7	-1.2	-1.2	-1.1	-1.0	-1.0
$\Delta l/l_1-$	0	-0.1	0	0.3	0.4	0.1	0.2	0.3	0.3	0.3	0.3	0.4	0.4	0.5	0.5	0.4	0.4

*The measuring of dimensional variation l_1 of Yj1 specimen on the 1st day was troubled by the fact that during the test some of the metal settings became unglued from the surface of the specimen (generally, measuring the dimensional variation l_1 , l_2 of Yj1, Yj2 Yb1, Yb2, specimens was a difficult task and the corresponding precision was lower than that obtained in the other series); at that time the metal settings were glued on again and the test was restarted (the initial value relative to the end of the 1st day)

Table 6

Summary of test results: mean values for X1-X6 (autoclaved aerated concrete), Y1-Y6 (lightweight concrete with expanded clay aggregate), Z1-Z3 (bricks) series and Yj1-Yj2 (lightweight concrete block with expanded clay aggregate with mortar joint), Yb1-Yb2 (lightweight concrete block with expanded clay aggregate-concrete with mortar joint)

Temp. (°C)	Mass and dimensional variation (relative to the initial value)																
	Period in oven			Lab. condit.	Immersion in water				Lab. condit.	Period in oven				Lab. condit.	Immersion in water		
Time Days	0°D	1°D	2° D	3°D	6°D	7°D	8°D	13°D	14° D	15°D	16°D	17°D	20°D	21° D	22°D	27°D	28°D
X1-X6																	
$\Delta m/m$	0	0.01434	0.01462	0.01462	0.00164	-0.45449	-0.50943	-0.59734	-0.5562	-0.0905	0.0130	0.0154	0.0033	-0.4735	-0.5218	-0.5783	-0.5371
$\Delta l/l$	0	-0.27	-0.32	-0.37	0.04	0.28	0.28	0.28	0.23	-0.04	-0.02	-0.35	-0.33	-0.11	-0.16	0.17	0.23
Y1-Y6																	
$\Delta m/m$	0	0.0195	0.0257	0.0290	0.0230	-0.0737	-0.0816	-0.0875	-0.0535	-0.0016	0.01444	0.02111	0.01704	-0.07443	-0.0775	-0.0850	-0.0446
$\Delta l/l$	0	0.58	0.48	0.45	-0.2	0.12	-0.2	0.17	0.23	0.63	0.67	0.45	-0.2	0.12	0.1	0.21	0.21
Z1-Z6																	
$\Delta m/m$	0	0.0022	0.0022	0.0022	0.0004	-0.1245	-0.1265	-0.1304	-0.1073	-0.0026	0.0019	0.0019	0.0021	-0.1250	-0.1273	-0.1300	-0.0941
$\Delta l_1/l$	0	0.12	0.1	0.08	-0.24	-0.15	-0.1	-0.1	-0.35	-0.05	-0.2	-0.3	-0.6	-0.65	-0.7	-0.55	-0.5
$\Delta l_2/l$	0	0.13	0.1	0.1	-0.27	-0.07	-0.03	0	-0.27	-0.05	0.05	-0.1	-0.4	-0.55	-0.55	-0.35	0
Yj1-2																	
$\Delta m/m$	0	0.00807	0.00840	0.00878	0.00420	-0.09170	-0.09322	-0.10889	-0.0683	-0.0011	0.00554	0.00700	0.00428	-0.07230	-0.07469	-0.08167	-0.0353
$\Delta l/l_1$	0*	-0.4	-0.9	-0.7	-1.0	-0.9	-0.9	-0.9	-1.0	-0.9	-0.8	-0.8	-0.9	-0.8	-0.8	-0.9	-0.9
Yb1-2																	
$\Delta m/m$	0	0.00724	0.00724	0.00831	0.00563	-0.05470	-0.05551	-0.06114	-0.0354	-0.0043	0.00268	0.00483	0.00322	-0.05443	-0.05577	-0.05980	-0.0270
$\Delta l/l_1$	0	-0.15	0.25	0.5	0.35	0.25	0.5	0.6	0.5	0.5	0.5	0.6	0.55	0.65	0.65	0.6	0.55

* results relative to Yj2 only

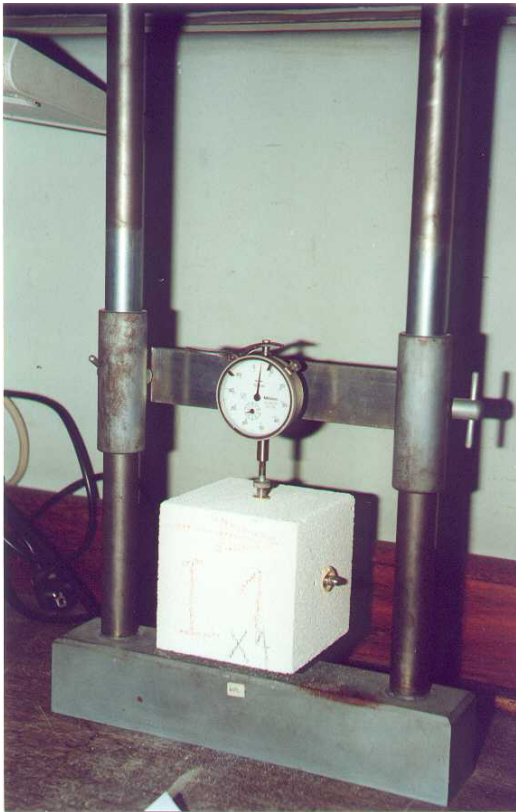


Figure 1-Measuring dimensional variation of X series specimen (autoclaved aerated concrete)

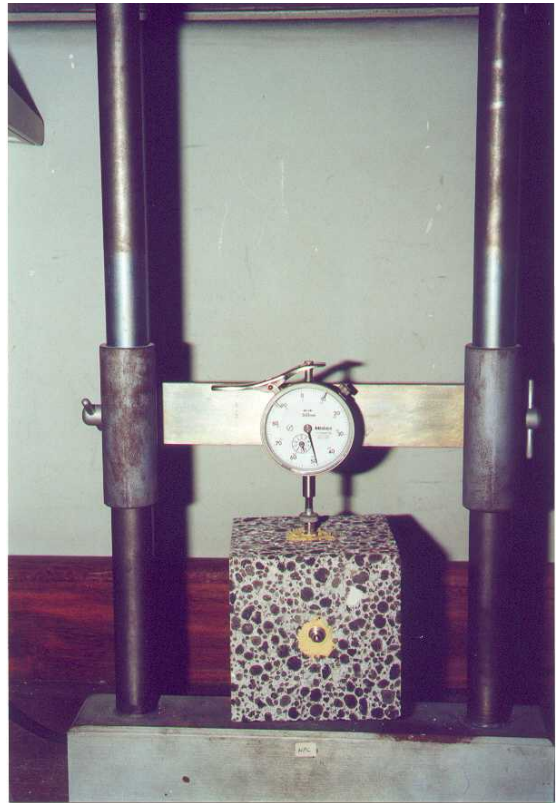


Figure 2-Measuring dimensional variation of Y series specimen (lightweight concrete with expanded clay aggregates)

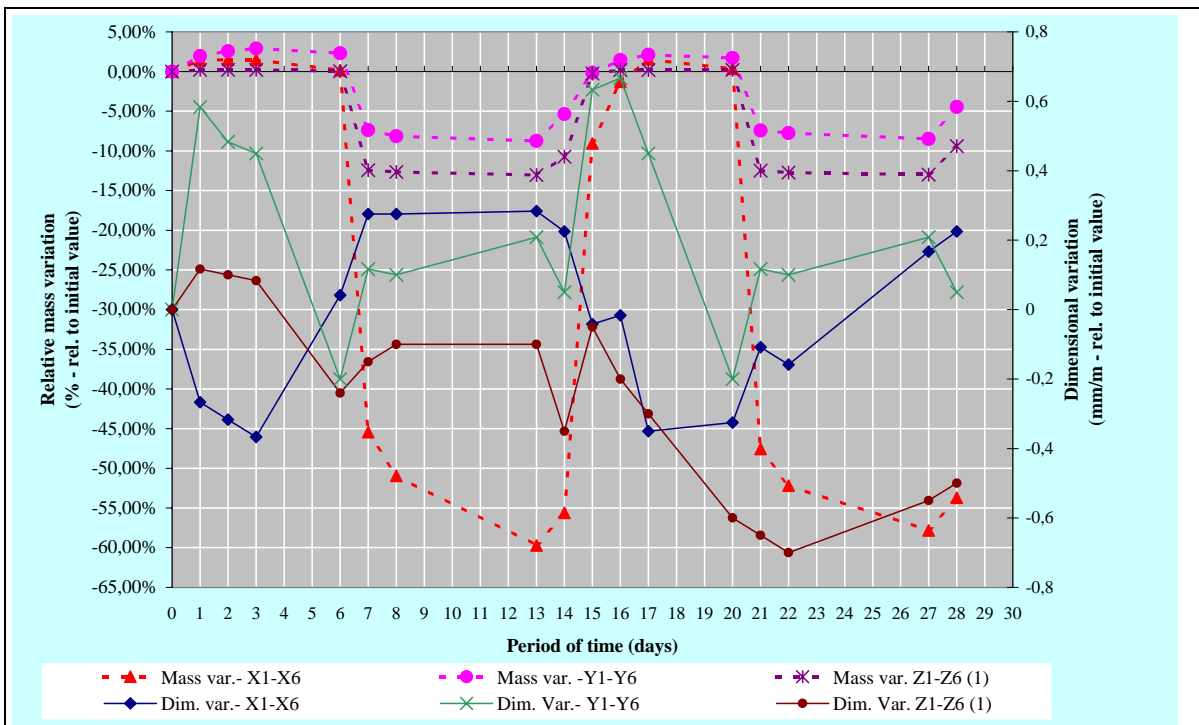


Figure 3-Mass and dimensional variation of X1-X6 (autoclaved aerated concrete), Y1-Y6 (lightweight concrete with expanded clay aggregate) and Z1-Z6 (bricks) - Temperature and moisture variation test

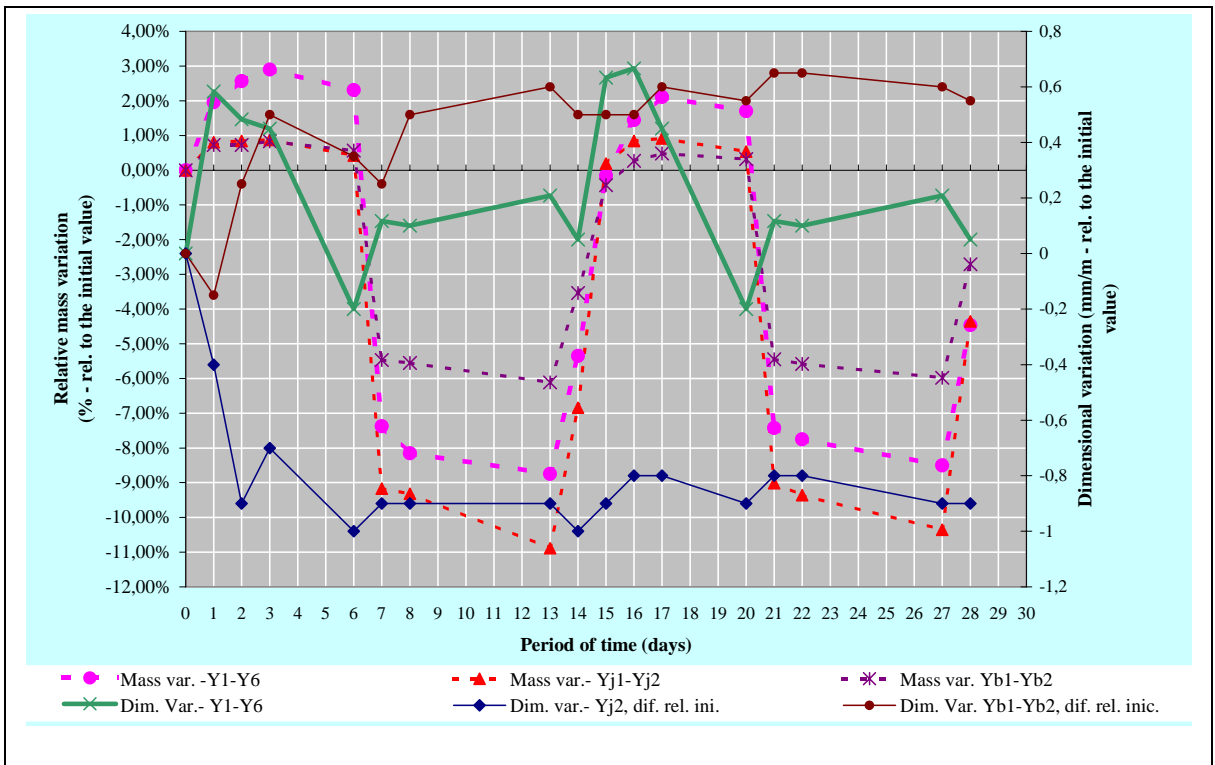


Figure 4-Mass and dimensional variation of Y1-Y6 series (lightweight concrete blocks with expanded clay aggregate) and Yj1,Yj2 (with mortar joint - block/block), Yb1,Yb2 (with mortar joint - block/concrete) - Temperature and moisture variation test (dif. rel. ini. - means difference of dimensional variation ($\Delta/l_1 - \Delta/l_2$) relative to the initial value)



Figure 5-Measuring mass variation of Yj1 specimen (with mortar joint - block/block)



Figure 6-Measuring mass variation of Yb1 specimen (with mortar joint - block/concrete)

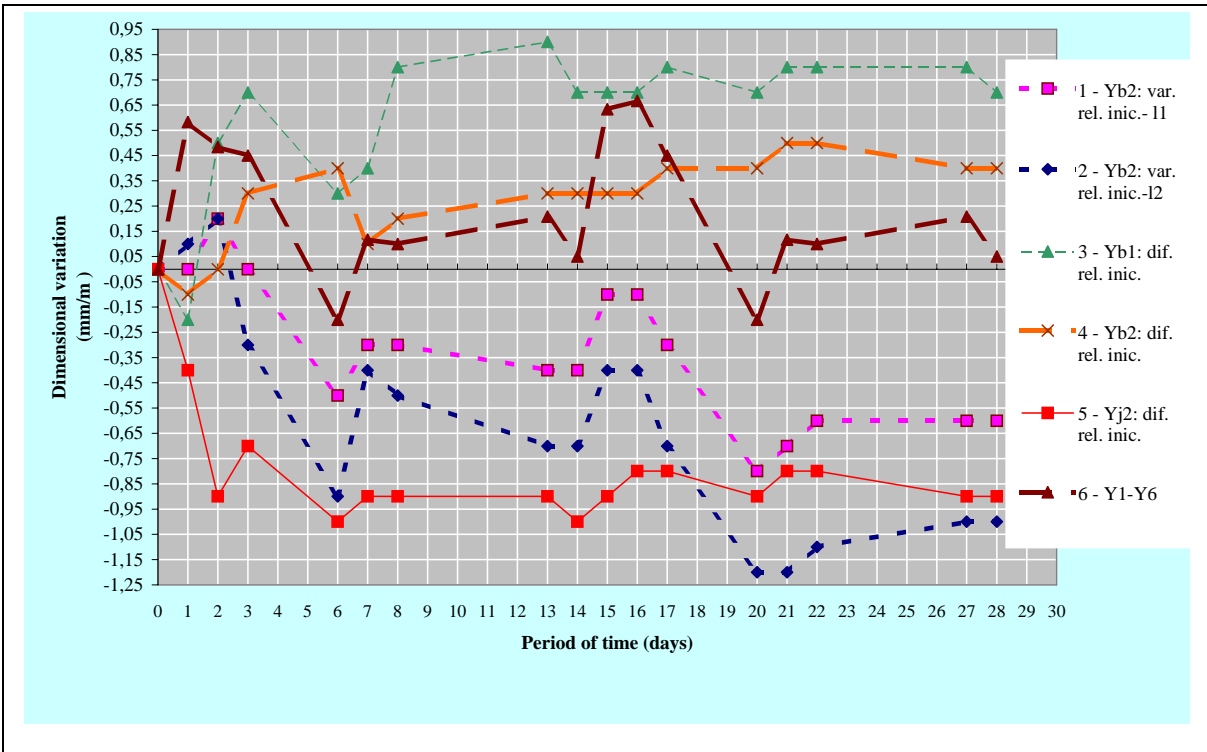


Figure 7-Dimensional variation of Y1-Y6 series (lightweight concrete blocks with expanded clay aggregate) and Yb1,Yb2 (with mortar joint – block/concrete),Yj2 (with mortar joint- block/block) - Temperature and moisture variation test (dif. rel. ini. - means difference of dimensional variation ($\Delta l_1-\Delta l_2$) relative to the initial value)